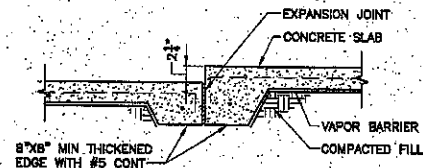
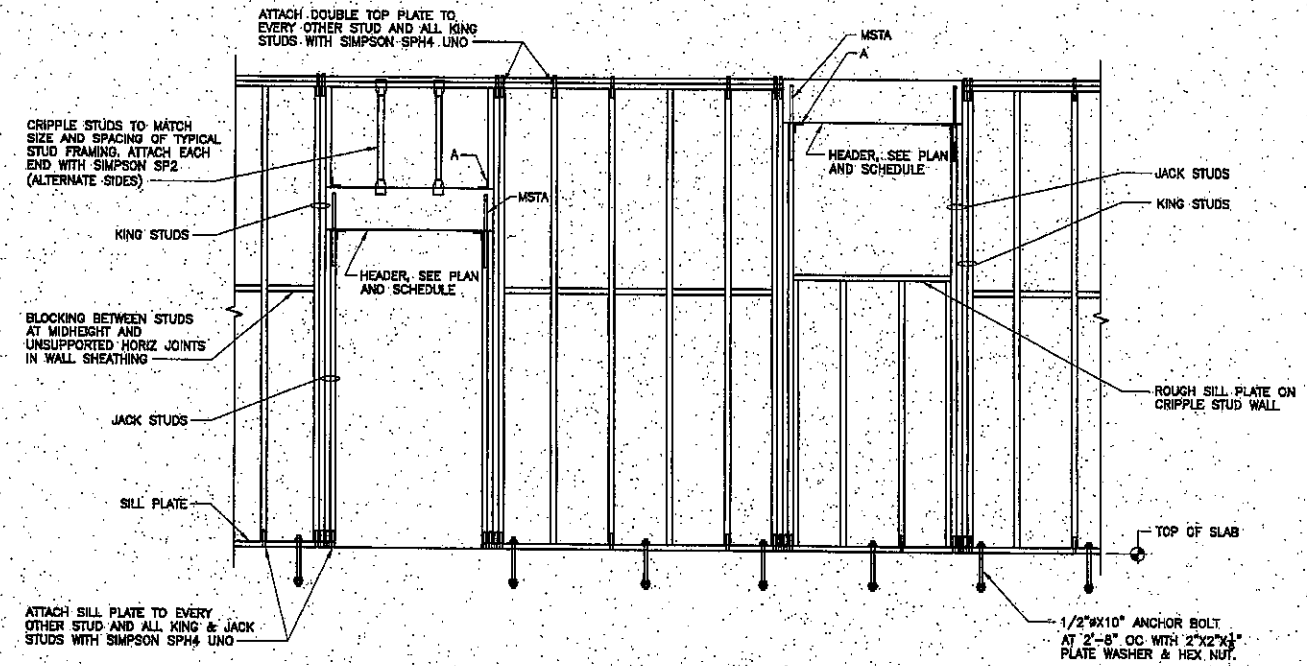


TYPICAL CONTRACTION JOINT DETAIL (A)
NOT TO SCALE

- NOTES:**
- CONTRACTION JOINT INDICATED BY "CJ" ON PLAN
 - SAW-CUT JOINTS SHALL BE MADE WITHIN 8 HOURS OF CONCRETE PLACEMENT
 - VERIFY LOCATION OF EXPOSED JOINTS WITH ARCHITECT. USE FORMED OR TOOLED JOINTS AS REQUIRED
 - SLAB REINF SHALL BE CHAIRED AS REQUIRED BY LOCAL BUILDING CODE

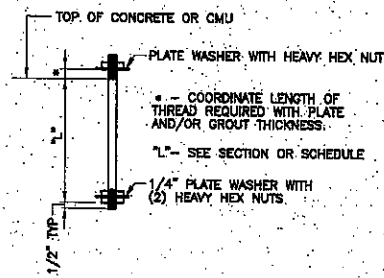


TYPICAL RECESSED SLAB DETAIL (B)
SCALE: 3/4" = 1'-0"



