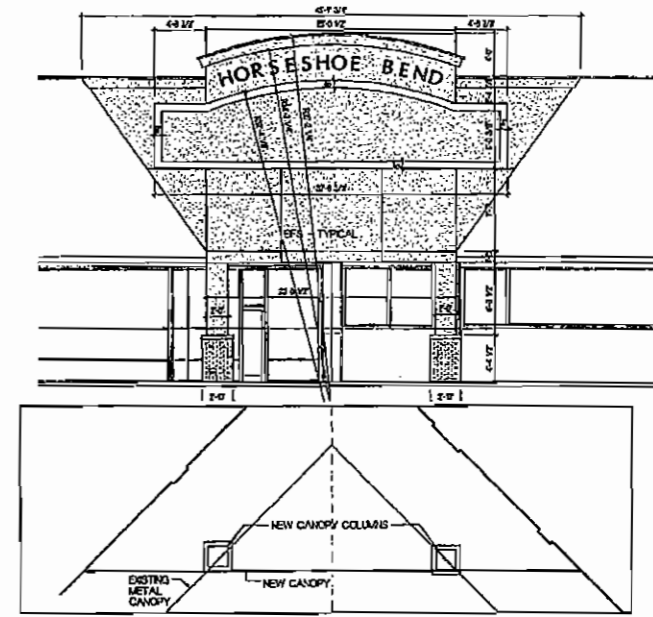
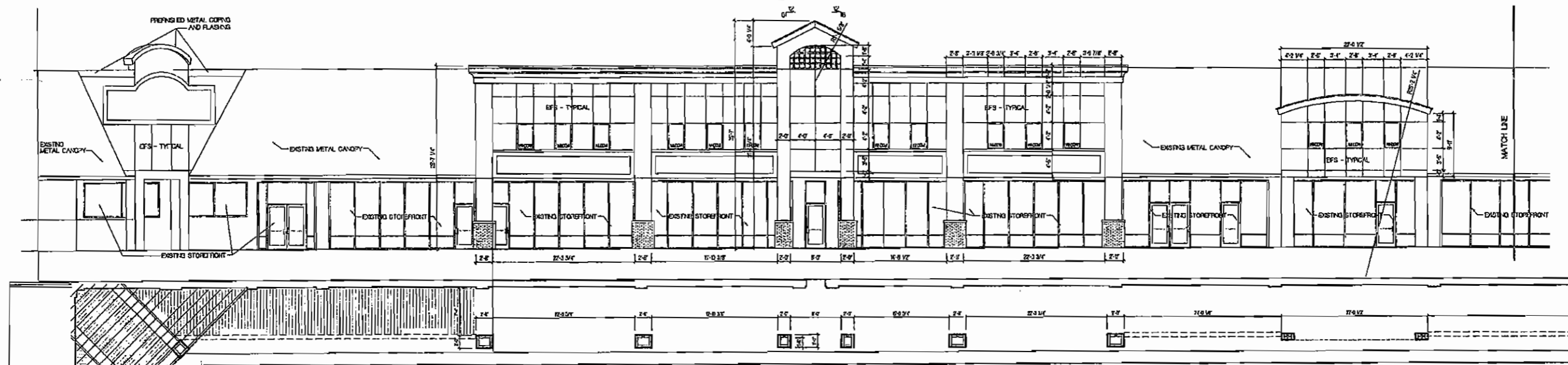


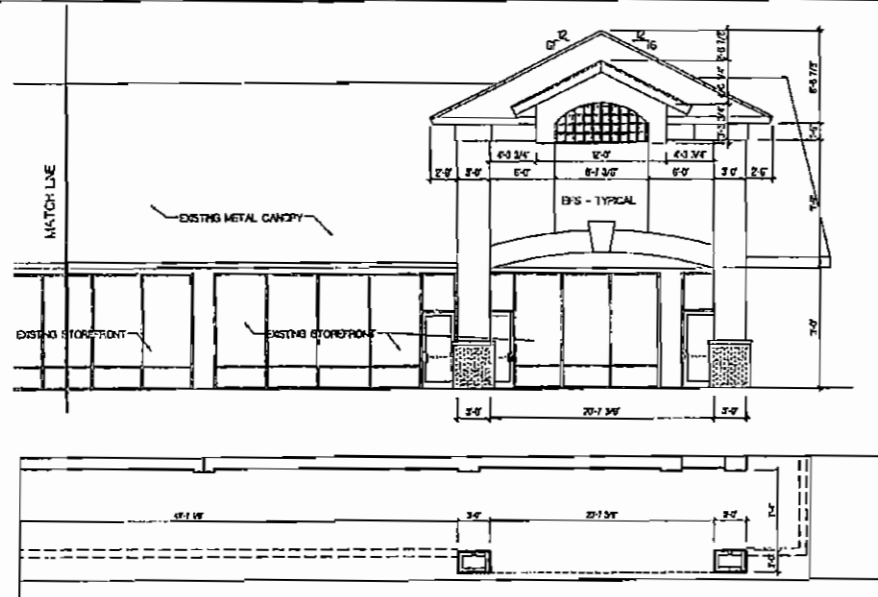
ELEVATIONS & PLAN ①  
1/8" = 1'-0"



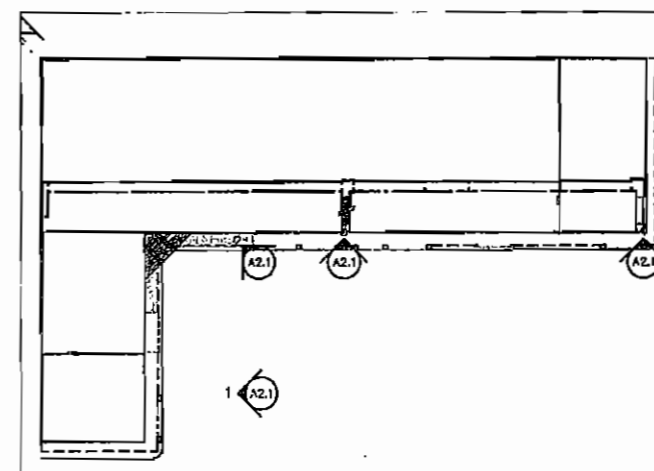
ELEVATIONS & PLAN ②  
1/8" = 1'-0"



ELEVATIONS & PLAN ③  
1/8" = 1'-0"



ELEVATIONS & PLAN ④  
1/8" = 1'-0"



KEY PLAN ⑤  
NOT TO SCALE

NOTES:  
 SCOPE OF WORK:  
 CONTRACTOR SHALL REMOVE SECTIONS OF EXISTING STANDING SEAM METAL ROOF, ROOFING AND LIGHTS IN AREAS AS SHOWN ON THE DRAWINGS. AREAS OF EXISTING CANOPY ROOF NOT REMOVED ARE TO BE CLEANED AND PAINTED.  
 REPLACE ANY DAMAGED METAL ROOFING, COPING OR FLASHING.  
 NEW METAL FRAMING, AS SHOWN, SHALL BE INSTALLED AND NEW EXTERIOR FINISHES, INCLUDING EPS AND STONE, IN CONFIGURATIONS SHOWN AS ON THE DRAWINGS, ARE TO BE INSTALLED.  
 ALL EXISTING ROOFING IS TO BE REPLACED WITH NEW CAN LIGHTS AS SELECTED BY THE OWNER, ARE TO BE INSTALLED ON THE ENTIRE BUILDING.  
 ONLY IN AREAS WHERE THERE ARE EXISTING OCCUPIED SECOND LEVEL OCCUPIED SECOND LEVEL OCCUPIED SPACES, ARE THE FLOOR ASSEMBLIES AND WALLS TO BE EXTENDED INTO THE CANOPY. NO NEW SECOND LEVEL SPACES ARE TO BE CREATED UNDER THIS CONTRACT.  
 ALL EXISTING TENANT OPERATIONS AND RATED ASSEMBLIES CURRENTLY IN PLACE AND WITHIN THE SCOPE OF WORK ARE TO BE EXTENDED TO THE NEW CANOPY AND SEALED TO NEW CONSTRUCTION AS REQUIRED TO MAINTAIN REQUIRED SEPARATIONS.  
 NEW CANOPIES ARE TO BE OPEN TO INTERIOR, NO CONCEALED CANOPY SPACES ARE TO BE CREATED.

