

COLUMN SCHEDULE

MARK	SIZE	BASE E.	CAP E.
C-1	6" x 14" TS	SEE DETAIL S-1/0520	SEE DETAIL S-0/0520

PILASTER SCHEDULE

MARK	SIZE	REINFORCEMENT	REMARKS
P-1	8" x 16"	4 #5 VERTICAL	SEE DETAIL P-0/0510

1. DONUT PILASTER REINFORCING INTO FOOTING.

FOUNDATION PLAN
 SCALE: 1/8" = 1'-0"

- NOTES:
- FOOTING ELEVATIONS INDICATED AS (X'-X") ARE REFERENCED FROM FINISH FLOOR ELEVATION OF 0'-0" (ASSUMED).
 - FOOTING MARKS ARE AS FOLLOWS:
 F-X FOOTING REFERENCE SEE FOOTING SCHEDULE FOR SIZE AND REINFORCING.
 (X'-X") TOP OF FOOTING ELEVATION (SEE NOTE 1).
 - C-X COLUMN REFERENCE SEE COLUMN SCHEDULE FOR SIZE.
 - PROVIDE #5 VERT. REINFORCING @ 48" O.C. IN ALL EXTERIOR MASONRY WALLS. DONUT REINFORCING INTO FOOTING AND EXTEND TO TOP OF WALL. FILL CELLS W/ GROUT.

FOOTING SCHEDULE

MARK	SIZE	THK.	REINFORCEMENT	REMARKS
F-1	2'-6" X 2'-6"	12"	3 #4 EACH WAY	
F-2	5'-0" X 5'-0"	12"	3 #4 EACH WAY	
F-3	5'-6" X 5'-6"	12"	4 #4 EACH WAY	SEE DETAIL F-11/0510
F-4	4'-0" X 4'-0"	12"	4 #5 EACH WAY	SEE DETAIL S-1/0520 FOR BASE #4 ANCHOR BOLTS.
F-5	4'-6" X 4'-6"	12"	5 #5 EACH WAY	
F-6	5'-0" X 5'-0"	12"	6 #5 EACH WAY	
F-7	5'-0" X 5'-0"	12"	#5 @ 12" EACH WAY	
F-8	6'-0" X 6'-0"	16"	6 #6 EACH WAY	
F-9	6'-0" X 16'-0"	24"	#6 @ 12" E.H. T&B	SEE DETAIL S-1/0520 FOR BASE #4 ANCHOR BOLTS.

1. PROVIDE 3" COVER (MIN) FOR ALL REINFORCING.

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PROJECT NAME
 The Shops at Crossroads
 Partners Two Investment Corp.
 3264 Meclock Bridge Road
 Norcross, Georgia 30092

DRAWING TITLE
 Foundation Plan

PROJECT NUMBER
 MTA-07-005-01

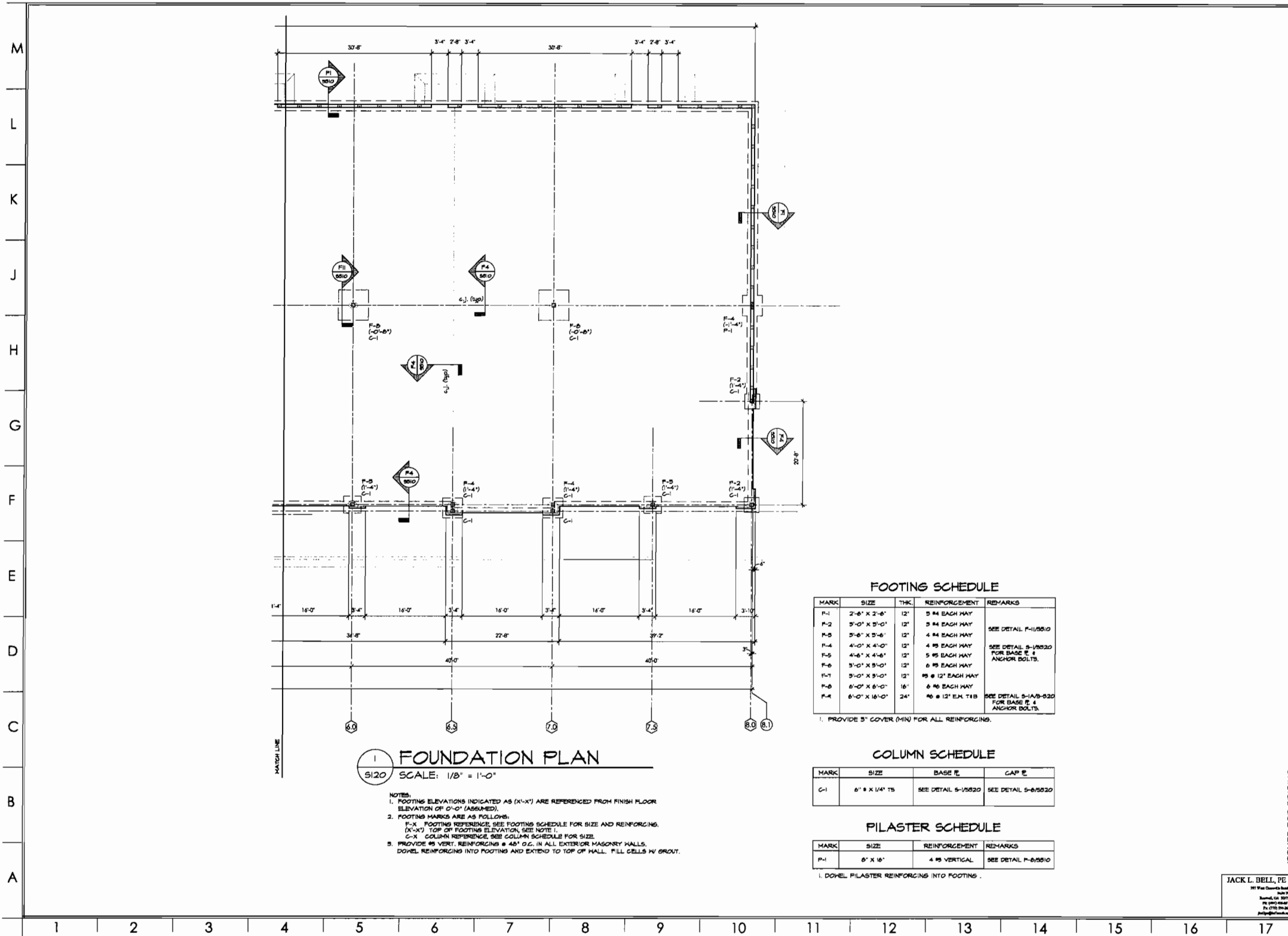
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FOUNDATION PLAN
 S120 SCALE: 1/8" = 1'-0"

- NOTES:
- FOOTING ELEVATIONS INDICATED AS (X'-X") ARE REFERENCED FROM FINISH FLOOR ELEVATION OF 0'-0" (ASSUMED).
 - FOOTING MARKS ARE AS FOLLOWS:
 F-X FOOTING REFERENCE. SEE FOOTING SCHEDULE FOR SIZE AND REINFORCEMENT.
 (X'-X") TOP OF FOOTING ELEVATION. SEE NOTE 1.
 C-X COLUMN REFERENCE. SEE COLUMN SCHEDULE FOR SIZE.
 - PROVIDE #5 VERT. REINFORCING @ 48" O.C. IN ALL EXTERIOR MASONRY WALLS. DOVEL REINFORCING INTO FOOTING AND EXTEND TO TOP OF WALL. FILL CELLS W/ GROUT.