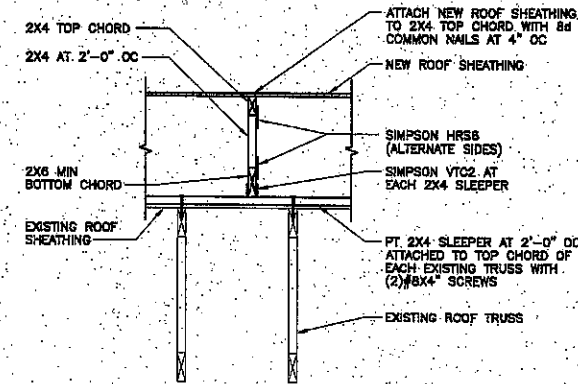
TYPICAL BEARING STUD WALL ELEVATION (D)
NOT TO SCALE

- NOTES:**
- COORDINATE ROUGH OPENING DIMENSIONS WITH ARCHITECT AND WINDOW/DOOR MANUFACTURER



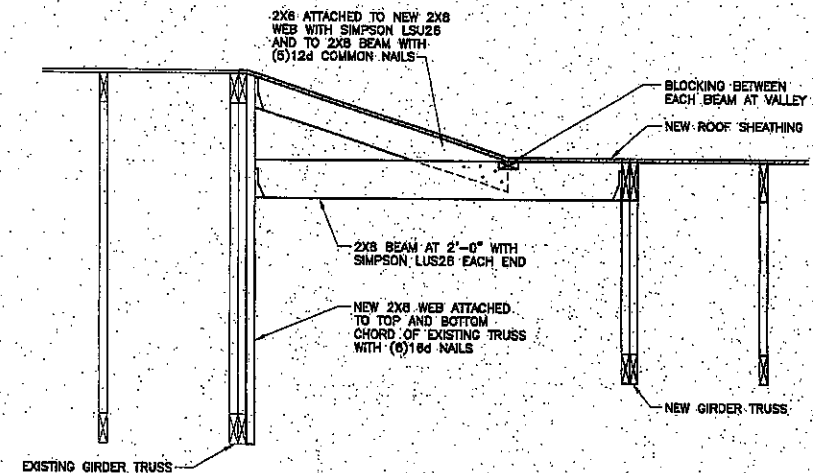
TYPICAL ANCHOR BOLT DETAIL (C)
NOT TO SCALE

- NOTES:**
- SEE SECTION OR SCHEDULE FOR ANCHOR BOLT DIAMETER AND EMBEDMENT LENGTH "L"
 - HILT ADHESIVE ANCHORING SYSTEM OFTEN MAY BE USED IN LIEU OF CAST-IN-PLACE BOLTS. CONTACT ENGINEER FOR REVISED SIZE, SPACING AND EMBEDMENT OF DRILLED AND EPOXYED ANCHOR RODS PRIOR TO CONCRETE OR GROUT PLACEMENT. THE CONTRACTOR SHALL NOT ANTICIPATE THE USE OF ADHESIVE ANCHORS PRIOR TO ENGINEER'S APPROVAL.

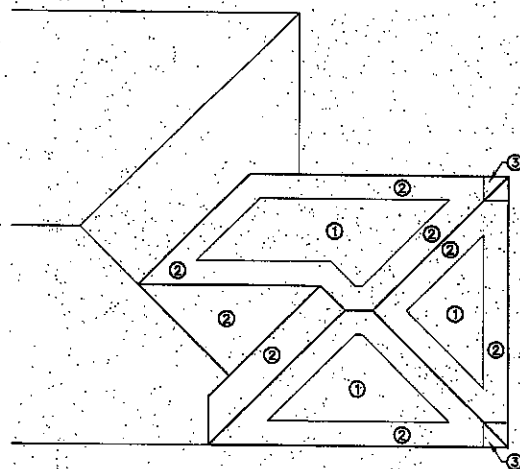


TYPICAL OVERFRAMING (E)
NOT TO SCALE

- NOTES:**
- ONLY BOTTOM CHORD REQUIRED WHERE HEIGHT PERMITS



SECTION (F)
SCALE: 3/4" = 1'-0"



ROOF ZONE DIAGRAM
N/S

ROOF COMPONENTS AND CLADDING WIND PRESSURES			
ZONE	EFFECTIVE AREA (FT ²)	WIND PRESSURES (PSF)	
		POSITIVE	SUCTION
1	10.0	33.2	-36.3
1	20.0	32.2	-34.4
1	50.0	31.1	-31.6
1	100.0	30.1	-30.1
2	10.0	33.2	-42.4
2	20.0	32.2	-40.6
2	50.0	31.1	-38.1
2	100.0	30.1	-36.3
3	10.0	33.2	-42.4
3	20.0	32.2	-40.6
3	50.0	31.1	-38.1
3	100.0	30.1	-36.3

- NOTES:**
- FOR EFFECTIVE AREAS BETWEEN THOSE GIVEN ABOVE THE LOAD MAY BE INTERPOLATED, OTHERWISE USE THE LOAD ASSOCIATED WITH THE LOWER EFFECTIVE AREA.
 - PLUS AND MINUS SIGNS SIGNIFY PRESSURES ACTING TOWARD AND AWAY FROM THE BUILDING SURFACES.
 - ZONE 2 LOCATED 3'-0" FROM EDGE OF ROOF AND EACH SIDE OF RIDGE AND HIP LINES. ZONE 3 LOCATED 3'-0" FROM ROOF CORNERS.