EXTERIOR ELEVATIONS  
 AND PLANS  
 DATE: 1.20.06  
 JOB No. 605

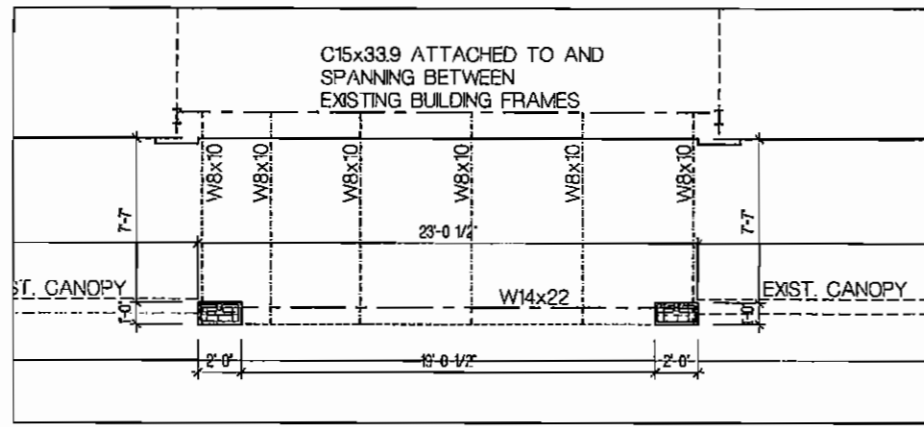


OSCAR DAYAN  
 200 Laurel Mill Court, Roswell, Georgia 30075  
 770/541-0324

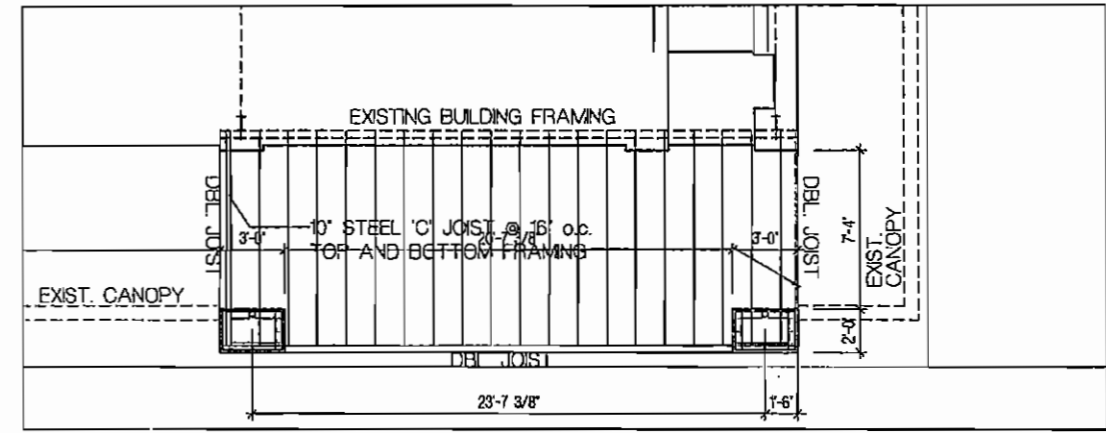
RENOVATION OF EXISTING  
 HORSESHOE BEND PLAZA  
 POWDER SPRINGS ROAD, COBB COUNTY, GEORGIA  
 BRUNO ARCHITECTURAL

TO ORDER PRINTS GO TO WWW.LDIREPRO.COM

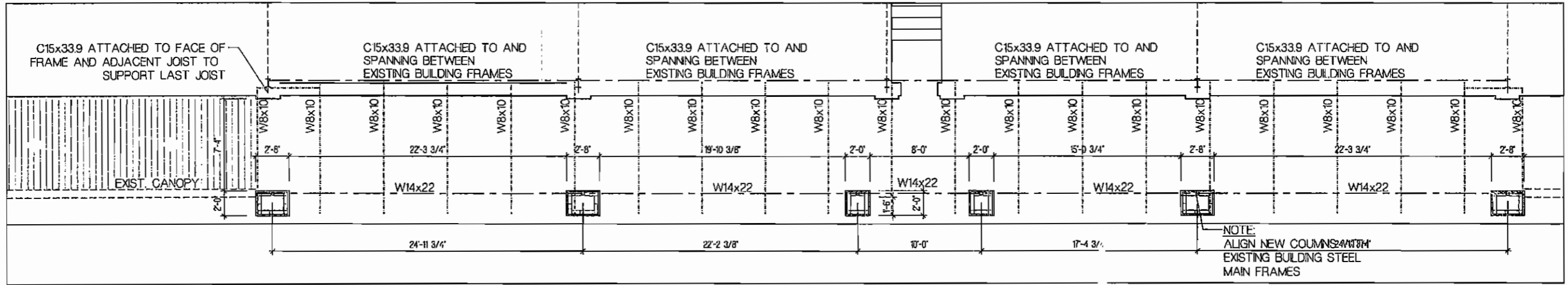
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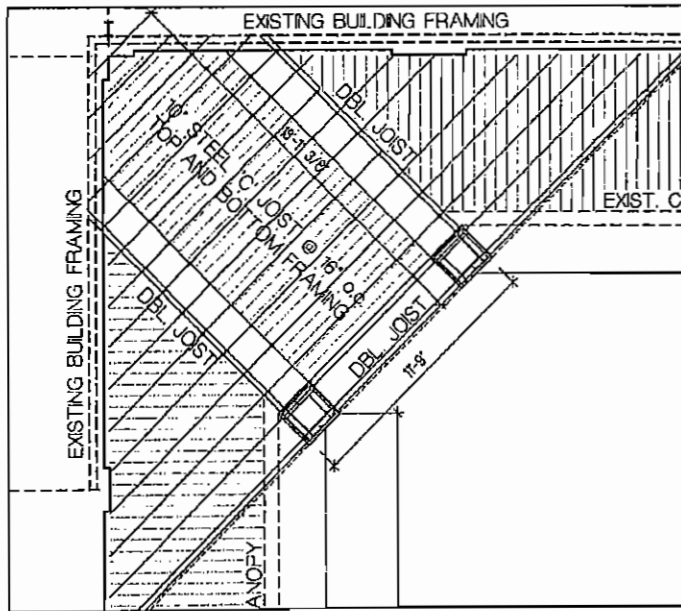
FRAMING PLAN ②  
1/8" = 1'-0"



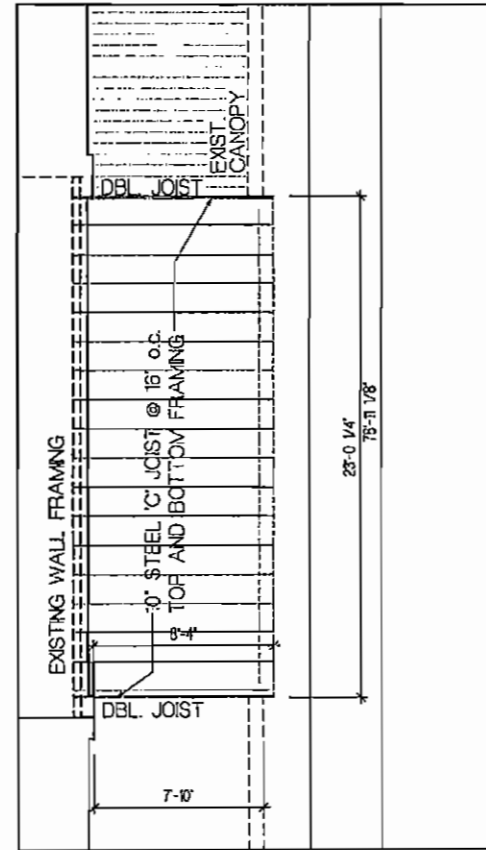
FRAMING PLAN ①  
1/8" = 1'-0"



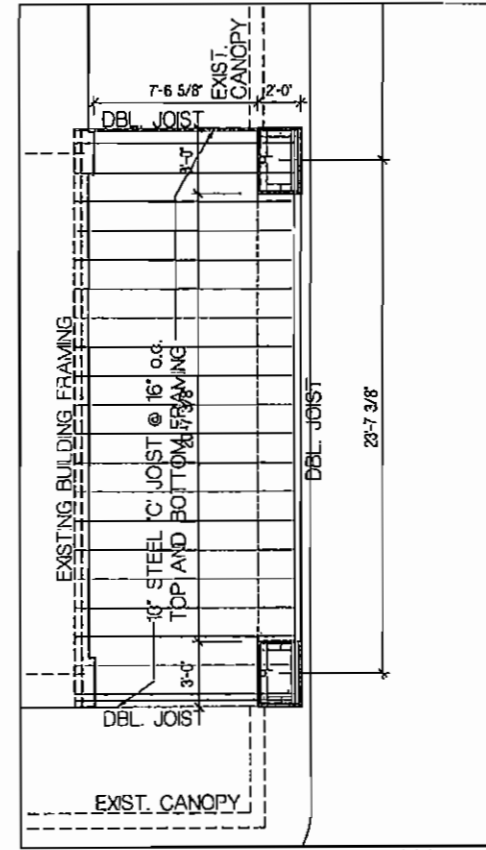
FRAMING PLAN ③  
1/8" = 1'-0"