FOOTING SCHEDULE

MARK	SIZE	THK.	REINFORCEMENT	REMARKS
F-1	2'-6" X 2'-6"	12"	5 #4 EACH WAY	
F-2	5'-0" X 5'-0"	12"	5 #4 EACH WAY	SEE DETAIL F-1/US510
F-3	5'-6" X 5'-6"	12"	4 #4 EACH WAY	
F-4	4'-0" X 4'-0"	12"	4 #5 EACH WAY	SEE DETAIL S-1/US530
F-5	4'-6" X 4'-6"	12"	5 #5 EACH WAY	FOR BASE # 4 ANCHOR BOLTS.
F-6	5'-0" X 5'-0"	12"	6 #5 EACH WAY	
F-7	5'-0" X 5'-0"	12"	#5 @ 12" EACH WAY	
F-8	6'-0" X 6'-0"	16"	6 #6 EACH WAY	
F-9	6'-0" X 16'-0"	24"	#6 @ 12" E.M. T18	SEE DETAIL S-1A/B-020 FOR BASE # 4 ANCHOR BOLTS.

1. PROVIDE 5" COVER (MIN) FOR ALL REINFORCING.

COLUMN SCHEDULE

MARK	SIZE	BASE #	CAP #
C-1	6" X 14" TS	SEE DETAIL S-1/US520	SEE DETAIL S-6/US530

PILASTER SCHEDULE

MARK	SIZE	REINFORCEMENT	REMARKS
P-1	6" X 16"	4 #5 VERTICAL	SEE DETAIL P-6/US510

1. DOVEL PILASTER REINFORCING INTO FOOTING.



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 Partners Two Investment Corp.
 3264 Medlock Bridge Road
 Norcross, Georgia 30092

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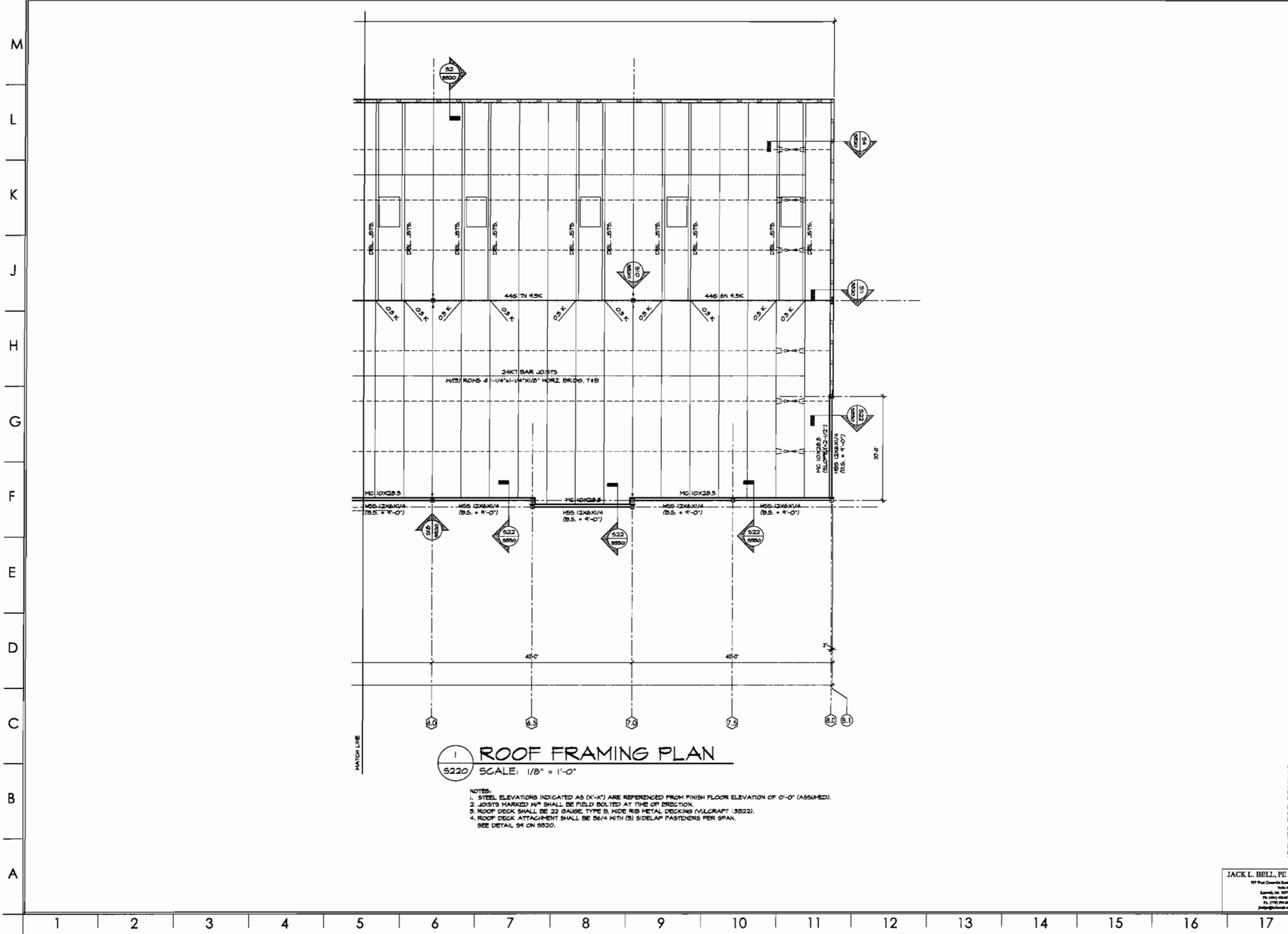
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5220 **ROOF FRAMING PLAN**
SCALE: 1/8" = 1'-0"

- NOTES:
1. STEEL ELEVATIONS INDICATED AS (K-K) ARE REFERENCED FROM FINISH FLOOR ELEVATION OF 0'-0" (ASSUMED).
 2. JOISTS MARKED N/P SHALL BE FIELD BOLTED AT TIME OF ERECTION.
 3. ROOF DECK SHALL BE 22 GAUGE TYPE D, INDE R18 METAL DECKING (NLCRAFT 18022).
 4. ROOF DECK ATTACHMENT SHALL BE 56/4 WITH (B) S/DELAP PASTENERS PER SPAN. SEE DETAIL 94 ON 5220.

