

GENERAL NOTES

100 DESIGN CRITERIA

- 100.1 DESIGN BUILDING CODE: 2004 FLORIDA BUILDING CODE
100.2 GRAVITY DESIGN LOADS: DEAD LOADS: SUPERIMPOSED 15 PSF
LIVE LOADS: b. ROOF 20 PSF c. PUBLIC AREAS 100 PSF d. CORRIDORS & STAIRS 100 PSF e. RESIDENTIAL 40 PSF
100.3 LATERAL LOADS: 1. WIND LOADS (2004 FBC, ASCE 7-02) a. V = 120 MPH b. EXPOSURE CATEGORY: C c. ENCLOSURE CLASSIFICATION: "ENCLOSED" d. BUILDING CATEGORY: II e. IMPORTANCE FACTOR (I) = 1.00 f. INTERNAL PRESSURE COEFFICIENT (Cp) = 0.18 OR -0.18 g. COMPONENTS AND CLADDING PRESSURES: SEE "COMPONENTS AND CLADDING WIND LOADS" TABLE, AND "COMPONENTS AND CLADDING WIND PRESSURE DIAGRAM"

110 GENERAL

- 110.1 THIS DRAWING HAS BEEN PRODUCED ENTIRELY ON ATLANTIC ENGINEERING SERVICES AND SYSTEMS. ANY OTHER LETTERING, LINES OR SYMBOLS, OTHER THAN PROFESSIONAL STAMPS AND SIGNATURES, HAVE BEEN MADE WITHOUT THE AUTHORIZATION OF ATLANTIC ENGINEERING SERVICES AND ARE INVALID.
110.2 THE STRUCTURAL DRAWINGS SHALL GOVERN THE WORK FOR ALL STRUCTURAL FEATURES, UNLESS NOTED OTHERWISE. THE ARCHITECTURAL DRAWINGS SHALL GOVERN THE WORK FOR ALL DIMENSIONS.
110.3 THE METHOD AND FREQUENCY OF ATTACHING MECHANICAL EQUIPMENT UNITS, ETC., TO THE STRUCTURAL ELEMENTS SHALL BE SUBJECT TO THE STRUCTURAL ENGINEER'S REVIEW AND APPROVAL.
110.4 THE CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS, DIMENSIONS, ETC., AND SHALL NOTIFY THE ARCHITECT OF ANY AND ALL DISCREPANCIES, ADDITIONAL INFORMATION, ETC., BEFORE BEGINNING THE WORK.
110.5 THE CONTRACTOR SHALL USE EXTREME CAUTION IN THE DEMOLITION OF EXISTING STRUCTURES. SUCH DEMOLITION SHALL BE PERFORMED IN SUCH A MANNER AS TO MAINTAIN THE STRUCTURAL INTEGRITY OF ALL EXISTING STRUCTURES TO REMAIN. PROVIDE SHORING AS REQUIRED.
110.6 ALL STRUCTURAL WORK SHALL BE INSPECTED IN ACCORDANCE WITH THE BUILDING CODE AND ALL LOCAL ORDINANCES. THE OWNER SHALL ENGAGE AN EXPERIENCED, QUALIFIED INSPECTION AGENCY, SUBJECT TO THE REVIEW OF THE ARCHITECT, TO PERFORM ALL INSPECTION WORK, AS REQUIRED. THRESHOLD BUILDINGS SHALL BE INSPECTED BY A LICENSED THRESHOLD INSPECTOR IN ACCORDANCE WITH THE THRESHOLD INSPECTION PLAN.
110.7 ALL PRE-FABRICATED STAIRS SHALL BE CAPABLE OF SUPPORTING THE SELFWEIGHT PLUS SUPERIMPOSED DEAD LOADS AND A SUPERIMPOSED LIVE LOAD OF 100 PSF. IN ADDITION, STAIR DESIGN SHALL MEET ALL THE RELEVANT REQUIREMENTS OF THE LOCAL BUILDING CODE. THE CONTRACTOR SHALL SUBMIT CALCULATIONS AND SHOP DRAWINGS SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED TO PRACTICE IN THE STATE OF FLORIDA FOR REVIEW BY THE STRUCTURAL ENGINEER.
110.8 STEP FOOTINGS BELOW ALL SANITARY AND WATER LINES. PROVIDE STEPS IN THE FOUNDATIONS IN ACCORDANCE WITH THE TYPICAL DETAILS. COORDINATE THE EXACT LOCATION AND ELEVATION OF THE PLUMBING LINES WITH THE MECHANICAL AND PLUMBING DRAWINGS AND CONTRACTORS. PROVIDE SLEEVES IN THE FOUNDATION WALLS AS REQUIRED FOR PIPE PENETRATIONS.