WALL COMPONENTS AND CLADDING WIND PRESSURES			
ZONE	EFFECTIVE AREA (FT ²)	WIND PRESSURES (PSF)	
		POSITIVE	SUCTION
INTERIOR	10.0	36.3	-39.3
INTERIOR	20.0	34.6	-37.7
INTERIOR	50.0	32.5	-35.6
INTERIOR	100.0	30.8	-33.9
END	10.0	36.3	-48.6
END	20.0	34.6	-46.4
END	50.0	32.5	-41.0
END	100.0	30.8	-37.7

- NOTES:**
- FOR EFFECTIVE AREAS BETWEEN THOSE GIVEN ABOVE THE LOAD MAY BE INTERPOLATED, OTHERWISE USE THE LOAD ASSOCIATED WITH THE LOWER EFFECTIVE AREA.
 - PLUS AND MINUS SIGNS SIGNIFY PRESSURES ACTING TOWARD AND AWAY FROM THE BUILDING SURFACES.
 - END ZONE LOCATED 3'-0" FROM EACH CORNER OF THE BUILDING.
 - WINDOWS AND DOORS SHALL BE CERTIFIED AND INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS TO WITHSTAND THE COMPONENTS AND CLADDING DESIGN PRESSURES. WINDOWS AND DOORS SHALL BE PROTECTED WITH AN IMPACT RESISTANT COVERING OR SHALL BE MANUFACTURED IMPACT RESISTANT.

NORTH FLORIDA IMAGING
2380 SOUTH THIRD STREET
JACKSONVILLE BEACH, FLORIDA

 LATITUDE 30 ENGINEERING <small>1510 B Second Street South - Jacksonville Beach, FL 32218 Phone: 904-247-1800 Fax: 904-247-1800 Project: 0823</small>	Fisher & Simmons Architects, Inc. <small>1510B Second Street South Jacksonville Beach, Florida 904-247-1800 AA C000188</small>
<small>DATE: 0823</small>	<small>1 of 2</small>