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Roof Framing Plan

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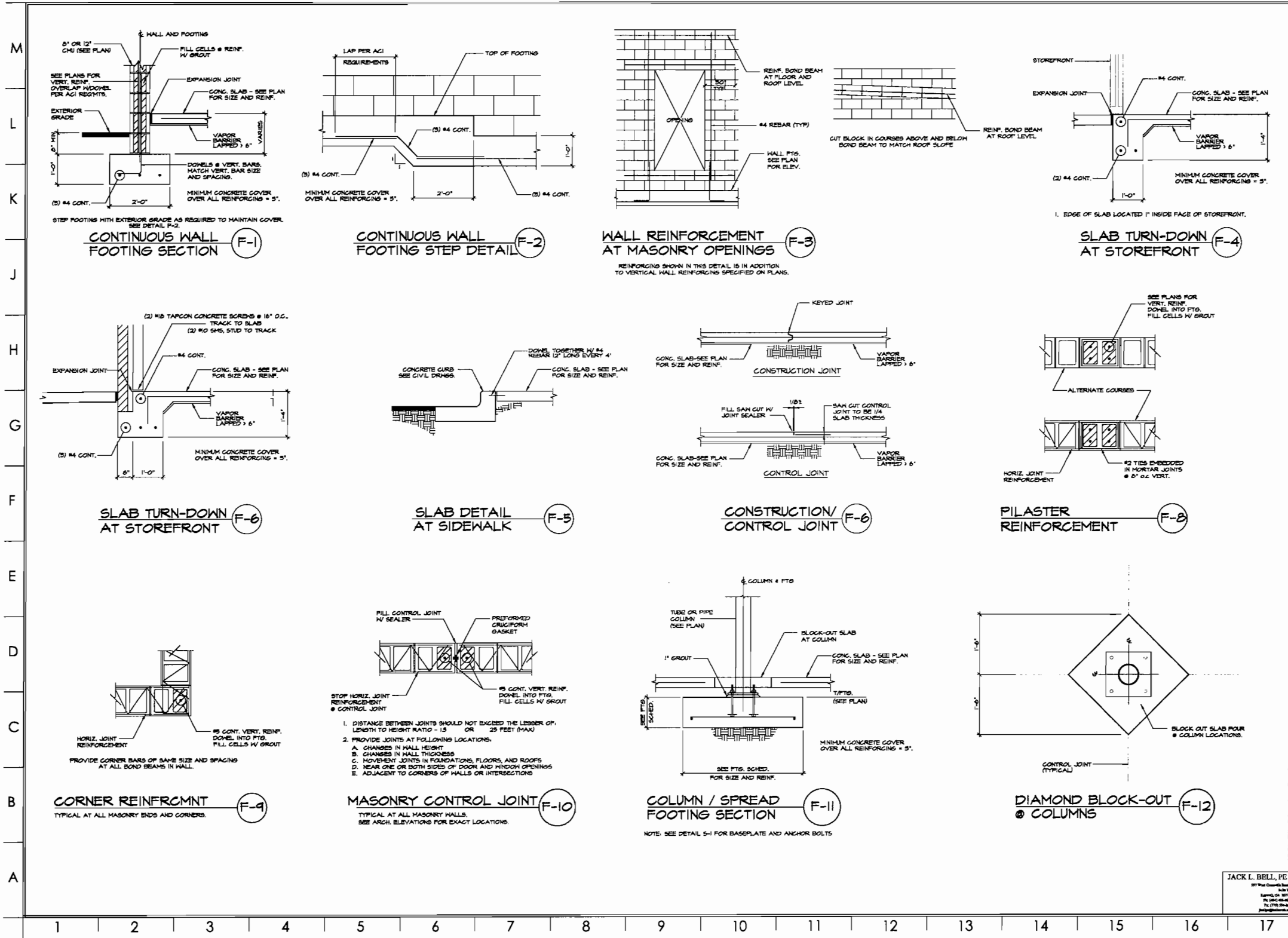
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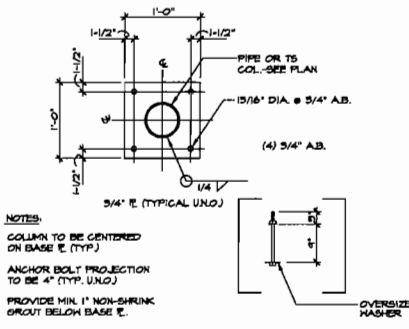
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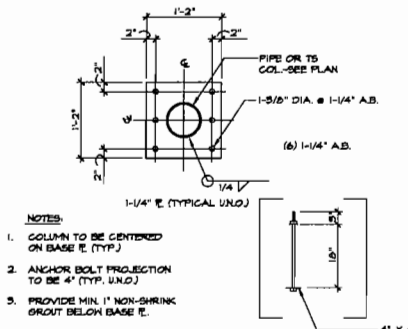
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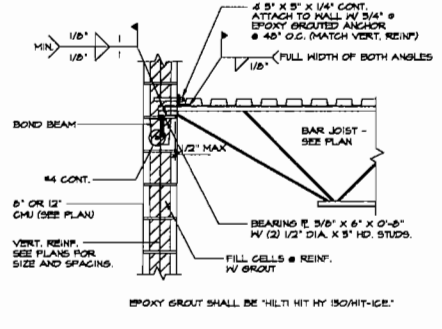
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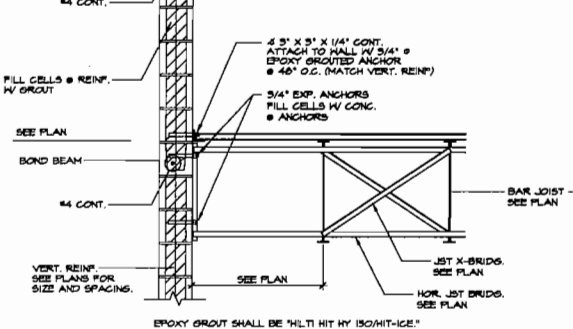
BASE PLATE DETAIL S-1