120 SHOP DRAWINGS

- 120.1 THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR REVIEW BY ATLANTIC ENGINEERING SERVICES AND THE PROJECT ARCHITECT. SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL COMPONENTS INCLUDING, BUT NOT LIMITED TO THE FOLLOWING: a. FABRICATED STRUCTURAL STEEL b. REINFORCING STEEL FOR CONCRETE AND MASONRY c. POST-TENSIONING STRANDS AND ANCHORAGES d. STUDRAILS e. CONCRETE AND/OR MASONRY POST-INSTALLED ANCHORS f. CONCRETE FORMWORK FOR STRUCTURAL CONCRETE MEMBERS g. SHORING AND RESHORING h. LIGHT GAGE METAL FRAMING i. PREFABRICATED LIGHT GAGE METAL TRUSSES
120.2 SHOP DRAWINGS TO BE SUBMITTED SHALL PROVIDE COMPLETE INFORMATION FOR THE PRODUCTS OR COMPONENTS TO BE SUPPLIED. SUBMITTAL INFORMATION SHALL INCLUDE, BUT NOT BE LIMITED TO: MEMBER SIZES AND DIMENSIONS; GRADES OF MATERIAL; FURNISHED MATERIAL PREPARATION REQUIRED; MATERIAL FINISH AND MATERIAL COATINGS TO BE FURNISHED; INFORMATION REGARDING CUTS, COPIES AND HOLES REQUIRED FOR OTHER TRADES; END CONNECTIONS; CAMBER AND OTHER DEVIATION FROM LINE; SPECIAL ERECTION AND/OR INSTALLATION PROCEDURES INCLUDING REQUIREMENTS FOR TEMPORARY STABILIZATION.
120.3 THE CONTRACTOR SHALL SUBMIT PRINTED COPIES OF SHOP DRAWINGS FOR REVIEW BY ATLANTIC ENGINEERING SERVICES. PRINTED COPIES OF SHOP DRAWINGS SHALL CONSIST OF TWO (2) NON-REPRODUCIBLE PRINTS AND ONE (1) REPRODUCIBLE TRACING/SEPA UPON THE COMPLETION OF THE SHOP DRAWING REVIEW AND ONE (1) TRACING/SEPA SHALL BE RETURNED TO THE PROJECT ARCHITECT FOR DISTRIBUTION TO THE CONTRACTOR.
120.4 THE CONTRACTOR SHALL NOT DIRECTLY INCORPORATE THE STRUCTURAL DRAWINGS OR PORTIONS THEREOF INTO SHOP DRAWINGS OR ERECTION DRAWINGS TO BE SUBMITTED FOR THIS PROJECT WITHOUT THE EXPRESS PERMISSION OF ATLANTIC ENGINEERING SERVICES. SUBMITTED SHOP DRAWINGS WHICH CONTAIN COPIES OR REPRODUCTIONS OF ANY PORTION OF THE STRUCTURAL DRAWINGS WITHOUT THE EXPRESS WRITTEN PERMISSION OF ATLANTIC ENGINEERING SERVICES WILL BE RETURNED REJECTED. IF PERMISSION TO USE THE STRUCTURAL DRAWINGS IS GRANTED BY ATLANTIC ENGINEERING SERVICES, THE CONTRACTOR SHALL BE REQUIRED TO ENTER INTO A WRITTEN AGREEMENT WITH ATLANTIC ENGINEERING SERVICES AND TO PAY A SERVICE FEE TO USE THE STRUCTURAL DRAWINGS IN THE PREPARATION OF SHOP DRAWINGS.
120.5 THE REVIEW OF SHOP DRAWINGS AND OTHER SUBMITTALS FOR THIS PROJECT IS FOR CONFORMANCE WITH THE DESIGN CONCEPT AND FOR GENERAL COMPLIANCE WITH THE INFORMATION CONTAINED IN THE CONTRACT DOCUMENTS. COMMENTS REGARDING THESE SUBMITTALS DO NOT RELIEVE THE CONTRACTOR FROM COMPLIANCE WITH THE CONTRACT DOCUMENTS. THE CONTRACTOR IS RESPONSIBLE FOR PERFORMING HIS WORK IN A SAFE AND SATISFACTORY MANNER.