TYPICAL DETAILS S1

GENERAL NOTES

- 100 DESIGN CRITERIA
- 100.1 DESIGN BUILDING CODE: 2004 FLORIDA BUILDING CODE
- 100.2 GRAVITY LOADS:
 - DEAD LOADS
 - a. ROOF (TOP CHORD) 7 PSF
 - b. ROOF (BOTTOM CHORD) 5 PSF
 - LIVE LOADS
 - a. ROOF (TOP CHORD) 20 PSF
 - b. ATTIC WITHOUT STORAGE (BOTTOM CHORD) 10 PSF
 - c. LOBBIES & OFFICE 50 PSF
 - d. MRI EQUIPMENT 10,100 LBS
- 100.3 WIND LOADS: (2004 FBC)
 - a. BASIC WIND SPEED: 120 MPH
 - b. EXPOSURE CATEGORY: "ENCLOSED"
 - c. ENCLOSURE CLASSIFICATION: II
 - d. BUILDING CATEGORY: II
 - e. IMPORTANCE FACTOR: 1.00
 - f. INTERNAL PRESSURE COEFFICIENT: ±0.18
 - g. COMPONENTS AND CLADDING WIND PRESSURE TABLES: SEE SHEET S1
- 110 GENERAL
- 100.1 THE METHOD AND FREQUENCY OF ATTACHING MECHANICAL EQUIPMENT UNITS, ETC., TO THE STRUCTURAL ELEMENTS SHALL BE SUBJECT TO THE STRUCTURAL ENGINEER'S REVIEW AND APPROVAL.
- 100.2 THE CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS, DIMENSIONS, MEMBER SIZES, SPACING, ETC., AND SHALL IMMEDIATELY NOTIFY THE ENGINEER OF ANY AND ALL DISCREPANCIES.
- 100.3 SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR EMBEDS, OPENINGS, SLEEVES, ETC. WHICH ARE NOT SHOWN ON THE STRUCTURAL DRAWINGS.
- 100.4 THE FOUNDATIONS HAVE BEEN DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2000 PSF. THE ARCHITECT/ENGINEER SHALL BE NOTIFIED IF THE BEARING ELEVATIONS OCCUR IN DISTURBED OR UNSTABLE SOIL.
- 100.5 THE CONTRACTOR SHALL USE EXTREME CAUTION IN THE DEMOLITION OF EXISTING STRUCTURES. DEMOLITION SHALL BE PERFORMED IN SUCH A MANNER AS TO MAINTAIN THE STRUCTURAL INTEGRITY OF ALL EXISTING STRUCTURES TO REMAIN. PROVIDE SHORING AS REQUIRED.
- 100.6 COORDINATE ROUGH OPENING DIMENSIONS WITH ARCHITECT, WINDOW/DOOR MANUFACTURER AND INSTALLER. WINDOWS AND DOORS SHALL BE CERTIFIED AND INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS TO WITHSTAND THE COMPONENTS AND CLADDING WIND PRESSURES. WINDOWS AND DOORS SHALL BE PROTECTED WITH AN IMPACT RESISTANT COVERING OR SHALL BE MANUFACTURED IMPACT RESISTANT PER THE 2004 FLORIDA BUILDING CODE.
- 100.7 LATITUDE 30 ENGINEERING SHALL NOT HAVE CONTROL OR BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, PROCEDURES, OR SEQUENCES; FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR, OR ANY OTHER PERSONS, PERFORMING THE WORK OR THE FAILURE OF ANY OF THEM TO CONSTRUCT THE WORK IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS.
- 100.8 THE CONTRACTOR SHALL PROVIDE TEMPORARY BRACING, SHORING, GUYING OR OTHER MEANS TO SUPPORT THE STRUCTURAL ELEMENTS DURING ERECTION AND IN PLACE PRIOR TO THE COMPLETION OF CONSTRUCTION.
- 120 SHOP DRAWINGS
- 120.1 THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR REVIEW BY LATITUDE 30 ENGINEERING AND THE PROJECT ARCHITECT. SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL COMPONENTS INCLUDING, BUT NOT LIMITED TO THE FOLLOWING:
 - a. REINFORCEMENT DRAWINGS FOR CONCRETE AND MASONRY STEEL REINFORCEMENT INCLUDING BAR SIZES, BENDS, & LENGTHS
 - b. CONCRETE AND/OR MASONRY POST-INSTALLED ANCHORS
 - c. PRE-ENGINEERED WOOD TRUSSES
- 120.2 THE CONTRACTOR SHALL SUBMIT AT LEAST TWO (2) PRINTED COPIES OF SHOP DRAWINGS FOR REVIEW BY LATITUDE 30 ENGINEERING UPON THE COMPLETION OF THE SHOP DRAWING REVIEW. ONE (1) PRINT WILL BE RETAINED BY THE STRUCTURAL ENGINEER.
- 120.3 THE REVIEW OF SHOP DRAWINGS AND OTHER SUBMITTALS FOR THIS PROJECT IS FOR CONFORMANCE WITH THE DESIGN CONCEPT AND FOR GENERAL COMPLIANCE WITH THE INFORMATION CONTAINED IN THE CONTRACT DOCUMENTS. COMMENTS REGARDING THESE SUBMITTALS DO NOT RELIEVE THE CONTRACTOR FROM COMPLIANCE WITH THE CONTRACT DOCUMENTS. THE CONTRACTOR IS RESPONSIBLE FOR PERFORMING HIS WORK IN A SAFE AND SATISFACTORY MANNER.
- 300 REINFORCED CONCRETE
- 300.1 ALL REINFORCED CONCRETE WORK SHALL BE IN CONFORMANCE WITH THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318, LATEST EDITION) AND SPECIFICATIONS FOR STRUCTURAL CONCRETE (ACI 301, LATEST EDITION) OF THE AMERICAN CONCRETE INSTITUTE.
- 300.2 MINIMUM CONCRETE COMPRESSIVE STRENGTH REQUIRED AT 28 DAYS:
 - a. FOUNDATIONS: 3000 PSI
 - b. SLABS ON GRADE: 3000 PSI
- 300.3 ALL CONCRETE SHALL BE NORMAL WEIGHT CONCRETE (144 PCF) WITH ALL CEMENT CONFORMING TO ASTM C150, TYPE I, MAXIMUM AGGREGATE SIZE SHALL BE 1-1/2" FOR FOOTINGS AND 3/4" FOR WALLS AND SLABS, CONFORMING TO ASTM C33.
- 300.4 REINFORCEMENT
 - a. DEFORMED BARS: ASTM A615, GRADE 60
 - b. WELDED WIRE MESH: ASTM A185
- 300.5 MINIMUM COVER FOR CAST-IN-PLACE CONCRETE REINFORCEMENT UNO SHALL BE AS FOLLOWS:
 - a. FOUNDATIONS: 3"
 - b. SLABS CAST AGAINST EARTH: CENTERED
 - c. THICKENED SLAB EQUIPMENT
- 300.6 SPLICES IN REINFORCEMENT, WHERE PERMITTED, SHALL BE AS FOLLOWS:
 - a. WELDED WIRE MESH: 8" LAP
 - b. ALL OTHERS: 48" BAR DIAMETER LAP UNO