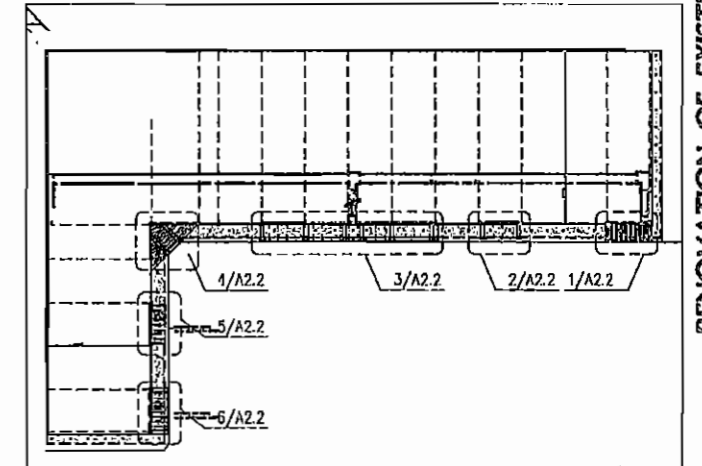
FRAMING PLAN ④  
1/8" = 1'-0"



FRAMING PLAN ⑤  
1/8" = 1'-0"



FRAMING PLAN ⑥  
1/8" = 1'-0"



KEY PLAN  
NOT TO SCALE

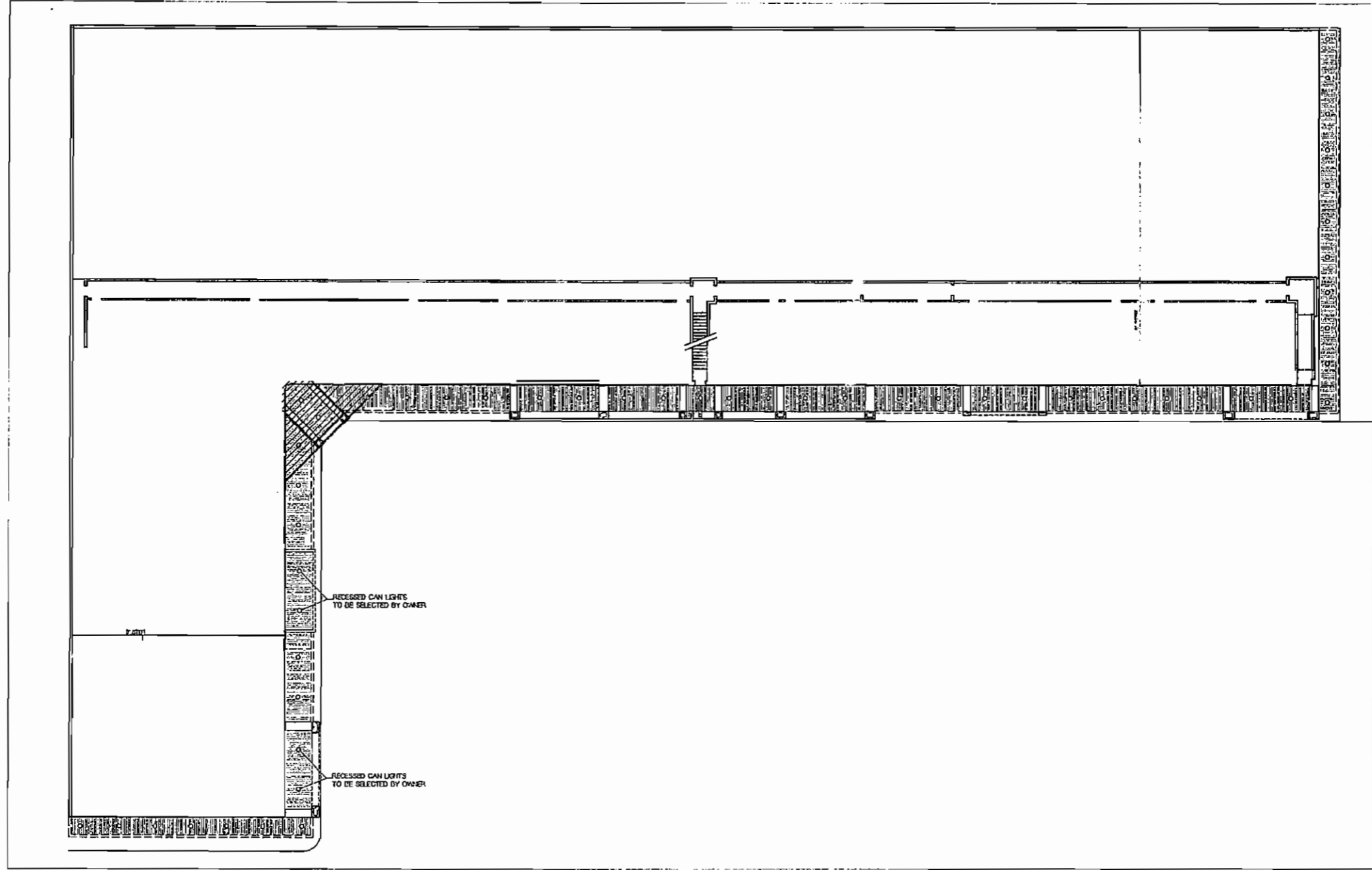


OSCAR DAYANI  
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770/541-0324

BRUNO ARCHITECTURAL  
RENOVATION OF EXISTING HORSESHOE BEND PLAZA  
POWDER SPRINGS ROAD, COBB COUNTY, GEORGIA

FRAMING PLANS AND DETAILS  
DATE 1.20.06  
For No. 605  
A2.2  
07

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**SOFFIT PLAN** ①  
 1/16" = 1'-0"



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 770/541-0324

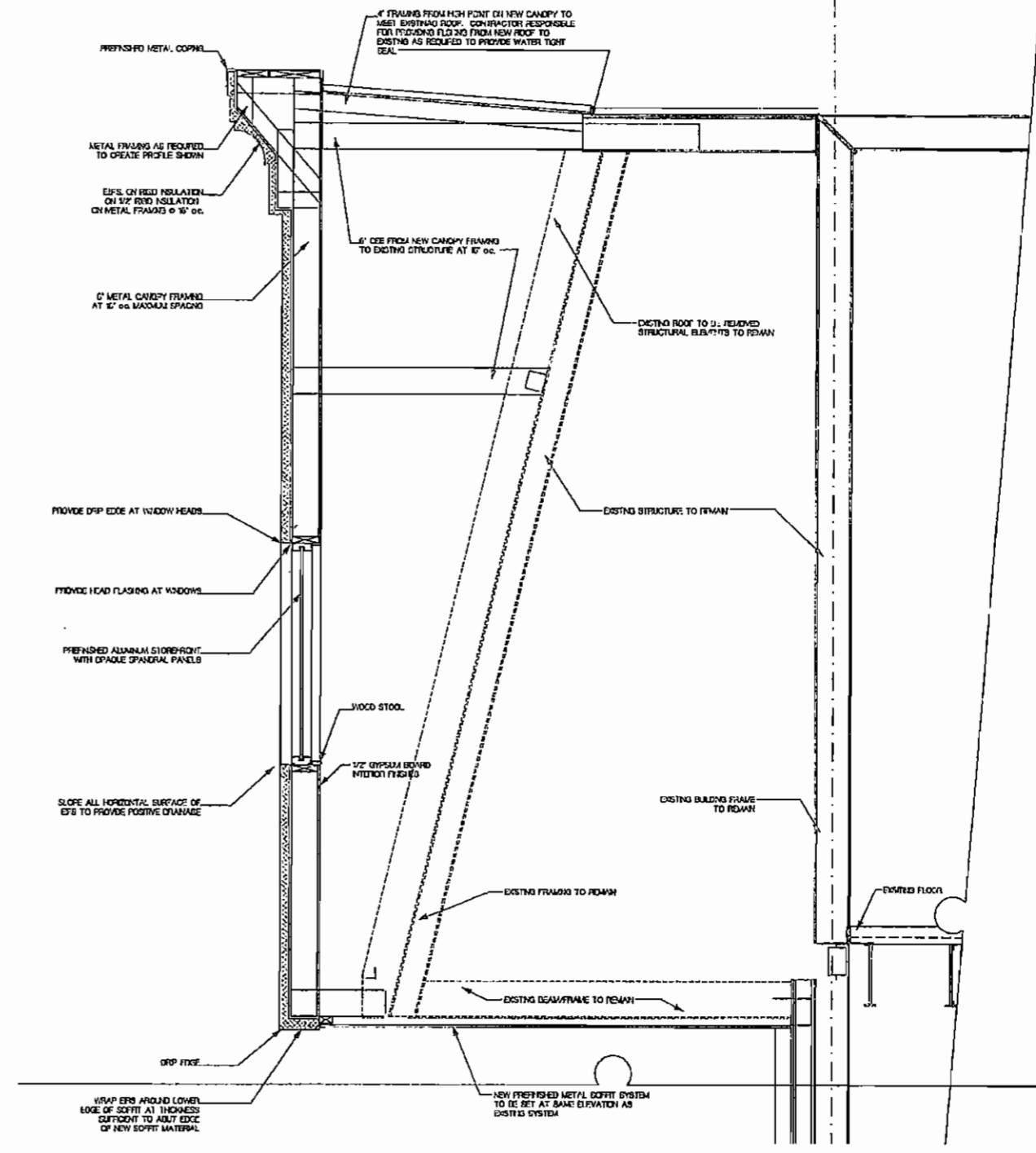
**BRUNO ARCHITECTURAL**  
 200 Powder Springs Road, Cobb County, Georgia