200 FOUNDATIONS - GENERAL

- 200.1 FOUNDATIONS HAVE BEEN DESIGNED AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH CRITERIA ESTABLISHED BY MACTEC ENGINEERING AND CONSULTING, INC. IN THE GEOTECHNICAL REPORT DATED NOVEMBER 20, 2008 (MACTEC PROJECT NO. 6734-06-9223).
200.2 THE CONTRACTOR SHALL OBSERVE WATER CONDITIONS AT THE SITE AND TAKE NECESSARY PRECAUTIONS TO INSURE THAT THE FOUNDATION EXCAVATIONS REMAIN DRY DURING CONSTRUCTION. CONTRACTOR TO PROVIDE FOR Dewatering AS NECESSARY.
200.3 THE CONTRACTOR SHALL USE EXTREME CAUTION DURING EXCAVATION. SUCH EXCAVATION SHALL BE PERFORMED IN SUCH A MANNER AS TO MAINTAIN THE STRUCTURAL INTEGRITY OF ALL EXISTING STRUCTURES TO REMAIN. CONTRACTOR TO PROVIDE TEMPORARY SHORING AS REQUIRED.
200.4 CONCRETE SLABS ON GRADE HAVE BEEN DESIGNED TO BEAR ON COMPACTED SUBGRADE SOILS OR PROPERLY COMPACTED FILL AS PER 200.1.