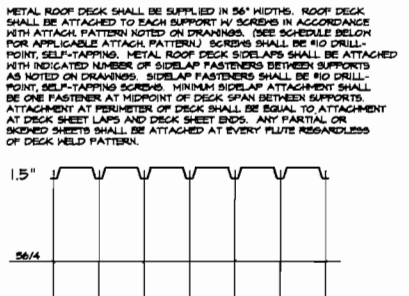
BASE PLATE DETAIL S-1A



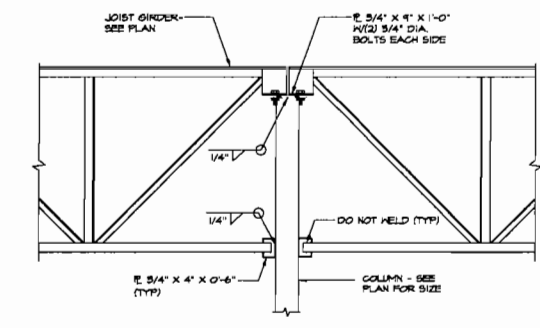
ROOF JOIST DETAIL S-2



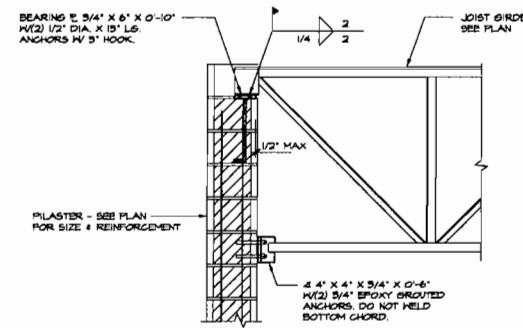
DECK BEARING DETAIL S-4



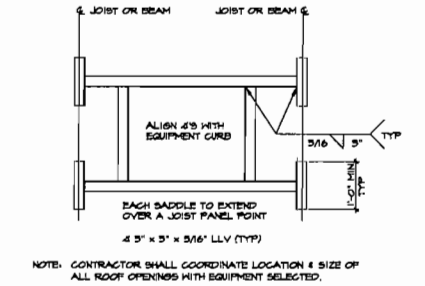
DECK ATTACHMENT DETAIL S-9



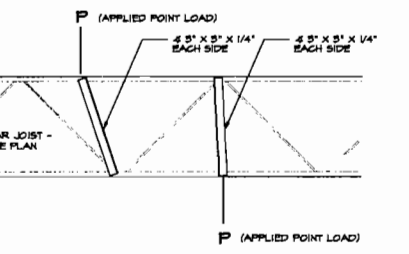
JOIST GIRDER TO COLUMN S-10



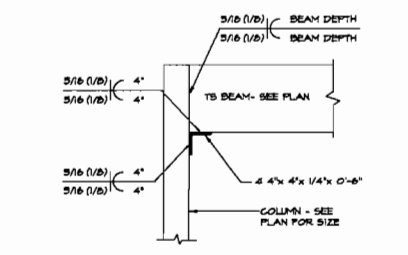
JOIST GIRDER ON PILASTER S-11



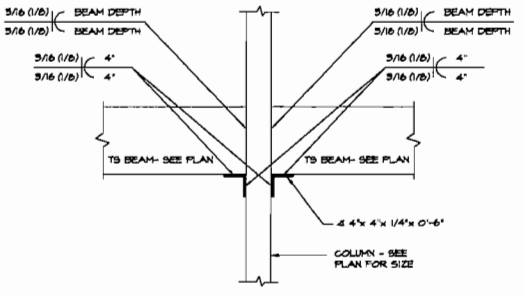
ROOF CURB FRAME DETAIL S-13



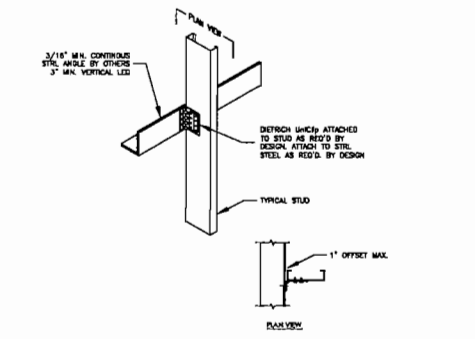
JOIST REINFORCEMENT DETAIL S-14



BEAM / COL DETAIL S-17



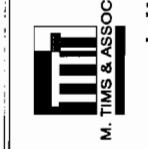
BEAM / COL DETAIL S-18



STUD CONNECTION DETAIL S-20



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STRUCTURAL NOTES:

BUILDING CODES USED FOR DESIGN:
 DESIGN IS IN COMPLIANCE WITH 2000 STANDARD BUILDING CODE
 & GEORGIA STATE AMENDMENTS.
DESIGN LOADS
 A. ROOF LIVE LOAD = 30 PSF
 B. ROOF DEAD LOAD = 30 PSF
 C. BASIC WIND SPEED = 140 MPH (3 SEC GUST)
 WIND IMPORTANCE FACTOR, $I_w = 1.00$
 WIND EXPOSURE CATEGORY = B

ROOF STRUCTURE IS DESIGNED TO SUPPORT ROOF TOP MECHANICAL UNITS.
 D. COMPONENTS AND GLAZINGS NOT DESIGNED BY THE ENGINEER OF RECORD SHALL BE DESIGNED FOR THE FOLLOWING WIND PRESSURES:
 WALLS
 MAIN AREA POSITIVE 17.1 PSF
 NEGATIVE 10.7 PSF
 CORNERS POSITIVE 21.1 PSF
 NEGATIVE 13.9 PSF
 ROOF
 MAIN AREA POSITIVE 8.1 PSF
 NEGATIVE 14.0 PSF
 CORNERS POSITIVE 22.8 PSF
 NEGATIVE 12.3 PSF

E. SEISMIC USE GROUP: I
 SEISMIC SITE CLASS: D
 DESIGN SPECTRAL RESPONSE ACCELERATION COEFFICIENT $S_{DS} = 0.50$
 DESIGN SPECTRAL RESPONSE ACCELERATION COEFFICIENT $S_{D1} = 0.18$
 DESIGN SEISMIC CATEGORY: C
 BASIC SEISMIC FORCE RESISTING SYSTEM:
 ORDINARY REINFORCED MASONRY SHEAR WALLS
 RESPONSE MODIFICATION COEFFICIENT, $R = 1.42$
 SEISMIC ANALYSIS PROCEDURE: SIMPLIFIED ANALYSIS PROCEDURE

NOTES:
 1. THE CONTRACTOR SHALL FOLLOW WRITTEN DIMENSIONS ONLY. DO NOT SCALE DRAWINGS.
 2. ALL DETAILS AND SECTIONS SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL BE CONSIDERED TO APPLY AT ANY SIMILAR SITUATION ELSEWHERE ON THE JOB EXCEPT WHERE A DIFFERENT DETAIL OR SECTION IS SHOWN.
 3. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. WHERE DISCREPANCIES OCCUR, IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE ARCHITECT PRIOR TO CONSTRUCTION.