RENOVATION OF EXISTING  
 HORSESHOE BEND PLAZA

REFLECTED  
 SOFFIT PLANS  
 DATE 1.20.06

JOB No.  
 805  
 A2  
 of 5





**CANOPY SECTION ①**  
3/4" = 1'-0"

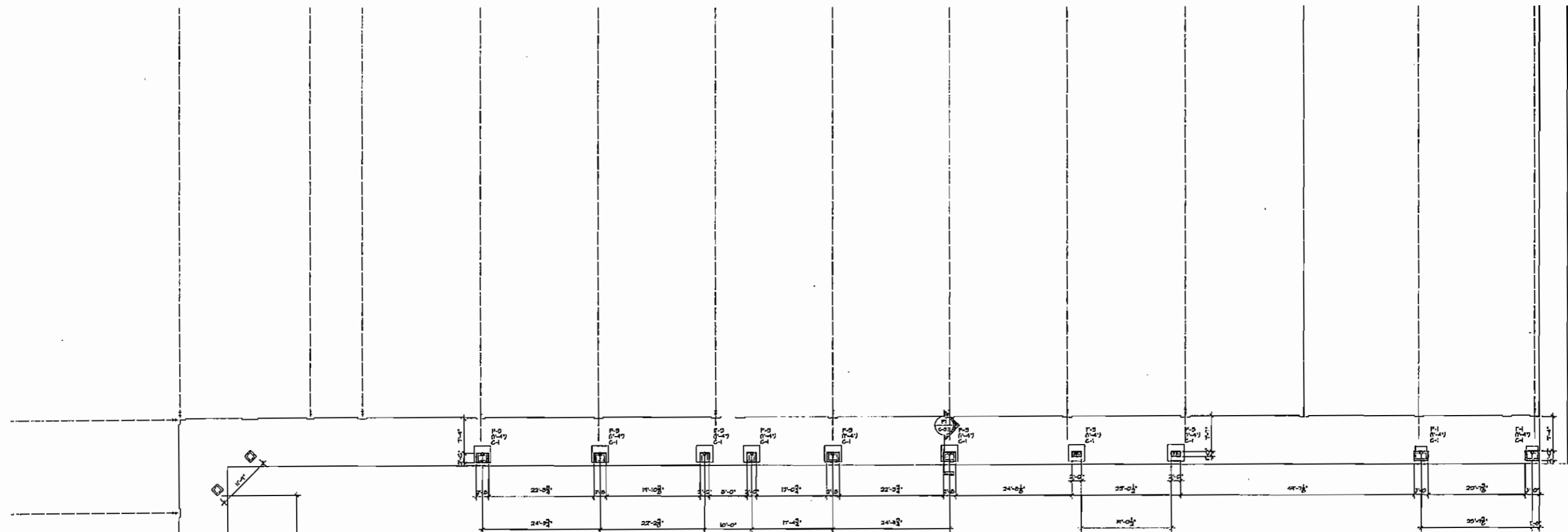


OSCAR DAYAN  
200 Laurel Hill Court, Roswell, Georgia 30076  
770/541-0324

RENOVATION OF EXISTING  
HORSESHOE BEND PLAZA  
POWDER SPRINGS ROAD, COBB COUNTY, GEORGIA  
BRUNO ARCHITECTURAL

REV 2/27/06  
CANOPY SECTION  
DATE 1/20/06

JOB No.  
605  
A6.2  
OF 5



# FOUNDATION PLAN

SCALE: 1/8" = 1'-0"

- NOTES:**
- FOOTING ELEVATIONS INDICATED AS (F-X) ARE REFERENCED FROM FINISH FLOOR ELEVATION OF 0'-0" (ASSUMED).
  - FOOTING MARKS ARE AS FOLLOWS:  
 F-X FOOTING REFERENCE, SEE FOOTING SCHEDULE FOR SIZE AND REINFORCING.  
 (F-X) TOP OF FOOTING ELEVATION, SEE NOTE 1.  
 C-X COLUMN REFERENCE, SEE COLUMN SCHEDULE FOR SIZE.

## COLUMN SCHEDULE

MARK	SIZE	WAGE E	CAP E
C-1	3'-0" x 3'-0" HT.	SEE DETAIL 3-4/5-01	SEE DETAIL 3-4/5-01

## CONCRETE

- ALL CONCRETE WORK INCLUDING FORMING, REINFORCING, MIXING, PLACING AND CURING SHALL BE DONE IN ACCORDANCE WITH THE ACI MANUAL OF CONCRETE PRACTICE INCLUDING "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", ACI 318 AND "SPECIFICATIONS FOR STRUCTURAL CONCRETE", ACI 308.
- ALL CONCRETE SHALL ATTAIN A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 1% = 5000 PSI FOR FOUNDATIONS AND 1% = 4000 PSI FOR DETACHED WALLS AND RETAINING WALLS SLABS AT 28 DAYS.
- CONCRETE SHALL BE VIBRATED INTO FORMS WHILE PLACING, WITHOUT OVER-VIBRATING. REINFORCING SHALL BE PROTECTED BY CONCRETE OF THICKNESS AS FOLLOWS:
 

FOOTINGS	3"
PIERS	1/2" TO TIES
BASEMENT WALLS:	
EXTERIOR FACE	2"
INTERIOR FACE	1"
OTHER FOUNDATION	2" BOTH FACES
COLUMNS	1/2" TO TIES
EXPOSED EXTERIOR CONCRETE	2"
SLAB ON GRADE	1/2" FROM TOP
- SLAB-ON-GRADE SHALL BE PLACED IN CONTINUOUS STRIPS PER ACI RECOMMENDATIONS.
- COORDINATE CONCRETE WORK WITH THAT OF OTHER TRADES TO ALLOW FOR SETTING OF SLEEVES, ACCESSORIES, ETC.
- ALL ANCHOR RODS SHALL BE IN PLACE PRIOR TO POURING OF CONCRETE.
- CONCRETE TEST REPORTS SHALL BE MADE AVAILABLE AT JOB SITE FOR REVIEW BY THE INSPECTOR.
- DESIGN OF CONCRETE STRUCTURAL ELEMENTS INCLUDING WALLS, FORMED SLABS, BEAMS AND COLUMNS IS IN ACCORDANCE WITH ACI 318-19.

## FOOTING SCHEDULE

MARK	SIZE	THK.	REINFORCING	REMARKS
F-1	3'-0" x 3'-0"	12"	5 #4 EACH MAT	SEE DETAIL F-5/5-01
F-2	NOT USED			
F-3	3'-0" x 3'-0"	12"	4 #4 EACH MAT	SEE DETAIL F-3/3-01 FOR BAR # & ANCHOR BOLTS.

- PROVIDE 3" COVER (MIN) FOR ALL REINFORCING.

## STRUCTURAL NOTES:

**BUILDING CODES USED FOR DESIGN**  
 DESIGN IS IN COMPLIANCE WITH 2000 STANDARD BUILDING CODE & GEORGIA STATE AMENDMENTS.

### DESIGN LOADS

- ROOF LIVE LOAD = 20 PSF.
- ROOF DEAD LOAD = 20 PSF.
- BASIC WIND SPEED = 90 MPH. (3 SEC. GUST)  
 WIND IMPORTANCE FACTOR,  $I_w = 1.00$   
 WIND EXPOSURE CATEGORY = B

### NEW CONSTRUCTION

- THE CONTRACTOR SHALL FOLLOW WRITTEN DIMENSIONS ONLY. DO NOT SCALE DRAWINGS.
- ALL DETAILS AND SECTIONS SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL BE CONSIDERED TO APPLY AT ANY SIMILAR SITUATION ELSEWHERE ON THE JOB EXCEPT WHERE A DIFFERENT DETAIL OR SECTION IS SHOWN.
- THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. WHERE DISCREPANCIES OCCUR, IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE ARCHITECT PRIOR TO CONSTRUCTION.
- THE STEEL FRAMING MEMBERS SHOWN RELY ON BUILDING COMPONENTS OTHER THAN STRUCTURAL STEEL FOR FINAL STRUCTURAL STABILITY (PREVIOUSLY REFERRED TO AS A NON-SELF-SUPPORTING STEEL FRAME BY THE AISC "CODE OF STANDARD PRACTICES"). THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND PROVISION OF ANY AND ALL TEMPORARY BRACINGS AND SHORING AGAINST WIND, ERECTION AND ALL CONSTRUCTION LOADS UNTIL ALL ELEMENTS, MEMBERS AND CONNECTIONS (FLOORS, ROOFS, SHEAR WALLS, ETC.), AS SHOWN ON THE CONTRACT DOCUMENTS, ARE COMPLETELY INSTALLED. THE STRUCTURAL MEMBERS SHOWN ON THE CONTRACT DOCUMENTS ARE DESIGNED FOR THE ANTICIPATED LOADS THAT THE STRUCTURE WILL BE SUBJECTED TO ONLY AFTER ALL STRUCTURAL ELEMENTS ARE IN PLACE AND FINAL CONNECTIONS ARE COMPLETE.
- THE GENERAL CONTRACTOR SHALL VERIFY ALL OPENING SIZES, PAD SIZES AND LOCATIONS WITH THE RESPECTIVE CONTRACTORS.