205 SITE NOTES

- 205.1 THE GEOTECHNICAL EVALUATION AND RECOMMENDATIONS ARE PROVIDED SOLELY BY MACTEC ENGINEERING AND CONSULTING, INC. THE FOLLOWING SITE PREPARATION INSTRUCTIONS ARE ABBREVIATED EXCERPTS FROM THE GEOTECHNICAL REPORT. THE CONTRACTOR SHALL OBTAIN AND REVIEW THE GEOTECHNICAL REPORT PRIOR TO THE COMMENCEMENT OF SITE WORK. ACTUAL SITE PREPARATION PROCEDURES SHALL CONFORM TO THE INSTRUCTIONS IN THE UNABRIDGED GEOTECHNICAL REPORT AND THE FIELD DIRECTIONS BY THE GEOTECHNICAL ENGINEER.
205.2 REMOVE ALL TOPSOIL, VEGETATION, ORGANIC AND DELETERIOUS MATERIAL. STRIPPING DEPTHS ARE ANTICIPATED TO RANGE BETWEEN 0 INCHES AND 18 INCHES. SOME AREAS MAY REQUIRE STRIPPING DEEPER THAN 18 INCHES.
205.3 EXISTING BUILDING, FOUNDATIONS, AND BURIED UTILITIES SHALL BE REMOVED FROM THE AREA. ANY ABANDONED UTILITIES THAT ARE NOT REMOVED SHALL BE GROUTED SOLID.
205.4 A TEST PIT IS RECOMMENDED TO DETERMINE THE EXTENT OF THE BURIED ORGANIC SOILS THAT WERE DISCOVERED IN THE SOIL BORINGS. THE PIT SHALL BE OBSERVED AND DOCUMENTED BY A PROFESSIONAL GEOTECHNICAL ENGINEER. SOME OVEREXCAVATION AND BACKFILLING SHOULD BE ANTICIPATED.
205.5 AFTER STRIPPING AND SITE CLEARING, COMPACT THE EXPOSED SOILS WITH A MINIMUM OF EIGHT OVERLAPPING PASSES OF A VIBRATORY DRUM ROLLER HAVING A TOTAL OPERATING STATIC WEIGHT OF AT LEAST 10 TONS AND A DRUM DIAMETER OF 5 FEET. COMPACTION SHALL CONTINUE UNTIL A DENSITY OF AT LEAST 95% OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY (ASTM D-1557) HAS BEEN OBTAINED TO A DEPTH OF AT LEAST 12 INCHES BELOW THE COMPACTED SURFACE. THE ROLLER COVERAGES SHALL BE DIVIDED EVENLY INTO TWO PERPENDICULAR DIRECTIONS.
205.6 STRUCTURAL FILL SHALL CONSIST OF ALL INORGANIC, NON-PLASTIC, GRANULAR SOIL CONTAINING LESS THAN 10% MATERIAL PASSING THE NO. 200 MESH SIEVE. PLACE STRUCTURAL FILL IN LIFTS NOT EXCEEDING 12 INCHES IN LOOSE THICKNESS AND COMPACT EACH LIFT THOROUGHLY WITH A VIBRATORY DRUM ROLLER HAVING A TOTAL OPERATING STATIC WEIGHT OF AT LEAST 10 TONS AND A DRUM DIAMETER OF 5 FEET. COMPACTION SHALL BE CONTINUED UNTIL A DENSITY OF AT LEAST 95% OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY IS OBTAINED IN EACH LIFT.
205.7 THE UPPER 12 INCHES OF ALL SANDY BEARING SOILS IN THE SHALLOW FOUNDATION EXCAVATION BOTTOM SHALL BE COMPACTED TO AT LEAST 95% OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY. COMPACTION OR RE-COMPACTION OF SANDY BEARING SOILS SHALL BE DONE WITH A RELATIVELY LIGHTWEIGHT WALK-BEHIND SLED OR ROLLER. IN ORDER TO ACHIEVE THE RECOMMENDED COMPACTION CRITERION, IT WILL BE NECESSARY FOR THE GROUNDWATER TABLE TO BE MAINTAINED AT LEAST TWO FEET BELOW THE FOOTING BEARING LEVEL.