- 300.7 ALL REINFORCING STEEL SHALL BE PLACED IN ACCORDANCE WITH THE TYPICAL BAR BENDING DIAGRAMS AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. REINFORCING SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACCESSORIES DURING PLACEMENT OF CONCRETE. REINFORCING SUPPORTS FOR ALL EXPOSED CONCRETE SHALL BE GALVANIZED WITH PLASTIC COATED FEET. WELDED WIRE MESH SHALL BE CHAINED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE.
- 300.8 REINFORCING BARS SHALL NOT BE CUT TO ACCOMMODATE THE INSTALLATION OF ANCHORS, EMBEDS OR OTHER ITEMS.
- 300.9 AT CHANGES IN DIRECTION OF WALLS, BEAMS AND FOOTINGS, PROVIDE CORNER BARS TO MATCH SIZE AND SPACING OF ADJOINING REINFORCEMENT. CORNER BARS SHALL HAVE A MINIMUM 48 BAR DIAMETER LAP LENGTH WITH ADJOINING REINFORCEMENT.
- 300.10 CONTRACTION JOINTS FOR SLABS ON GRADE SHALL BE SPACED NO MORE THAN 16'-0" O.C. PANELS SHALL BE AS SQUARE AS POSSIBLE WITH A LENGTH TO WIDTH RATIO NOT TO EXCEED 1.5. VERIFY LOCATION OF CONTRACTION JOINTS IN EXPOSED SLABS WITH ARCHITECT.
- 300.11 PRIOR TO CONCRETE PLACEMENT, THE CONTRACTOR SHALL SUBMIT A CONCRETE MIX DESIGN FOR REVIEW BY LATITUDE 30 ENGINEERING. THE INFORMATION SHALL INCLUDE:
 - a. INTENDED USAGE AND LOCATION FOR EACH TYPE
 - b. MIX DESIGN INCLUDING MATERIAL TYPES AND AMOUNTS
 - c. WATER CEMENT RATIO BY WEIGHT
 - d. SLUMP RANGE
 - e. AIR CONTENT
 - f. STRENGTH TEST DATA REQUIRED TO ESTABLISH MIX DESIGN
- 300.12 PROVIDE (4) CONCRETE TEST CYLINDERS FOR EACH 40 CY OF CONCRETE PLACED OR FRACTION THEREOF PER DAY. A QUALIFIED TESTING LABORATORY SHALL PROVIDE A COPY OF THE COMPRESSIVE STRENGTH TEST RESULTS TO THE ARCHITECT FOR REVIEW.
- 350 CONCRETE/MASONRY ANCHORS
- 350.1 ALL ADHESIVE STUD ANCHORS SHALL BE "HILTI HIT-HY 150 ADHESIVE CONCRETE ANCHORS" AS MANUFACTURED BY HILTI FASTENING SYSTEMS, INC. (OR APPROVED EQUAL).
- 350.2 ALL EXPANSION STUD ANCHORS SHALL BE "HILTI KWIK-BOLT II EXPANSION CONCRETE ANCHORS" AS MANUFACTURED BY HILTI FASTENING SYSTEMS, INC. (OR APPROVED EQUAL).
- 350.3 THE "HAS ANCHOR ROD" SHALL CONFORM TO ASTM A36 STEEL. THE "HAS SUPER ANCHOR ROD" SHALL CONFORM TO ASTM A193 STEEL. THE "HAS STANDARD NUT" SHALL CONFORM TO ASTM A563, GRADE A. THE "HAS SUPER ANCHOR ROD NUT" SHALL CONFORM TO ASTM A 563, GRADE H.
- 350.4 THE "KWIK-BOLT II EXPANSION ANCHORS" STUD SHALL CONFORM TO ASTM A510 OR ASTM A108 STEEL AND THE NUT SHALL CONFORM TO ASTM A563, GRADE A.
- 350.5 THE INSTALLATION OF POST INSTALLED ANCHORS SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDED PROCEDURES.
- 610 STRUCTURAL LUMBER
- 610.1 ALL STRUCTURAL LUMBER SHALL CONFORM TO THE MOST CURRENT APPLICABLE SPECIFICATIONS OF THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION.