### NEW WORK IN CONNECTION WITH EXISTING CONSTRUCTION

- THE CONTRACTOR SHALL VERIFY, BY FIELD CHECK, ALL SIZES, DIMENSIONS, ELEVATIONS, LOCATIONS, ETC. OF ELEMENTS OF THE EXISTING CONSTRUCTION WHICH ARE RELATIVE TO THE NEW CONSTRUCTION.
- ALL DIMENSIONS INVOLVING NEW WORK TYING INTO OR GOVERNED BY EXISTING CONSTRUCTION SHALL BE FIELD CHECKED BY THE CONTRACTOR AND FURNISHED TO THE SUBCONTRACTOR PRIOR TO FABRICATION OF ANY WORK. THE VERIFIED DIMENSIONS SHALL APPEAR AND BE NOTED AS SUCH ON THE FIRST SHOP DRAWING SUBMITTED.
- THE ENGINEER HAS MADE ASSUMPTIONS CONCERNING THE SOUNDNESS OF THE EXISTING BUILDINGS AND THESE ASSUMPTIONS ARE THAT THIS BUILDING WAS DESIGNED AND CONSTRUCTED IN CONFORMITY WITH GOOD DESIGN AND CONSTRUCTION PRACTICES. THE CONTRACTOR SHALL TAKE EXTRAORDINARY PRECAUTIONS CONCERNING PRESERVATION OF THE BUILDING DURING DEMOLITION AND NEW CONSTRUCTION WORK. FURTHER, HE SHALL AGREE TO ASSUME ALL RESPONSIBILITY FOR THE PRESERVATION OF THIS PROPERTY.
- THE CONTRACTOR SHALL NOTIFY THE ARCHITECT/ENGINEER IMMEDIATELY OF ANY DISCREPANCIES BETWEEN CONSTRUCTION DOCUMENTS AND ACTUAL FIELD CONDITIONS.
- ALL HOLES THROUGH EXISTING CONSTRUCTION SHALL BE CORE DRILLED OR SAM CUT.

### EXCAVATION AND EARTHWORK

- THE SOILS AND FOUNDATION ENGINEERING REPORT IS FOR INFORMATIONAL PURPOSES ONLY AND SHALL NOT BE CONSIDERED PART OF THE CONTRACT DOCUMENTS.
- WATER LEVELS INDICATED ON THE BORING LOGS MAY BE SUBJECT TO SEASONAL AND/OR ANNUAL VARIATIONS. A Dewatering SYSTEM OF SUFFICIENT CAPACITY SHALL BE INSTALLED AND OPERATED TO MAINTAIN THE CONSTRUCTION AREA FREE OF WATER AT ALL TIMES.
- THE BEARING VALUE OF THE SOIL WAS DETERMINED BY FIELD EXPLORATION AND LABORATORY ANALYSIS. THE FOUNDATION DESIGN IS BASED ON THE FOLLOWING NET ALLOWABLE BEARING PRESSURES:  
 SPREAD FOOTINGS: 2000 PSF  
 WALL FOOTINGS: 2000 PSF
- ALL FOOTING EXCAVATIONS SHALL BE INSPECTED, PRIOR TO CONCRETE PLACEMENT, BY A SOILS ENGINEER TO VERIFY SUITABLE BEARING MATERIAL OF CAPACITY AS SPECIFIED.
- NOTIFY THE OWNER'S REPRESENTATIVE WHEN ADDITIONAL EXCAVATION IS REQUIRED TO REACH SUITABLE BEARING MATERIAL.
- THE SOILS ENGINEER SHALL CERTIFY IN WRITING THAT ALL FOUNDATIONS WERE PLACED ON SOIL WITH THE BEARING VALUE AS SPECIFIED.
- WITHIN THE EXCAVATION AREA OF THE FOUNDATIONS, ALL VEGETATION, TOPSOIL, PREVIOUSLY PLACED FILL AND UNSUITABLE SOILS SHALL BE REMOVED. ALL FOOTINGS TO BEAR ON VIRGIN SOIL OR PROPERLY PLACED AND COMPACTED ENGINEERED FILL.
- THE FOUNDATION DESIGN DOES NOT ACCOUNT FOR WINTER CONSTRUCTION. ANY UNENCLOSED/UNHEATED SPACES SHALL BE ADEQUATELY PROTECTED AGAINST FROST DURING WINTER CONSTRUCTION BY CONTRACTOR.

### REINFORCING STEEL

- THE REINFORCING STEEL CONTRACTOR SHALL FABRICATE ALL REINFORCING AND FURNISH ACCESSORIES, CHAIRS, SPACER BARS AND SUPPORTS NECESSARY TO SECURE THE REINFORCING UNLESS SHOWN OTHERWISE ON THE PLANS AND/OR DETAILS.
- REINFORCING STEEL SHALL BE ASTM A615, GRADE 60.
- WELDED WIRE FABRIC SHALL CONFORM TO ASTM A615.
- CONCRETE REINFORCEMENT SHALL BE PLACED ACCORDING TO THE CASE "RECOMMENDED PRACTICE FOR PLACING REINFORCING BARS".
- ALL REINFORCEMENT SPLICES SHALL BE LAPPED 48 BAR DIAMETERS MINIMUM UNLESS OTHERWISE NOTED. PROVIDE CORNER BARS FOR ALL HORIZONTAL REINFORCEMENT AT CORNERS AND INTERSECTIONS.
- TOP BARS SHALL BE HOOKED AT END SPANS.
- REINFORCEMENT TO BE WELDED SHALL CONFORM TO THE REQUIREMENTS OF ASTM A 706 AND THAT WELDING SHALL BE IN ACCORDANCE WITH AWS D1.1, STRUCTURAL WELDING CODE-REINFORCING STEEL BY AMERICAN WELDING SOCIETY.

### JOINTS IN CONCRETE

- CONSTRUCTION AND/OR CONTROL JOINTS SHALL BE MADE AS DETAILED ON THE DRAWINGS.
- CONSTRUCTION AND/OR CONTROL JOINTS FOR SLAB-ON-GRADE CONSTRUCTION SHALL BE LOCATED ON COLUMN LINES.
- MAXIMUM SPACING OF CONSTRUCTION AND/OR CONTROL JOINTS IN SLAB-ON-GRADE CONSTRUCTION SHALL BE AS SHOWN ON DRAWINGS.
- CONSTRUCTION OR CONTRACTION JOINTS IN CONCRETE FOUNDATION WALLS SHALL BE SPACED NO FURTHER THAN 20 FEET APART.
- CONSTRUCTION JOINTS FOR ELEVATED PLATWORK SHALL BE LOCATED AT THE CENTER OF THE SPAN. REINFORCEMENT SHALL BE CONTINUOUS ACROSS ALL CONSTRUCTION JOINTS.
- MAXIMUM SPACING OF CONSTRUCTION AND/OR CONTROL JOINTS IN CANTILEVERED RETAINING WALLS SHALL BE 30'-0".
- 1" EXPANSION JOINTS IN CANTILEVERED RETAINING WALLS SHALL BE PLACED AT 60' ON CENTER MAXIMUM.

### FOUNDATION/UNDERGROUND MECHANICAL COORDINATION

- UNDERGROUND SEWER, WATER, GAS LINES, ETC. CROSSING CONTIGUOUS WALL FOUNDATIONS SHALL NOT PASS THROUGH FOOTINGS. WHERE PIPE OCCURS ABOVE TOP OF FOOTING, SLEEVE THROUGH WALL. WHERE PIPE OCCURS IN FOOTING DEPTH, DROP TOP OF FOOTING SUCH THAT PIPE PASSES JUST ABOVE FOOTING. IF TOP OF PIPE IS LESS THAN 6" BELOW BOTTOM OF FOOTING, PROVIDE 1" COMPRESSIBLE FOAM INSULATION BELOW FOOTING FOR WIDTH OF TRENCH.