4. THE STEEL FRAMING MEMBERS SHOWN RELY ON BUILDING COMPONENTS OTHER THAN STRUCTURAL STEEL FOR FINAL STRUCTURAL STABILITY (PREVIOUSLY REFERRED TO AS A NON-Self-SUPPORTING STEEL FRAME BY THE AISC 'CODE OF STANDARD PRACTICE'). THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND PROVISION OF ANY AND ALL TEMPORARY BRACING AND SHORING AGAINST WIND, ERECTION AND ALL CONSTRUCTION LOADS UNTIL ALL ELEMENTS, MEMBERS AND CONNECTIONS (FLOORS, ROOFS, SHEAR WALLS, ETC.), AS SHOWN ON THE CONTRACT DOCUMENTS, ARE COMPLETELY INSTALLED. THE STRUCTURAL MEMBERS SHOWN ON THE CONTRACT DOCUMENTS ARE DESIGNED FOR THE ANTICIPATED LOADS THAT THE STRUCTURE WILL BE SUBJECTED TO ONLY AFTER ALL STRUCTURAL ELEMENTS ARE IN PLACE AND FINAL CONNECTIONS ARE COMPLETE.
 5. THE GENERAL CONTRACTOR SHALL VERIFY ALL OPENING SIZES, PAD SIZES AND LOCATIONS WITH THE RESPECTIVE CONTRACTORS.

EXCAVATION AND EARTHWORK
 1. THE SOILS AND FOUNDATION ENGINEERING REPORT IS FOR INFORMATIONAL PURPOSES ONLY AND SHALL NOT BE CONSIDERED PART OF THE CONTRACT DOCUMENTS.
 2. WATER LEVELS INDICATED ON THE BORING LOGS MAY BE SUBJECT TO SEASONAL AND/OR ANNUAL VARIATIONS. A DEWATERING SYSTEM OF SUFFICIENT CAPACITY SHALL BE INSTALLED AND OPERATED TO MAINTAIN THE CONSTRUCTION AREA FREE OF WATER AT ALL TIMES.
 3. THE BEARING VALUE OF THE SOIL HAS BEEN DETERMINED BY FIELD EXPLORATION AND LABORATORY ANALYSIS. THE FOUNDATION DESIGN IS BASED ON THE FOLLOWING NET ALLOWABLE BEARING PRESSURES:
 SPREAD FOOTINGS: 3000 PSF
 WALL FOOTINGS: 3000 PSF

4. ALL FOOTING EXCAVATIONS SHALL BE INSPECTED PRIOR TO CONCRETE PLACEMENT BY A SOILS ENGINEER TO VERIFY SUITABLE BEARING MATERIAL OF CAPACITY AS SPECIFIED.
 5. NOTIFY THE OWNER'S REPRESENTATIVE WHEN ADDITIONAL EXCAVATION IS REQUIRED TO REACH SUITABLE BEARING MATERIAL.
 6. THE SOILS ENGINEER SHALL CERTIFY IN WRITING THAT ALL FOUNDATIONS WERE PLACED ON SOIL WITH THE BEARING VALUE AS SPECIFIED.
 7. WITHIN THE EXCAVATION AREA OF THE FOUNDATIONS, ALL VEGETATION, TOPSOIL, PREVIOUSLY PLACED FILL AND UNSUITABLE SOILS SHALL BE REMOVED. ALL FOOTINGS TO BEAR ON VIRGIN SOIL OR PROPERLY PLACED AND COMPACTED ENGINEERED FILL.
 8. FOUNDATION DESIGN DOES NOT ACCOUNT FOR HINTER CONSTRUCTION. ANY UNENCLOSURE/HEATED SPACES SHALL BE ADEQUATELY PROTECTED AGAINST FROST DURING HINTER CONSTRUCTION BY CONTRACTOR.

CONCRETE
 1. ALL CONCRETE WORK INCLUDING FORMING, REINFORCING, MIXING, PLACING AND CURING SHALL BE DONE IN ACCORDANCE WITH THE ACI MANUAL OF CONCRETE PRACTICE INCLUDING BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, ACI 308 AND SPECIFICATIONS FOR STRUCTURAL CONCRETE, ACI 308.
 2. ALL CONCRETE SHALL ATTAIN A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF $f_c = 5000$ PSI FOR FOUNDATIONS AND $f_c = 4000$ PSI FOR BASEMENT WALLS AND RETAINING WALLS SLABS AT 28 DAYS.
 3. CONCRETE SHALL BE VIBRATED INTO FORMS WHILE PLACING, WITHOUT OVER-VIBRATING. REINFORCING SHALL BE PROTECTED BY CONCRETE OF THICKNESS AS FOLLOWS:
 FOOTINGS 8"
 PILES 15" TO TIES
 BASEMENT WALLS:
 EXTERIOR FACE 2"
 INTERIOR FACE 1"
 OTHER FOUNDATION 2" BOTH FACES
 COLUMNS 15" TO TIES
 EXPOSED EXTERIOR CONCRETE 2"
 SLAB ON GRADE 1/8" FROM TOP

4. SLAB-ON-GRADE SHALL BE PLACED IN CONTINUOUS STRIPS PER ACI RECOMMENDATIONS.
 5. COORDINATE CONCRETE WORK WITH THAT OF OTHER TRADES TO ALLOW FOR SETTING OF SLEEVES, ACCESSORIES, ETC.
 6. ALL ANCHOR RODS SHALL BE IN PLACE PRIOR TO POURING OF CONCRETE.
 7. CONCRETE TEST REPORTS SHALL BE MADE AVAILABLE AT JOBSITE FOR REVIEW BY THE INSPECTOR.
 8. DESIGN OF CONCRETE STRUCTURAL ELEMENTS INCLUDING WALLS, FORMED SLABS, BEAMS AND COLUMNS IS IN ACCORDANCE WITH ACI 318-05.

REINFORCING STEEL
 1. THE REINFORCING STEEL CONTRACTOR SHALL FABRICATE ALL REINFORCING AND FURNISH ACCESSORIES, CHAIRS, SPACERS BARS AND SUPPORTS NECESSARY TO SECURE THE REINFORCEMENT UNLESS SHOWN OTHERWISE ON THE PLANS AND/OR DETAILS.
 2. REINFORCING STEEL SHALL BE ASTM A601, GRADE 60.
 3. HELDED WIRE FABRIC SHALL CONFORM TO ASTM A954.
 4. CONCRETE REINFORCEMENT SHALL BE PLACED ACCORDING TO THE CRSI RECOMMENDED PRACTICE FOR PLACING REINFORCING BARS.
 5. ALL REINFORCEMENT SPLICES SHALL BE LAPPED PER ACI REQUIREMENTS UNLESS OTHERWISE NOTED. PROVIDE CORNER BARS FOR ALL HORIZONTAL REINFORCEMENT AT CORNERS AND INTERSECTIONS.
 6. TOP BARS SHALL BE HOOKED AT END SPANS.
 7. REINFORCEMENT TO BE HELDED SHALL CONFORM TO THE REQUIREMENTS OF ASTM A 706 AND AS NOTICED IN THE PROJECT SPECIFICATION TOUGH UP ALL STRUCTURAL WELDING CODE-REINFORCING STEEL BY AMERICAN WELDING SOCIETY.