- 610.2 ALL STRUCTURAL LUMBER SHALL BE NO. 2 GRADE SOUTHERN YELLOW PINE OR BETTER UNO WITH ALLOWABLE MATERIAL STRESSES IN ACCORDANCE WITH THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION BY THE AMERICAN FOREST & PAPER ASSOCIATION.
- 610.3 ALL LUMBER SHALL COMPLY WITH PS 20 "AMERICAN SOFTWOOD LUMBER STANDARD" AND WITH THE APPLICABLE RULE OF INSPECTION AGENCIES CERTIFIED BY AMERICAN LUMBER STANDARD. FACTORY MARK EACH PIECE OF LUMBER WITH GRADE STAMP OF INSPECTION AGENCY EVIDENCING COMPLIANCE WITH GRADING RULE REQUIREMENTS.
- 610.4 PROVIDE NAILING PATTERN IN COMPLIANCE WITH FBC RECOMMENDED FASTENING SCHEDULE WHEN JOINING TWO OR MORE FRAMING MEMBERS.
- 610.5 ALL METAL CONNECTORS SHALL BE MANUFACTURED BY THE SIMPSON STRONG-TIE COMPANY AND SHALL BE EITHER HOT-DIP GALVANIZED OR STAINLESS STEEL. INSTALL METAL CONNECTORS PER THE MANUFACTURER'S SPECIFICATIONS FOR THE MAXIMUM NAILING QUANTITY AND CAPACITY. ONLY HOT-DIP GALVANIZED FASTENERS SHALL BE USED WITH GALVANIZED CONNECTORS AND ONLY STAINLESS STEEL FASTENERS SHALL BE USED WITH STAINLESS STEEL CONNECTORS.
- 610.6 ALL WOOD JOISTS OR HEADERS WHICH FRAME INTO BEAMS SHALL BE SUPPORTED BY SIMPSON LUS HANGERS WITH THE SAME WIDTH AND DEPTH OF THE MEMBER UNO. USE HANGERS WITH CONCEALED FLANGES WHERE THE CONNECTOR CANNOT BE HIDDEN BY WOOD TRIM OR THE SUPPORT MEMBER IS WIDER THAN THE STANDARD FLANGE.
- 610.7 BOLTED CONNECTIONS SHALL USE ASTM A36 BOLTS WITH 2 WASHERS PER BOLT UNO. ALL BOLTED CONNECTIONS EXPOSED TO WEATHER SHALL USE HOT-DIP GALVANIZED OR STAINLESS STEEL BOLTS AND HARDWARE.
- 610.8 LOAD BEARING STUD WALLS SHALL BE CONTINUOUSLY BRIDGED AT MID-HEIGHT AND UNSUPPORTED PLYWOOD WALL SHEATHING JOINTS WITH SOLID WOOD BLOCKING.
- 610.9 NO CUTS, HOLES, OR COPES REQUIRED FOR OTHER TRADES IN STRUCTURAL WOOD FRAMING SHALL BE PERMITTED WITHOUT PRIOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER AND ARCHITECT.
- 610.10 ONE ROW OF BRIDGING SHALL BE PROVIDED AT CENTER LINE OF JOIST SPAN OR AS INDICATED ON THE DRAWINGS.
- 610.11 PRESSURE TREAT WITH WATER-BORNE PRESERVATIVES ALL LUMBER IN CONTACT WITH CONCRETE OR MASONRY OR LUMBER WHICH MAY BE EXPOSED TO WEATHER OR EARTH. PRESURE TREATMENT SHALL COMPLY WITH REQUIREMENTS OF AWPA STANDARDS C2 AND LP-22.
- 610.12 ALL METAL FASTENERS, CONNECTORS, ANCHORS AND HARDWARE THAT ARE IN CONTACT WITH PRESSURE TREATED WOOD SHALL BE STAINLESS STEEL (A304 OR A316) OR HOT-DIP GALVANIZED. CONTRACTOR SHALL VERIFY COATING REQUIREMENTS WITH TREATED WOOD AND METAL CONNECTOR MANUFACTURER.
- 610.13 ALTERNATE CONNECTION DETAILS MAY BE PROPOSED TO THE STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL. HOWEVER, THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN OF THE ALTERNATE DETAILS.