205.8 TESTING: A MINIMUM OF ONE IN-PLACE DENSITY TEST PER 5000 SF PER LAYER OF COMPACTED SURFACE AND AT LEAST ONE IN-PLACE DENSITY TEST FOR EVERY 100 SF OF SPREAD FOOTING BEARING AREA AND EVERY 50 LINEAL FEET OF CONTINUOUS FOOTING.
210 SHALLOW FOUNDATIONS
210.1 FOUNDATIONS HAVE BEEN DESIGNED AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH CRITERIA ESTABLISHED IN THE GEOTECHNICAL REPORT PER 200.1.
210.2 SPREAD FOOTINGS HAVE BEEN DESIGNED TO BEAR ON UNDISTURBED SOILS OR PROPERLY COMPACTED FILL HAVING AN ALLOWABLE BEARING CAPACITY OF 3000 PSF, AS PER 200.1.
210.3 ELEVATIONS SHOWN ON THE DRAWINGS AT WHICH FOUNDATIONS ARE TO BEAR ARE APPROXIMATE. MATERIAL ON WHICH FOUNDATIONS ARE TO BEAR SHALL HAVE AT LEAST THE ABOVE NOTED CAPACITY. ALL EXTERIOR FOOTINGS SHALL BEAR A MINIMUM OF 1'-9" BELOW FINISHED GRADE.
210.4 THE OWNER/CONTRACTOR SHALL RETAIN THE SERVICES OF A PROFESSIONAL GEOTECHNICAL ENGINEER, SUBJECT TO THE APPROVAL OF THE ARCHITECT, TO INSPECT THE FOUNDATIONS, BEARING LEVELS, ETC., AND VERIFY THAT THE MATERIAL ON WHICH FOUNDATIONS BEAR HAS AT LEAST THE ABOVE NOTED CAPACITY AS PER 200.1.
300 REINFORCED CONCRETE
300.1 ALL REINFORCED CONCRETE WORK SHALL BE IN CONFORMANCE WITH THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318, LATEST EDITION) AND SPECIFICATIONS FOR STRUCTURAL CONCRETE (ACI 301, LATEST EDITION) OF THE AMERICAN CONCRETE INSTITUTE.
300.2 MINIMUM Fc REQUIRED AT 28 DAYS: a. FOUNDATIONS AND CONCRETE FILL: Fc = 3000 PSI b. SLABS ON GRADE: MIN. Fc = 4000 PSI c. BEAMS AND ELEVATED SLABS: MIN. Fc = 5000 PSI d. COLUMNS: MIN. Fc = 4000 PSI UNO
300.3 MAXIMUM WATER CEMENT RATIO: a. FOUNDATIONS AND CONCRETE FILL: 0.60 b. SLABS ON GRADE: 0.58 c. BEAMS AND ELEVATED SLABS: 0.45 d. COLUMNS: 0.56
300.4 ALL CONCRETE SHALL BE NORMAL WEIGHT CONCRETE (144 PCF +/-) WITH ALL CEMENT CONFORMING TO ASTM C150, TYPE I, MAXIMUM AGGREGATE SIZE SHALL BE 1-1/2" FOR FOOTINGS AND 3/4" FOR WALLS AND SLABS, CONFORMING TO ASTM C33.
300.5 REINFORCEMENT: a. DEFORMED BARS: ASTM A615, GRADE 60 b. WELDED WIRE MESH: ASTM A185
300.6 MINIMUM COVER FOR CAST-IN-PLACE CONCRETE REINF., UNLESS OTHERWISE SHOWN ON DRAWINGS, SHALL BE AS FOLLOWS: FOOTINGS & GRADE BEAMS: 3" COLUMNS & PEDESTALS (OVER VERT. REINF.): 2" BEAMS (OVER MAIN REINF.): 2" SLABS CAST AGAINST EARTH: CENTERED 2" EXTERIOR WALLS: 2" SHEAR WALLS: 3/4"
300.7 SPLICES IN REINFORCEMENT, WHERE PERMITTED, SHALL BE AS FOLLOWS: WELDED WIRE MESH: 8" ALL OTHERS: CLASS "B" TENSION, CASE "1" MINIMUM, UNLESS OTHERWISE NOTED
300.8 CLASS "B" CASE "1" TENSION SPLICES IN INCHES, SHALL BE AS FOLLOWS: 3000 PSI TOP BARS ALL OTHERS TOP BARS ALL OTHERS TOP BARS ALL OTHERS
#3 (#10) 28 22 18 22 17
#4 (#13) 37 29 22 25 22
#5 (#16) 47 36 40 31 36
#6 (#19) 58 43 38 37 43
#7 (#22) 61 63 70 54 63
#8 (#25) 63 62 60 62 72
#9 (#28) 105 81 70 70 81
#10 (#32) 119 102 79 91 101
#11 (#36) 131 101 113 87 101