JOINTS IN CONCRETE
 1. CONSTRUCTION AND/OR CONTROL JOINTS SHALL BE MADE AS DETAILED ON THE DRAWINGS.
 2. CONSTRUCTION AND/OR CONTROL JOINTS FOR SLAB-ON-GRADE CONSTRUCTION SHALL BE LOCATED ON COLUMN LINES.
 3. MAXIMUM SPACING OF CONSTRUCTION AND/OR CONTROL JOINTS IN SLAB-ON-GRADE CONSTRUCTION SHALL BE AS SHOWN ON DRAWINGS.
 4. CONSTRUCTION OR CONTROL JOINTS IN CONCRETE FOUNDATION WALLS SHALL BE SPACED NO FURTHER THAN 20 FEET APART.
 5. CONSTRUCTION JOINTS FOR ELEVATED PLATEWORK SHALL BE LOCATED AT THE CENTER OF THE SPAN. REINFORCEMENT SHALL BE CONTINUOUS ACROSS ALL CONSTRUCTION JOINTS.
 6. MAXIMUM SPACING OF CONSTRUCTION AND/OR CONTROL JOINTS IN CANTILEVERED RETAINING WALLS SHALL BE 20'-0".
 7. 1" EXPANSION JOINTS IN CANTILEVERED RETAINING WALLS SHALL BE PLACED AT 40' ON CENTER MAXIMUM.

BACKFILLING
 1. NO BACKFILLING AND COMPACTING OF EARTH SHALL BE PERMITTED AGAINST BASEMENT WALLS UNTIL SUPPORTING SLABS HAVE BEEN POURED AND HAVE REACHED 75% OF THEIR DESIGN STRENGTH OR UNLESS ADEQUATE BRACING SUBMITTED FOR REVIEW IS PROVIDED.
 2. BOTH SIDES OF FOUNDATION WALLS FOR SLAB-ON-GRADE CONSTRUCTION SHALL BE BACKFILLED SIMULTANEOUSLY SO AS TO PREVENT OVERTURNING OR LATERAL MOVEMENT OF WALLS.
 3. ALL GRADE BEAMS SHALL BE ADEQUATELY BRACED TO PREVENT LATERAL MOVEMENT DURING BACKFILLING AND COMPACTION.
 4. FOUNDATIONS SHALL BE BACKFILLED AS SOON AS PRACTICABLE SO AS TO LIMIT THE EFFECTS OF WATER INFILTRATION AND FROST HEAVE. IF FOUNDATIONS ARE UNABLE TO BE BACKFILLED SUFFICIENTLY PRIOR TO GOLD HEATER, ADEQUATE MEASURES SHALL BE INSTITUTED BY CONTRACTOR TO PROTECT FOUNDATIONS FROM FROST HEAVE.
 5. UNLESS NOTED OTHERWISE, HINTER FOUNDATIONS ARE NOT DESIGNED TO BEAR AT ELEVATIONS REQUIRED TO PROTECT AGAINST FROST HEAVE. IF HINTER CONSTRUCTION IS ANTICIPATED, CONTRACTOR SHALL BE WHOLLY RESPONSIBLE FOR PROVIDING ADEQUATE MEASURES TO PROTECT AGAINST FROST HEAVE.

FOUNDATION UNDERGROUND MECHANICAL COORDINATION
 1. UNDERGROUND WATER, GAS LINES, ETC. CROSSING CONTIGUOUS WALL FOUNDATIONS SHALL NOT PASS THROUGH FOOTINGS. WHERE PIPE OCCURS ABOVE TOP OF FOOTING, SLEEVE THROUGH WALL. WHERE PIPE OCCURS IN FOOTING DEPTH, DROP TOP OF FOOTING SUCH THAT PIPE PASSES JUST ABOVE FOOTING. IF TOP OF PIPE IS LESS THAN 4" BELOW BOTTOM OF FOOTING, PROVIDE 1" COMPRESSIBLE FOAM INSULATION BELOW FOOTING FOR WIDTH OF TRENCH.

MASONRY
 1. CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C 90-04. COMPRESSIVE STRENGTH OF MASONRY SHALL BE DETERMINED BY THE UNIT STRENGTH METHOD. NET AREA COMPRESSIVE STRENGTH, f_m , OF MASONRY SHALL BE 300 PSI.
 2. MORTAR AND ITS INGREDIENTS AS DELIVERED TO THE MASON SHALL CONFORM TO ASTM C 210, TYPE S.
 3. MASONRY CEMENT SHALL CONFORM TO ASTM C 91, TYPE S.
 4. BRICK FOR REINFORCED AND NON-REINFORCED MASONRY SHALL CONFORM TO ASTM C 90.
 5. MASONRY CONSTRUCTION SHALL CONFORM TO THE LATEST STANDARDS OF THE MASONRY STANDARDS JOINT COMMITTEE (ACI 530-08) AND (MCA 901-08).
 6. MASONRY BORDERS SHALL BE RUNNING BOND UNLESS NOTED OTHERWISE.

STRUCTURAL STEEL

1. FABRICATION AND ERECTION OF STRUCTURAL STEEL MEMBERS IS TO BE IN ACCORDANCE WITH THE AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES AS INDICATED IN THE NINTH EDITION OF THE "MANUAL OF STEEL CONSTRUCTION" FOR ALLOWABLE STRESS DESIGN UNLESS NOTED OTHERWISE.
 2. ALL CONNECTIONS SHALL BE BOLTED OR WELDED AND SHALL BE DESIGNED FOR THE END REACTIONS INDICATED ON PLANS OR IF NOT INDICATED SHALL BE DESIGNED TO DEVELOPE 80% OF THE ALLOWABLE UNIFORM LOAD TABULATED IN THE NINTH EDITION OF THE AISC "MANUAL OF STEEL CONSTRUCTION" FOR ALLOWABLE STRESS DESIGN UNLESS NOTED OTHERWISE. NUMBER OF BOLTS MUST SATISFY MIN. REQUIREMENTS AS FOLLOWS: 2 BOLTS PER CONNECTION FOR 8" AND 10" DEEP MEMBERS, 3 FOR 12" AND 14" DEEP MEMBERS, 4 FOR 16" AND 18" DEEP MEMBERS, 5 FOR 20" AND 24" DEEP MEMBERS AND 6 FOR 27" & DEEPER MEMBERS.
 3. ALL STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING STANDARDS:
 STEEL WIDE FLANGES, CHANNELS ASTM A992
 STEEL ANGLE & PLATES ASTM A36
 STEEL PIPE ASTM A53, GRADE B OR ASTM A501
 STEEL TIE ASTM A500, GRADE B
 4. ALL FILLER METAL USED IN WELDING SHALL BE TENSILE YIELD, LOW-HYDROGEN.
 5. ALL WELDING SHALL BE BY CERTIFIED WELDERS AND SHALL CONFORM TO THE AISC AND MEET AISC MINIMUM REQUIREMENTS FOR WELD SIZE. ALL WELDED JOINTS SHALL CONFORM TO THE PROVISIONS OF AND THE STRUCTURAL WELDING CODE BY AMERICAN WELDING SOCIETY. ALL WORK SHALL BE PERFORMED BY FULLY QUALIFIED WELDERS IN THE TYPE OF CONSTRUCTION INVOLVED. WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.