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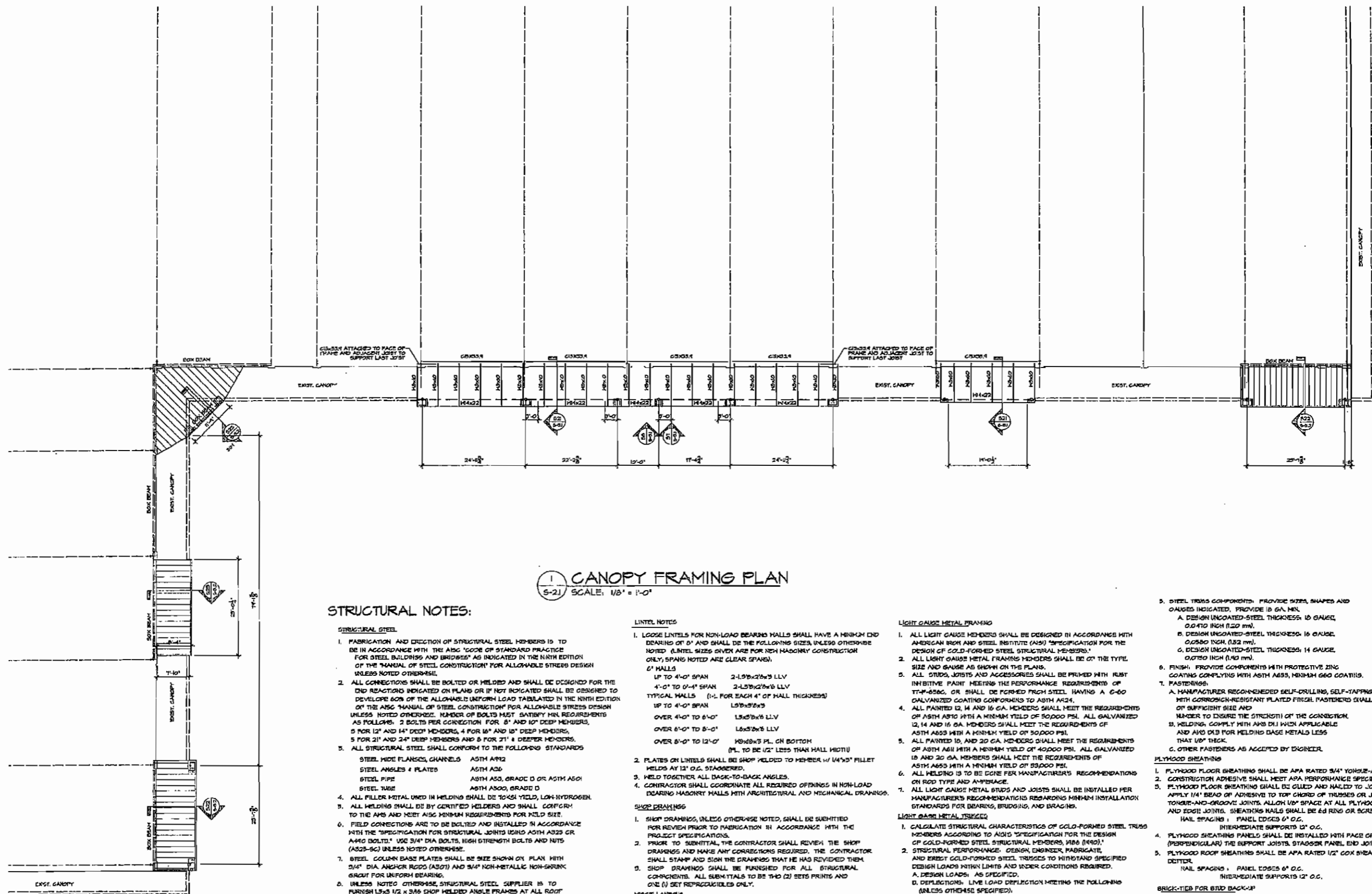
RENOVATION OF EXISTING  
 HORSESHOE BEND PLAZA  
 POWDER SPRINGS ROAD, COBB COUNTY, GEORGIA

BRUNO ARCHITECTURAL

200 Pgs  
 605

FOUNDATION  
 PLAN  
 DATE: 1.20.06

\$1.7



1 CANOPY FRAMING PLAN  
 5-21 SCALE: 1/8" = 1'-0"

STRUCTURAL NOTES:

STRUCTURAL STEEL

- FABRICATION AND ERECTION OF STRUCTURAL STEEL MEMBERS IS TO BE IN ACCORDANCE WITH THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" AS INDICATED IN THE NINTH EDITION OF THE "MANUAL OF STEEL CONSTRUCTION" FOR ALLOWABLE STRESS DESIGN UNLESS NOTED OTHERWISE.
- ALL CONNECTIONS SHALL BE BOLTED OR WELDED AND SHALL BE DESIGNED FOR THE END REACTIONS INDICATED ON PLANS OR IF NOT INDICATED SHALL BE DESIGNED TO DEVELOPE 60% OF THE ALLOWABLE UNIFORM LOAD TABULATED IN THE NINTH EDITION OF THE AISC "MANUAL OF STEEL CONSTRUCTION" FOR ALLOWABLE STRESS DESIGN UNLESS NOTED OTHERWISE. NUMBER OF BOLTS MUST SATISFY MIN. REQUIREMENTS AS FOLLOWS: 2 BOLTS PER CONNECTION FOR 8" AND 10" DEEP MEMBERS, 3 FOR 12" AND 14" DEEP MEMBERS, 4 FOR 16" AND 18" DEEP MEMBERS, 5 FOR 21" AND 24" DEEP MEMBERS AND 6 FOR 21" & DEEPER MEMBERS.
- ALL STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING STANDARDS:
 

STEEL WIDE FLANGES, CHANNELS	ASTM A992
STEEL ANGLES & PLATES	ASTM A36
STEEL PIPE	ASTM A53, GRADE D OR ASTM A501
STEEL TUBE	ASTM A500, GRADE D
- ALL FILLER METAL USED IN WELDING SHALL BE 100% YIELD, LOW-HYDROGEN.
- ALL WELDING SHALL BE BY CERTIFIED WELDERS AND SHALL CONFORM TO THE AWS AND MEET ALL MINIMUM REQUIREMENTS FOR WELD SIZE.
- FIELD CONNECTIONS ARE TO BE BOLTED AND INSTALLED IN ACCORDANCE WITH THE "SPECIFICATION FOR STRUCTURAL JOINTS USING A508 A325 OR A325-SC" UNLESS NOTED OTHERWISE.
- STEEL COLUMN BASE PLATES SHALL BE SIZE SHOWN ON PLAN WITH 3/4" DIA. ANCHOR BOLDS (A307) AND 3/4" NON-METALLIC NON-SHRINK GROUT FOR UNIFORM BEARING.
- UNLESS NOTED OTHERWISE, STRUCTURAL STEEL SUPPLIER IS TO FURNISH 1/2" X 3/16" SHOP WELDED ANGLE FRAMES AT ALL ROOF OPENINGS. VERIFY SIZE AND LOCATION WITH CONTRACTOR.
- ALL PLATES USED IN BOLTED CONNECTIONS SHALL HAVE ROLLED OR GAS CUT EDGES.
- ALL STRUCTURAL STEEL AND MISCELLANEOUS METALS SHALL BE PRIME PAINTED WITH ONE COAT OF FABRICATOR'S STANDARD RUST-INHIBITIVE PRIMER OR AS INDICATED IN THE PROJECT SPECIFICATION TOUGH UP ALL DISTURBED AREAS AFTER ERECTION. STEEL TO BE FIREPROOFED SHALL RECEIVE PAINT/FINISH PROCESS COMPATIBLE WITH FIREPROOFING.
- ADJUSTABLE MASONRY TIES SHALL BE FURNISHED AT 16" O.C. VERTICALLY AND 24" O.C. HORIZONTALLY ON ALL STEEL MEMBERS ENCASED IN OR ADJACENT TO MASONRY WALLS WHETHER OR NOT SUCH ANCHORS ARE SHOWN ON THE DRAWINGS. TIES SHALL BE CAPABLE OF TRANSMITTING FORCES PERPENDICULAR TO THE PLANE OF THE WALL.

LINTEL NOTES

- LOOSE LINTELS FOR NON-LOAD BEARING WALLS SHALL HAVE A MINIMUM END BEARING OF 6" AND SHALL BE THE FOLLOWING SIZES, UNLESS OTHERWISE NOTED (LINTEL SIZES GIVEN ARE FOR NON-MASONRY CONSTRUCTION ONLY, SPANS NOTED ARE CLEAR SPANS).
 

UP TO 4'-0" SPAN	2-1/2"x2"x3 LLV
4'-0" TO 6'-4" SPAN	2-1/2"x2"x3 LLV
TYPICAL WALLS (1-1/2" FOR EACH 4' OF WALL THICKNESS)	
UP TO 4'-0" SPAN	1-5/8"x2"x3
OVER 4'-0" TO 6'-4"	1-5/8"x2"x3 LLV
OVER 6'-0" TO 8'-0"	1-5/8"x2"x3 LLV
OVER 8'-0" TO 12'-0"	1-5/8"x2"x3 PL. ON BOTTOM (PL. TO BE 1/2" LESS THAN HALL WIDTH)
- PLATES ON LINTELS SHALL BE SHOP WELDED TO MEMBER W/ 1/4"x3" FILLET WELDS AT 12" O.C. STAGGERED.
- WELD TOGETHER ALL BACK-TO-BACK ANGLES.
- CONTRACTOR SHALL COORDINATE ALL REQUIRED OPENINGS IN NON-LOAD BEARING MASONRY WALLS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.