- 300.13 ALL TIES SHALL HAVE 135 DEGREE HOOKS.
300.14 PROVIDE 1/2" PRE-MOLDED EXPANSION MATERIAL WHERE SLAB ON GRADE IS POURED AROUND COLUMNS AND AGAINST WALLS UNLESS OTHERWISE SHOWN ON DRAWINGS.
300.15 CONSTRUCTION JOINTS FOR SLABS ON GRADE SHALL BE SPACED NO MORE THAN 18'-0" ON CENTER PANELS SHALL BE AS SQUARE AS POSSIBLE WITH A LENGTH TO WIDTH RATIO NOT TO EXCEED 1.5.
300.16 CONTRACTOR SHALL VERIFY DIMENSIONS AND LOCATIONS OF ALL SLOTS, PIPE SLEEVES, ETC., AS REQUIRED FOR MECHANICAL TRADES BEFORE CONCRETE IS PLACED. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS SHOWING LOCATIONS FOR ALL SLAB OPENINGS FOR REVIEW BY STRUCTURAL ENGINEER.
300.17 PIPES OR CONDUITS PLACED IN SLABS SHALL NOT HAVE AN OUTSIDE DIAMETER LARGER THAN 1/3 THE SLAB THICKNESS AND SHALL NOT BE SPACED CLOSER THAN 3 DIAMETERS ON CENTERS. ALUMINUM CONDUITS SHALL NOT BE PLACED IN CONCRETE. NO CONDUITS SHALL BE PLACED IN SLAB WITHIN 12" OF COLUMN FACE OR FACE OF BEARING WALL. NO CONDUITS MAY BE PLACED IN EXTERIOR SLABS.
300.18 PRIOR TO PLACING CONCRETE, THE CONTRACTOR SHALL SUBMIT FOR REVIEW BY STRUCTURAL ENGINEER A CONCRETE POUR SCHEDULE SHOWING LOCATION OF ALL PROPOSED CONSTRUCTION JOINTS.
300.19 PRIOR TO CONCRETE PLACEMENT, THE CONTRACTOR SHALL SUBMIT A CONCRETE MIX DESIGN, PREPARED IN ACCORDANCE WITH THE SPECIFICATIONS, TO THE STRUCTURAL ENGINEER FOR REVIEW.
310 POST-TENSIONING CONCRETE
310.1 REFER TO GENERAL NOTES SECTION 300 REINFORCED CONCRETE FOR GENERAL CONCRETE REQUIREMENTS. THIS SECTION SPECIFICALLY ADDRESSES THE ADDITIONAL REQUIREMENTS FOR POST-TENSIONING CONCRETE.
310.2 MINIMUM Fc REQUIRED AT 28 DAYS: a. POST-TENSIONING CONCRETE: 5000 PSI
310.3 ALL POST-TENSIONING REINFORCING SHALL BE 1/2" DIAMETER 7- WIRE LOW RELAXATION STRANDS CONFORMING TO ASTM A418 AND HAVING THE FOLLOWING MINIMUM PROPERTIES: NOMINAL STRENGTH: 270 KSI ULTIMATE STRENGTH: 0.1531 SO. IN. 28,5000 KSI MODULUS OF ELASTICITY (E)
310.4 POST-TENSIONING REINFORCEMENT FOR SLABS SHALL BE UNBONDED STRAND PER NOTE 310.3 AND SHALL BE ENCASED IN AN APPROVED PLASTIC SHEATHING OF SUFFICIENT TENSILE STRENGTH AND WATER RESISTANCE TO RESIST NON-REPAIRABLE DAMAGE AND DEGRADATION DURING TRANSPORTATION, STORAGE AND INSTALLATION. REINFORCING WITHIN THE SHEATHING SHALL BE PROTECTED AGAINST CORROSION WITH A PROPERLY APPLIED COATING OF APPROVED MATERIAL WHICH SHALL CONFORM WITH ACI RECOMMENDATIONS. THE COATING SHALL BE CHEMICALLY STABLE AND NON-REACTIVE WITH THE CONCRETE AND SHEATHING MATERIAL.
310.5 POST-TENSIONING TENDONS SHALL BE PLACED IN SMOOTH PARABOLIC CURVES IN ACCORDANCE WITH THE PROFILES SHOWN ON THE PLANS. ADEQUATE CHAIRS AND SUPPORT BARS SHALL BE FURNISHED TO HOLD TENDONS IN THEIR CORRECT POSITION DURING THE PLACEMENT OF CONCRETE. TENDON LOCATION SHALL NOT VARY BY MORE THAN 1/4" FROM THEIR SPECIFIED POSITION. SLIGHT VARIATION IN THE SPACING OF TENDONS SHALL BE PERMITTED TO AVOID CONFLICTS WITH OPENINGS AND EMBEDDED ITEMS. WHERE THERE ARE CONFLICTS BETWEEN CONVENTIONAL REINFORCING AND POST-TENSIONING REINFORCING, THE TENDON LOCATION HAS PRIORITY.
310.6 THE REQUIRED POST-TENSIONING FORCES SHOWN ARE EFFECTIVE FORCES AND DO NOT INCLUDE ANY POST-TENSION LOSSES. THE NUMBERS AND SIZES OF THE TENDONS SHALL BE DETERMINED BY THE POST-TENSION REQUIREMENTS OF ACI AND FIT AND SHALL BE SUBJECT TO THE APPROVAL OF THE ARCHITECT. MAXIMUM EFFECTIVE FORCE PER TENDON SHALL NOT EXCEED 26.8 KIPS. TENDONS MAY BE TEMPORARILY DETENSIONED TO A MAXIMUM OF 0.8 FULT AND LOCKED OFF AT A MAXIMUM STRESS OF 0.7 FULT. FRICTION LOSSES SHALL BE BASED ON EXPERIMENTALLY DETERMINED WOBBLE AND CURVATURE COEFFICIENTS AND SHALL BE VERIFIED DURING STRESSING OPERATIONS.