6. BOLTS SHALL BE HIGH-STRENGTH A-325 BOLTS. CONNECTIONS SHALL CONFORM TO AISC 308. THE SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A-325 OR A-490 BOLTS' CONNECTIONS ARE BEARING TYPE.
 DESIGN OF SPECIAL CONNECTIONS BETWEEN STEEL FRAMING COMPONENTS BY OTHER THAN THE PROJECT STRUCTURAL ENGINEER-OF-RECORD SHALL BE PERFORMED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF GEORGIA INCLUDING BUT NOT LIMITED TO BRACE END CONNECTIONS MOMENT-RESISTING CONNECTIONS, MODIFIED BEAM SEAT CONNECTIONS, AND MEMBER SPLICE CONNECTIONS.
 7. STEEL COLUMN BASE PLATES SHALL BE SIZE SHOWN ON PLAN WITH 3/4" DIA. ANCHOR RODS (ASO) AND 1" NON-METALLIC NON-SHRINK GROUT FOR UNIFORM BEARING.
 8. UNLESS NOTED OTHERWISE, STRUCTURAL STEEL SUPPLIER IS TO FURNISH LWS 1/2" X 5/8" SHOP HELDED ANGLE FRAMES AT ALL ROOF OPENINGS. VERIFY SIZE AND LOCATION WITH CONTRACTOR.
 9. ALL PLATES USED IN BOLTED CONNECTIONS SHALL HAVE ROLLED OR GAS CUT EDGES.
 10. ALL STRUCTURAL STEEL AND MISCELLANEOUS METALS SHALL BE PRIME PAINTED WITH ONE COAT OF FABRICATORS STANDARD RUST-INHIBITIVE PRIMER OR SHALL BE FURNISHED FROM STEEL HAVING A 8-40 GALVANIZED COATING CONFORMING TO ASTM A653. ALL GALVANIZED 12, 14 AND 18 GA. MEMBERS SHALL MEET THE REQUIREMENTS OF ASTM A653 WITH A MINIMUM YIELD OF 50,000 PSI. ALL GALVANIZED 18 AND 20 GA. MEMBERS SHALL MEET THE REQUIREMENTS OF ASTM A653 WITH A MINIMUM YIELD OF 30,000 PSI.
 11. ALL LIGHT GAUGE METAL STUDS AND JOISTS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS REGARDING MINIMUM INSTALLATION STANDARDS FOR BEARING, BRIDGING, AND BRACING.
 12. ADJUSTABLE MASONRY TIES SHALL BE FURNISHED AT 18" O.C. VERTICALLY AND 6" O.C. HORIZONTALLY ON ALL STEEL MEMBERS ENCASED IN OR ADJACENT TO MASONRY WALLS WHETHER OR NOT SUCH ANCHORS ARE SHOWN ON THE DRAWINGS. TIES SHALL BE CAPABLE OF TRANSMITTING FORCES PERPENDICULAR TO THE PLANE OF THE WALL.

STEEL ROOF DECK
 1. STEEL ROOF DECK SHALL BE PAINTED OR GALVANIZED AS INDICATED IN THE PROJECT SPECIFICATION.
 2. END JOINTS OF ROOF DECK SHALL HAVE 2" MINIMUM END LAPS.
 3. ROOF DECK SHALL BE 1/2" X 22 GA. WIDE RIB DECK, TYPE B, UNLESS NOTED OTHERWISE.
 4. ROOF DECK IS TO BE HELDED AT 12" O.C. OVER ALL INTERIOR SUPPORTS AND 6" O.C. OVER ALL EXTERIOR SUPPORTS, UNLESS NOTED OTHERWISE ON DRAWINGS.
 5. HELDS TO HAVE A NOMINAL DIAMETER OF 3/8".
 6. PROVIDE SLOPELAP BRACES AS NOTED ON DRAWINGS OR REQUIRED FOR MINIMUM BIDEAPS BY THE STEEL DECK INSTITUTE.
 7. MAKE JOINTS OVER SUPPORTING MEMBER ONLY AND USE MINIMUM THREE SPANS.
 8. SELF-TAPPING SCREWS MUST BE USED WITH THE APPROVAL OF THE ARCHITECT AND STRUCTURAL ENGINEER.

STEEL JOISTS
 1. ALL STEEL JOISTS SHALL CONFORM TO THE REQUIREMENTS OF THE STEEL JOIST INSTITUTE STANDARD SPECIFICATIONS. JOIST FABRICATOR SHALL BE MEMBER OF THE S.I.
 2. BRIDGING FOR "K" SERIES STEEL JOISTS SHALL BE CONTINUOUS STEEL ANGLE TOP AND BOTTOM OF JOISTS IN LENGTHS TO PERMIT LAPPING AT JOIST PANEL POINTS FOR WELDING. HELD BRIDGING TO CHAIRS. SPACING AND SIZE OF BRIDGING SHALL BE ACCORDANCE WITH ALL RECOMMENDED CODE OR STANDARD PRACTICE.
 3. PROVIDE BOTTOM CHAIR CEILING EXTENSIONS AS SHOWN ON ARCHITECTURAL DRAWINGS OR AS NOTED OTHERWISE.
 4. WELDER ANGLE FOR STEEL JOISTS SHALL BE DESIGNED AND FURNISHED BY THE JOIST FABRICATOR AS NOTED ON THE DRAWINGS.