- 630 PLYWOOD
- 630.1 ALL PLYWOOD SHALL CONFORM TO THE MOST CURRENT APPLICABLE SPECIFICATION AND SUPPLEMENTS OF THE AMERICAN PLYWOOD ASSOCIATION (APA). INSTALLATION OF THE PANELS SHALL BE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE APA. EACH PANEL SHALL BE IDENTIFIED WITH THE APPROPRIATE TRADEMARK OF THE APA AND SHALL MEET THE REQUIREMENTS OF THE LATEST EDITION OF THE APA PERFORMANCE STANDARDS.
- 630.2 ALL PANELS WHICH HAVE ANY EDGE OR FACE PERMANENTLY EXPOSED TO THE WEATHER SHALL BE CLASSED EXTERIOR AND PRESSURE TREATED, EXCEPT OPEN SOFFITS OR ROOF SHEATHING EXPOSED ON THE UNDERSIDE MAY BE CLASSED EXPOSURE 1.
- 630.3 ALL PLYWOOD PANEL END JOINTS SHALL OCCUR OVER SUPPORTS AND SHALL BE STAGGERED ONE HALF PANEL LENGTH FROM ADJACENT PANELS. PROVIDE 1/8" INCH SPACE AT PANEL ENDS AND EDGES.
- 630.4 ALL PLYWOOD ROOF SHEATHING SHALL BE 5/8"-INCH APA RATED EXPOSURE 1 SHEATHING. INSTALL WITH LONG DIMENSION OF PANEL ACROSS THREE OR MORE SUPPORTS. ALL PANELS SHALL BE NAILED AS INDICATED ON PLAN.
- 630.5 ALL WALL SHEATHING SHALL BE 1/2"-INCH APA RATED EXPOSURE 1 SHEATHING. INSTALL WITH LONG DIMENSION OF PANEL ACROSS THREE OR MORE SUPPORTS. ALL PANELS SHALL BE NAILED AS INDICATED ON PLAN.
- 635 WOOD TRUSSES
- 635.1 WOOD TRUSSES SHALL CONFORM TO THE MOST CURRENT APPLICABLE VERSION OF THE DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE CONNECTED WOOD ROOF AND FLOOR TRUSSES, BY THE TRUSS PLATE INSTITUTE, INC. AND THE NATIONAL DESIGN SPECIFICATIONS FOR STRESS GRADE LUMBER AND ITS FASTENING, BY THE NATIONAL FOREST PRODUCTS ASSOCIATION. ADDITIONAL DESIGN AND DETAILING REQUIREMENTS SET FORTH IN THIS SECTION SHALL BE PROVIDED BY THE TRUSS MANUFACTURER.
- 635.2 THE TRUSS MANUFACTURER SHALL CLOUD ON THE SHOP DRAWING ERECTION PLANS ALL AREAS WHERE THE TRUSS LAYOUT VARIES FROM THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL ANTICIPATE MODIFICATIONS TO THE STRUCTURAL DRAWINGS DUE TO A REVISED TRUSS LAYOUT.
- 635.3 WOOD ROOF TRUSSES SHALL BE DESIGNED BY THE TRUSS MANUFACTURER TO SUPPORT THE LOADS INDICATED IN THE DESIGN CRITERIA SECTION OF THE GENERAL NOTES AT THE DEPTH AND SPACING INDICATED ON THE DRAWINGS. PATTERN LOADING, UNBALANCED LOADING AND LOAD COMBINATIONS SHALL BE CONSIDERED IN THE DESIGN. WOOD TRUSSES SHALL BE DESIGNED WITH AT LEAST ONE HORIZONTAL ROLLER CONNECTION PER SPAN SO THAT NO HORIZONTAL REACTIONS ARE INDUCED ON THE SUPPORTS DUE TO DEAD AND LIVE LOADS.
- 635.4 THE DESIGN OF THE ROOF TRUSSES SHALL SATISFY THE FOLLOWING DEFLECTION CRITERIA:
 - a. THE DEFLECTION DUE TO LIVE LOADS SHALL NOT EXCEED THE SPAN LENGTH IN INCHES DIVIDED BY 480.
 - b. THE MAXIMUM DEFLECTION DUE TO THE TOTAL LOADS SHALL NOT EXCEED THE SPAN LENGTH IN INCHES DIVIDED BY 360.
- 635.5 THE WOOD TRUSS MANUFACTURER SHALL SPECIFY ALL BRACING AT TOP AND BOTTOM CHORDS REQUIRED TO STABILIZE THE ROOF STRUCTURE DURING AND AFTER CONSTRUCTION. IN ADDITION TO THE BRACING INDICATED ON THE STRUCTURAL DRAWINGS, BRIDGING SHALL BE NOMINAL 2 INCHES IN THICKNESS MINIMUM. ALL SUPPORTS, BRACING, AND CONNECTIONS REQUIRED FOR THE INSTALLATION OF THE WOOD TRUSS SYSTEM THAT ARE NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED AND SPECIFIED BY THE WOOD TRUSS FABRICATOR AND ARE SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER.
- 635.6 THE WOOD TRUSS MANUFACTURER SHALL SUBMIT STRUCTURAL CALCULATIONS AND SHOP DRAWINGS SIGNED, SEALED AND DATED BY A DESIGNATED PROFESSIONAL ENGINEER LICENSED TO PRACTICE IN THE STATE OF FLORIDA FOR THE "TRUSS SYSTEM" WHICH SHALL INCLUDE, BUT NOT LIMITED TO:
 - a. ALL TRUSS TYPES, WHICH INDICATE TRUSS CAPACITIES AND DEFLECTIONS.
 - b. MAXIMUM UPLIFT FORCES.
 - c. TRUSS TO TRUSS CONNECTIONS INCLUDING THOSE FOR OVERFRAMING, VALLEY OR PIGGYBACK TRUSSES.
 - d. TRUSS HANGER CONNECTOR TYPES FOR SUPPORT CONDITIONS OTHER THAN BOTTOM CHORD BEARING.
 - e. PERMANENT AND TEMPORARY BRIDGING SIZES, CONNECTIONS AND LOCATIONS.
- 635.7 TRUSS ERECTION SHALL BE IN ACCORDANCE WITH TRUSS PLATE INSTITUTE RECOMMENDATIONS.
- 635.8 BOTTOM CHORD BEARING PARALLEL CHORD TRUSSES SHALL BE CLEARLY MARKED IN A MANNER WHICH WILL AVOID INVERTED INSTALLATION IN ACCORDANCE WITH THE TRUSS PLATE INSTITUTE, "NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION," ANS/TP1 - LATEST EDITION.
- 635.9 TEMPORARY TRUSS BRACING SHALL BE INSTALLED IN ACCORDANCE WITH "RECOMMENDED DESIGN SPECIFICATIONS FOR TEMPORARY BRACING OF METAL PLATE CONNECTED WOOD TRUSSES" (OSB-88) AND "COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES" (HB-91). INSTALL ALL WEB BRACING REQUIRED BY THE TRUSS DESIGNER. TEMPORARY BOTTOM CHORD WEB BRACING SHALL REMAIN PERMANENTLY IN PLACE. THE BOTTOM CHORD BRACING SHALL NOT EXCEED 10' FOR TRUSSES WHERE NO SHEATHING IS ATTACHED TO THE TRUSS BOTTOM CHORD OR WITH TRUSS BOTTOM FILLER.
- 635.10 THE WOOD TRUSS MANUFACTURER SHALL COORDINATE TRUSS CONFIGURATIONS AND LOCATIONS WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS SO AS TO AVOID CONFLICTS WITH EQUIPMENT, DUCTS, STACKS, PIPES, STAIRS, ETC.
- 635.11 THE WOOD TRUSS MANUFACTURER SHALL REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR COORDINATION OF EQUIPMENT, PIPING AND/OR SPECIAL CONCENTRATED LOADS SUPPORTED BY THE WOOD TRUSSES NOT INDICATED ON THE STRUCTURAL DRAWINGS.