SHOP DRAWINGS

- SHOP DRAWINGS, UNLESS OTHERWISE NOTED, SHALL BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.
- PRIOR TO SUBMITTAL, THE CONTRACTOR SHALL REVIEW THE SHOP DRAWINGS AND MAKE ANY CORRECTIONS REQUIRED. THE CONTRACTOR SHALL STAMP AND SIGN THE DRAWINGS THAT HE HAS REVIEWED THEM.
- SHOP DRAWINGS SHALL BE FURNISHED FOR ALL STRUCTURAL COMPONENTS. ALL SUBMITTALS TO BE TWO (2) SETS PRINTS AND ONE (1) SET REPRACUCIBLES ONLY.

MISCELLANEOUS

- ALL ANCHOR BOLTS FOR MECHANICAL AND ELECTRICAL EQUIPMENT ARE FURNISHED AND LOCATED BY THE RESPECTIVE CONTRACTORS AND SET BY THE GENERAL CONTRACTOR EXCEPT WHERE THE OTHER CONTRACTORS FURNISH THEIR OWN CONCRETE PADS.
- ALL PIPE SLEEVES ARE FURNISHED BY AND LOCATED BY THE MECHANICAL AND ELECTRICAL CONTRACTOR AND SET BY THE GENERAL CONTRACTOR.
- THE GENERAL CONTRACTOR SHALL VERIFY ALL OPENING SIZES, PAD SIZES, AND LOCATIONS WITH THE RESPECTIVE CONTRACTORS.
- ALL CORE DRILLING SHALL BE DONE BY THE MECHANICAL AND ELECTRICAL CONTRACTORS FOR THEIR OWN WORK UNDER THE SUPERVISION OF THE GENERAL CONTRACTOR. NO REINFORCING SHALL BE CUT. VERIFY LOCATION OF REINFORCING BEFORE CORE DRILLING. THERE SHALL NOT BE ANY CORE DRILLING THROUGH BEAMS OR COLUMNS. MAXIMUM CORE HOLE THROUGH SLAB SHALL BE PIPE DIAMETER PLUS 1/2".

LIGHT GAUGE METAL FRAMING

- ALL LIGHT GAUGE MEMBERS SHALL BE DESIGNED IN ACCORDANCE WITH AMERICAN IRON AND STEEL INSTITUTE (AISI) "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS."
- ALL LIGHT GAUGE METAL FRAMING MEMBERS SHALL BE OF THE TYPE, SIZE AND GAUGE AS SHOWN ON THE PLANS.
- ALL STUDS, JOISTS AND ACCESSORIES SHALL BE PRIMED WITH RUST INHIBITIVE PAINT MEETING THE PERFORMANCE REQUIREMENTS OF TYP-5566, OR SHALL BE FORMED FROM STEEL HAVING A G-60 GALVANIZED COATING CONFORMING TO ASTM A653.
- ALL PAINTED 12, 14 AND 16 GA. MEMBERS SHALL MEET THE REQUIREMENTS OF ASTM A575 WITH A MINIMUM YIELD OF 50,000 PSI. ALL GALVANIZED 12, 14 AND 16 GA. MEMBERS SHALL MEET THE REQUIREMENTS OF ASTM A653 WITH A MINIMUM YIELD OF 50,000 PSI.
- ALL PAINTED 18, AND 20 GA. MEMBERS SHALL MEET THE REQUIREMENTS OF ASTM A575 WITH A MINIMUM YIELD OF 40,000 PSI. ALL GALVANIZED 18 AND 20 GA. MEMBERS SHALL MEET THE REQUIREMENTS OF ASTM A653 WITH A MINIMUM YIELD OF 50,000 PSI.
- ALL WELDING IS TO BE DONE PER MANUFACTURER'S RECOMMENDATIONS ON ROD TYPE AND ANCHORAGE.
- ALL LIGHT GAUGE METAL STUDS AND JOISTS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS REGARDING MINIMUM INSTALLATION STANDARDS FOR BEARINGS, BRIDGING, AND BRACING.

LIGHT GAUGE METAL TRUSSES

- CALCULATE STRUCTURAL CHARACTERISTICS OF COLD-FORMED STEEL TRUSS MEMBERS ACCORDING TO AISI'S "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, 1986 (1990)." STRUTURAL PERFORMANCE: DESIGN, ENGINEER, FABRICATE, AND ERECT COLD-FORMED STEEL TRUSSES TO WITHSTAND SPECIFIED DESIGN LOADS WITHIN LIMITS AND UNDER CONDITIONS REQUIRED.
  - DESIGN LOADS: AS SPECIFIED.
  - DEFLECTIONS: LIVE LOAD DEFLECTION MEETING THE FOLLOWING (UNLESS OTHERWISE SPECIFIED):
    - FLOOR TRUSSES: VERTICAL DEFLECTION LESS THAN OR EQUAL TO 1/360 OF THE SPAN.
    - ROOF TRUSSES: VERTICAL DEFLECTION LESS THAN OR EQUAL TO 1/240 OF THE SPAN.
  - DESIGN FRAMING SYSTEMS TO PROVIDE FOR MOVEMENT OF FRAMING MEMBERS WITHOUT DAMAGE OR OVERSTRESSING, SHEATHING FAILURE, CONNECTION FAILURE, UNEVEN STRAIN ON FASTENERS AND ANCHORS, OR OTHER DETRIMENTAL EFFECTS WHEN SUBJECT TO A MAXIMUM AMBIENT TEMPERATURE CHANGE RANGE OF 120° F (67° C).
- ALL COMPONENT GAUGES: FABRICATE COMPONENTS OF STRUCTURAL QUALITY STEEL SHEET PER ASTM A575 WITH A MINIMUM YIELD STRENGTH OF 40,000 PSI.
- BRACING, BRIDGING AND BLOCKING MEMBERS: FABRICATED COMPONENTS OF STRUCTURAL QUALITY STEEL SHEET PER ASTM A575 WITH A MINIMUM YIELD STRENGTH OF 40,000 PSI.

- STEEL TRUSS COMPONENTS: PROVIDE SIZES, SHAPES AND GAUGES INDICATED. PROVIDE 16 GA. MIN.
  - DESIGN UNCOATED-STEEL THICKNESS: 16 GAUGE, 0.0750 INCH (1.92 mm).
  - DESIGN UNCOATED-STEEL THICKNESS: 16 GAUGE, 0.0580 INCH (1.52 mm).
  - DESIGN UNCOATED-STEEL THICKNESS: 14 GAUGE, 0.0750 INCH (1.92 mm).
- FINISH: PROVIDE COMPONENTS WITH PROTECTIVE ZINC COATING CONPLYING WITH ASTM A653, MINIMUM 660 COATINGS.
- FASTENERS:
  - MANUFACTURER RECOMMENDED SELF-DRILLING, SELF-TAPPING SCREWS WITH CORROSION-RESISTANT PLATED FINISH. FASTENERS SHALL BE OF SUFFICIENT SIZE AND NUMBER TO INSURE THE STRENGTH OF THE CONNECTION.
  - WELDING: COMPLY WITH AWS D11 WHICH APPLICABLE AND ALSO D13 FOR WELDING BASE METALS LESS THAN 1/8" THICK.
  - OTHER FASTENERS AS ACCEPTED BY ENGINEER.

PLYWOOD SHEATHING

- PLYWOOD FLOOR SHEATHING SHALL BE APA RATED 3/4" TONGUE-AND-GROOVE.
- CONSTRUCTION ADHESIVE SHALL MEET APA PERFORMANCE SPECIFICATION APG-01.
- PLYWOOD FLOOR SHEATHING SHALL BE GULLED AND NAILED TO JOISTS AND TRUSSES. APPLY 1/4" BEAD OF ADHESIVE TO TOP CHORD OF TRUSSES OR JOISTS AND TO TONGUE-AND-GROOVE JOINTS. ALLOW 1/8" SPACE AT ALL PLYWOOD END JOINTS AND EDGE JOINTS. SHEATHING NAILS SHALL BE 8D NAIL OR SCREW SHANK NAIL SPACING:
 

PANEL EDGES 6" O.C.
INTERMEDIATE SUPPORTS 12" O.C.
- PLYWOOD SHEATHING PANELS SHALL BE INSTALLED WITH GRAIN ACROSS (PERPENDICULAR) THE SUPPORT JOISTS. STAGGER PANEL END JOINTS.
- PLYWOOD ROOF SHEATHING SHALL BE APA RATED 1/2" COX SHEATHING OR BETTER.
 

NAIL SPACING: PANEL EDGES 6" O.C.
INTERMEDIATE SUPPORTS 12" O.C.