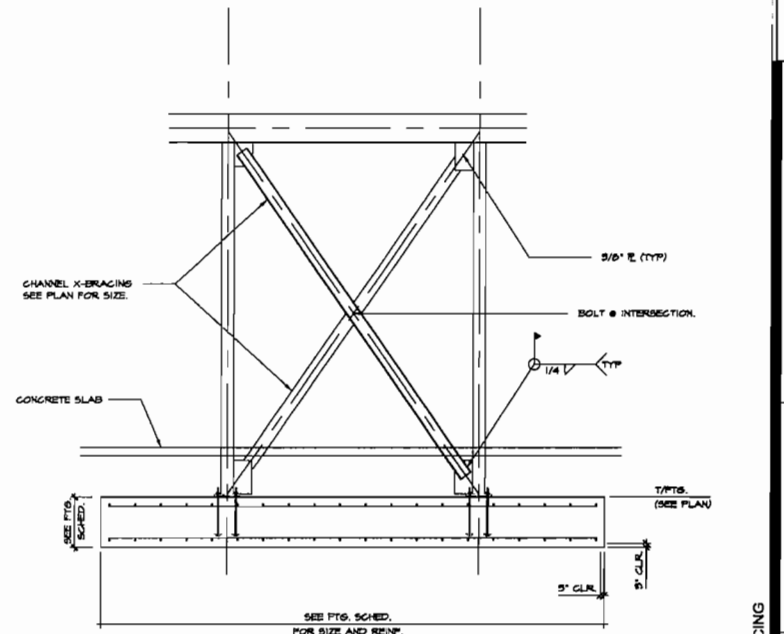
LINTEL NOTES
 1. LOOSE LINTELS FOR NON-LOAD BEARING WALLS SHALL HAVE A MINIMUM END BEARING OF 6" AND SHALL BE THE FOLLOWING SIZES UNLESS OTHERWISE NOTED (LINTEL SIZES GIVEN ARE FOR NON MASONRY CONSTRUCTION ONLY; SPANS NOTED ARE CLEAR SPANS):
 8" WALLS
 UP TO 4'-0" SPAN 2-L5x5x5/8 LLV
 4'-0" TO 6'-4" SPAN 2-L5x5x5/8 LLV
 TYPICAL WALLS 1-L FOR EACH 4" OF WALL THICKNESS
 UP TO 4'-0" SPAN L5x5x5/8
 OVER 4'-0" TO 6'-0" L5x5x5/8 LLV
 OVER 6'-0" TO 8'-0" L5x5x5/8 LLV
 OVER 8'-0" TO 12'-0" 16x16x5 PL ON BOTTOM
 PL TO BE 1/2" LESS THAN WALL WIDTH
 2. PLATES ON LINTELS SHALL BE SHOP HELDED TO MEMBER W/ 1/4" X 1/4" FILLET WELDS AT 12" O.C. STAGGERED.
 3. WELD TOGETHER ALL BACK-TO-BACK ANGLES.
 4. CONTRACTOR SHALL COORDINATE ALL REQUIRED OPENINGS IN NON-LOAD BEARING MASONRY WALLS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.

SHOP DRAWINGS
 1. SHOP DRAWINGS, UNLESS OTHERWISE NOTED, SHALL BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.
 2. PRIOR TO SUBMITTAL, THE CONTRACTOR SHALL REVIEW THE SHOP DRAWINGS AND MAKE ANY CORRECTIONS REQUIRED. THE CONTRACTOR SHALL STAMP AND SIGN THE DRAWINGS THAT HE HAS REVIEWED THEM.
 3. SHOP DRAWINGS SHALL BE FURNISHED FOR ALL STRUCTURAL COMPONENTS. ALL SUBMITTALS TO BE TWO (2) SETS PRINTS AND ONE (1) SET REPRODUCTIONS ONLY.

MISCELLANEOUS
 1. ALL ANCHOR BOLTS FOR MECHANICAL AND ELECTRICAL EQUIPMENT ARE FURNISHED AND LOCATED BY THE RESPECTIVE CONTRACTORS AND SET BY THE GENERAL CONTRACTOR EXCEPT WHERE THE OTHER CONTRACTORS FURNISH THEIR OWN CONCRETE PADS.
 2. ALL PIPE SLEEVES ARE FURNISHED BY AND LOCATED BY THE MECHANICAL AND ELECTRICAL CONTRACTOR AND SET BY THE GENERAL CONTRACTOR.
 3. THE GENERAL CONTRACTOR SHALL VERIFY ALL OPENING SIZES, PAD SIZES AND LOCATIONS WITH THE RESPECTIVE CONTRACTORS.
 4. ALL CORE DRILLING SHALL BE DONE BY THE MECHANICAL AND ELECTRICAL CONTRACTORS FOR THEIR OWN WORK UNDER THE SUPERVISION OF THE GENERAL CONTRACTOR. NO REINFORCING SHALL BE CUT. VERIFY LOCATION OF REINFORCING BEFORE CORE DRILLING. THERE SHALL NOT BE ANY CORE DRILLING THROUGH BEAMS OR COLUMNS. MAXIMUM CORE HOLE THROUGH SLABS SHALL BE FIVE DIAMETER PLUS 1". LIGHT GAUGE METAL FRAMING

1. ALL LIGHT GAUGE MEMBERS SHALL BE DESIGNED IN ACCORDANCE WITH AMERICAN IRON AND STEEL INSTITUTE (AISI) SPECIFICATION FOR THE DESIGN OF GOLD-FORMED STEEL STRUCTURAL MEMBERS.
 2. ALL LIGHT GAUGE METAL FRAMING MEMBERS SHALL BE OF THE TYPE, SIZE AND GAUGE AS SHOWN ON THE PLANS.
 3. ALL STUDS, JOISTS AND ACCESSORIES SHALL BE PRIME WITH RUST INHIBITIVE PAINT MEETING THE PERFORMANCE REQUIREMENTS OF TYP-400C. OR SHALL BE FURNISHED FROM STEEL HAVING A 8-40 GALVANIZED COATING CONFORMING TO ASTM A653.
 4. ALL PAINTED 12, 14 AND 18 GA. MEMBERS SHALL MEET THE REQUIREMENTS OF ASTM A653 WITH A MINIMUM YIELD OF 50,000 PSI. ALL GALVANIZED 12, 14 AND 18 GA. MEMBERS SHALL MEET THE REQUIREMENTS OF ASTM A653 WITH A MINIMUM YIELD OF 30,000 PSI.
 5. ALL PAINTED 18 AND 20 GA. MEMBERS SHALL MEET THE REQUIREMENTS OF ASTM A653 WITH A MINIMUM YIELD OF 40,000 PSI. ALL GALVANIZED 18 AND 20 GA. MEMBERS SHALL MEET THE REQUIREMENTS OF ASTM A653 WITH A MINIMUM YIELD OF 30,000 PSI.
 6. ALL WELDING IS TO BE DONE PER MANUFACTURER'S RECOMMENDATIONS ON ROOF TIE AND BRACINGS.
 7. ALL LIGHT GAUGE METAL STUDS AND JOISTS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS REGARDING MINIMUM INSTALLATION STANDARDS FOR BEARING, BRIDGING, AND BRACING.

BRICK-TIES FOR STUD BACK-UP
 1. THERE SHALL BE A MINIMUM ONE BRICK TIE FOR EVERY 2'-0" S.P. OF WALL AREA. THESE SHALL BE SPACED AT A MAXIMUM OF 24 INCHES ON CENTER. TIES SHALL BE OF MINIMUM 6 GA. CORROSION RESISTANT WIRE AND SHALL BE OF AN ADJUSTABLE TYPE SUCH AS "OUR-O-WAL" ADJUSTABLE DAD321 TYPE OR EQUAL. CORRUGATED GALVANIZED SHEET TIES ARE NOT ACCEPTABLE. ALL TIES MUST BE ATTACHED THROUGH THE SHEATHING TO THE STUDS PER MANUFACTURER'S RECOMMENDATIONS.



X-BRACING / FOOTING SECTION 5-27
 NOTE: SEE DETAIL 5-1A FOR BASEPLATE AND ANCHOR BOLTS

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