STRUCTURAL ABBREVIATIONS:

ARCH	ARCHITECT(URAL)	NS	NEAR SIDE
CJ	CONTRACTION JOINT	NO./	NUMBER
CMU	CONCRETE MASONRY UNIT	NTS	NOT TO SCALE
C	CENTER LINE	NW	NORMAL WEIGHT
CLR	CLEAR	OC	ON CENTER
CONT	CONTINUOUS	OPP	OPPOSITE
COORD	COORDINATE	PERP	PERPENDICULAR
DIA	DIAMETER	PLF	POUNDS PER LINEAR FOOT
DBL	DOUBLE	PSF	POUNDS PER SQUARE FOOT
EJ	EACH FACE	PSP	2.0E PARALLEL
EF	EXPANSION JOINT	PSI	POUNDS PER SQUARE INCH
ED	EDGE OF DECK	PT	PRESSURE TREATED/POST TENSION
EDS	EDGE OF SLAB	REINF	REINFORCED/REINFORCING
EL/ELEV	ELEVATION	REF	REFERENCE
EMBED	EMBEDMENT/EMBEDDED	SF	SQUARE FEET
EQ	EQUAL	SIM	SIMILAR
ETC	ETCETERA	SPEC	SPECIFICATION
EXIST	EXISTING	SPF	SPRUCE-PINE-FIR
FS	FAR SIDE	SS	STAINLESS STEEL
GC	GENERAL CONTRACTOR	STD	STANDARD
GAGE	GAGE	SYM	SYMMETRICAL
GALV	HOT-DIPPED GALVANIZED	SYN	SOUTHERN YELLOW PINE
HORIZ	HORIZONTAL	TD	TURNDOWN/SLAB EDGE
HK	HOOK	TOB	TOP OF BEAM
K	KIPS	TOC	TOP OF CONCRETE
KLF	KIPS PER LINEAR FOOT	TOF	TOP OF FOOTING
KSF	KIPS PER SQUARE FOOT	TOJ	TOP OF JOIST
KSI	KIPS PER SQUARE INCH	TOS	TOP OF STEEL
LBS	POUNDS	TOW	TOP OF WALL
LLH	LONG LEG HORIZONTAL	TS	TUBULAR STEEL
LLV	LONG LEG VERTICAL	TYP	TYPICAL
LW	1.5E MICROLAM	UNO	UNLESS NOTED OTHERWISE
LV	LIGHTWEIGHT	VCL	VERTICAL CONTROL JOINT
MEM	METAL BUILDING MANUFACTURER	VERT	VERTICAL
MC	MOMENT CONNECTION	WP	WORK POINT
MANUF	MANUFACTURER(ING)	WWM	WELDED WIRE MESH
MAX	MAXIMUM	XS	EXTRA STRONG
MECH	MECHANICAL	XXS	DOUBLE EXTRA STRONG
MIN	MINIMUM		
MISC	MISCELLANEOUS		
MO	MASONRY OPENING		

NORTH FLORIDA IMAGING
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JACKSONVILLE BEACH, FLORIDA

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GENERAL NOTES **S2**