BRICK-TIES FOR STUD BACK-UP

- THERE SHALL BE A MINIMUM OF ONE BRICK TIE FOR EVERY 2-2/3 S.F. OF WALL AREA. THESE SHALL BE SPACED AT A MAXIMUM OF 24 INCHES ON CENTER. TIES SHALL BE OF MINIMUM 4 GA. CORROSION RESISTANT WIRE AND SHALL BE OF AN ADJUSTABLE TYPE SUCH AS "DUR-O-WAL" ADJUSTABLE DAZOT TYPE OR EQUAL. CORRUGATED GALVANIZED SHEET TIES ARE NOT ACCEPTABLE. ALL TIES MUST BE ATTACHED THROUGH THE SHEATHINGS TO THE STUDS PER MANUFACTURER'S RECOMMENDATIONS.

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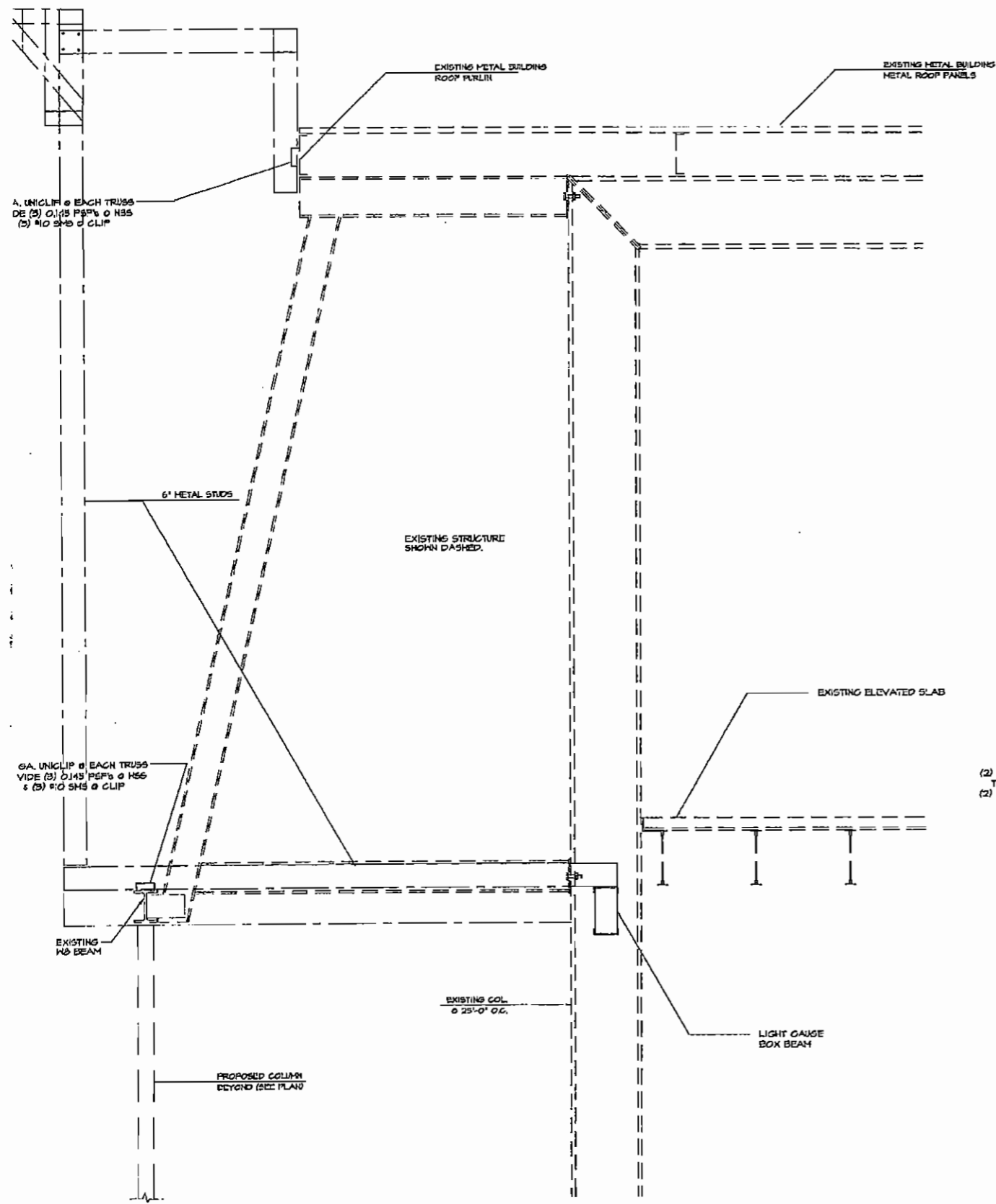


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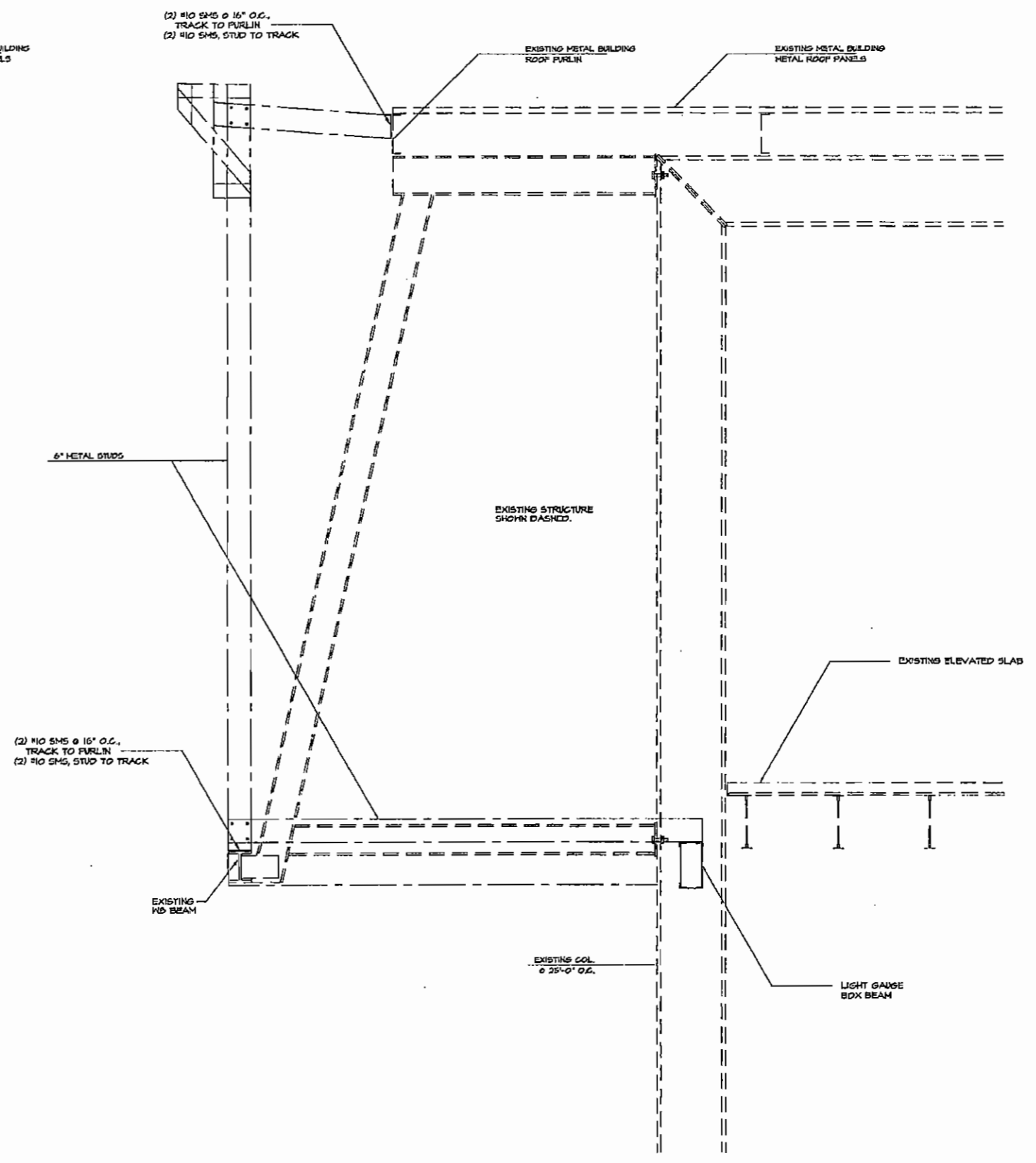
BRUNO ARCHITECTURAL  
 RENOVATION OF EXISTING  
 HORSESHOE BEND PLAZA  
 POWDER SPRINGS ROAD, COBB COUNTY, GEORGIA

CANOPY FRAMING  
 PLAN  
 DATE: 12.0.06  
 JOB NO. 605  
 \$2.7





FRAMING SECTION S-22



FRAMING SECTION S-23

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RENOVATION OF EXISTING  
 HORSESHOE BEND PLAZA  
 POWDER SPRINGS ROAD, COBB COUNTY, GEORGIA

BRUNO ARCHITECTURAL

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