310.7 THE POST-TENSIONING CONTRACTOR SHALL DESIGN, SUPPLY AND INSTALL ALL REQUIRED ANCHORAGE ZONE REINFORCEMENT, BUT IN NO CASE SHALL THAT REINFORCEMENT BE LESS THAN 2-#4 CONT. BARS INSTALLED PARALLEL TO THE EDGE OF SLAB AT ANCHORAGE.
310.8 THE COMPRESSIVE STRENGTH OF CONCRETE AT THE TIME OF TENSIONING POST-TENSIONED REINFORCING SHALL BE A MINIMUM OF 80 PERCENT OF THE SPECIFIED CONCRETE COMPRESSIVE STRENGTH.
310.9 THE POST-TENSIONING CONTRACTOR SHALL SUBMIT CALCULATIONS, TENDON MILL CERTIFICATES AND ANCHORAGE TEST RESULTS TO SUBSTANTIATE THE METHOD OF TENDON CALCULATIONS.
310.10 THE POST-TENSIONING CONTRACTOR SHALL SUBMIT TO THE ARCHITECT FOR REVIEW, DETAILED POST-TENSIONING PROCEDURES AND SEQUENCES, AS WELL AS DETAILED CALCULATIONS TO SUBSTANTIATE THE ABOVE. ALL POST-TENSIONING LOSSES SHALL BE ACCOUNTED FOR IN THE CALCULATIONS.
310.11 A STRESSING RECORD SHALL BE KEPT BY THE STRESSING CREW. THE RECORD SHALL CONTAIN THE FOLLOWING INFORMATION AS A MINIMUM: MEMBER BEING STRESSED; DATE OF STRESSING; MEASURED ELONGATION AT 188,000 PSI; SCHEDULED ELONGATION AT 188,000 PSI; AND THE AMOUNT OF OVERPULL FOR CHUCK SEATING.
310.12 AFTER ACCEPTANCE AND APPROVAL OF STRESSING RECORDS BY THE ARCHITECT, CUT OFF TENDON TAILS ABOUT 1/2" INSIDE THE POCKET. SEAL TENDON TAILS WITH RUST INHIBITING TREATMENT AND DRY PACK ALL POST-TENSIONING POCKETS WITH NON-SHRINK, NON-TERRIBLE GROUT. THE FINAL COLOR AND FINISH OF THE PATCH IN EXPOSED AREAS SHALL MATCH THE ADJACENT CAST-IN-PLACE CONCRETE AS VERIFIED AND APPROVED BY THE ARCHITECT PRIOR TO CONSTRUCTION.
310.13 ALL OPENINGS IN POST-TENSIONED SLABS OR BEAMS SHALL BE FORMED OR SLEEVED. CORING OF POST-TENSIONED BEAMS OR SLABS WILL NOT BE PERMITTED WITHOUT THE WRITTEN CONSENT OF THE ARCHITECT. ALL OPENINGS AND/OR SLEEVES MUST BE SHOWN ON THE SHOP DRAWINGS. ANY ADDITIONAL OPENINGS NOT SHOWN ON THE APPROVED SHOP DRAWINGS WILL REQUIRE A WRITTEN APPROVAL FROM THE ARCHITECT PRIOR TO CONSTRUCTION.
310.14 AFTER THE TENDONS ARE IN THEIR FINAL POSITION, AND PRIOR TO POURING CONCRETE, MARK THE LOCATION OF TENDONS ON THE FORMWORK USING CHALK OR PAINT SUCH THAT THE MARKS WILL BE VISIBLE ON THE UNDERSIDE OF THE SLAB AFTER THE FORMWORK IS REMOVED.
310.15 SEE PROJECT SPECIFICATIONS FOR ADDITIONAL INFORMATION.
320 STUDRAIL
320.1 SHEAR REINFORCEMENT AT THE SLAB COLUMN CONNECTION AS SHOWN ON THE DRAWINGS AND DETAILS SHALL BE STUDRAILS AS MANUFACTURED BY DECOR OR EQUIVALENT.
320.2 THE COMPLETE AND FINISHED STUDRAIL SHALL BE (C80 EVALUATED AND THE WELDING SHALL TAKE PLACE WITHIN AN (C80 APPROVED AND AUDITED FACILITY.
320.3 THE SHEAR STUDS USED IN THE FABRICATION OF THE STUDRAILS SHALL BE LOW CARBON STEEL, E1015 IN ACCORDANCE WITH ASTM-A108. THE STRENGTH AND DUCTILITY REQUIREMENTS SHALL BE AS FOLLOWS: TENSILE STRENGTH: 50,000 PSI MINIMUM 60,000 PSI MINIMUM ELONGATION: 20% IN 2 INCHES REDUCTION OF AREA: 50% MINIMUM
320.4 THE RAILS USED IN THE STUDRAILS SHALL BE LOW CARBON STEEL, TYPE 44W. THE STUDS SHALL BE WELDED IN ACCORDANCE WITH AMERICAN WELDING SOCIETY (AWS) D1.1. THE STRENGTH AND DUCTILITY REQUIREMENTS ARE AS FOLLOWS: YIELD STRENGTH: 44,000 PSI MINIMUM TENSILE STRENGTH: 65,000 PSI MINIMUM ELONGATION: 20% IN 6 INCHES

- 350 CONCRETE/MASONRY ANCHORS
350.1 ALL HEADED CONCRETE ANCHORS SHALL BE NELSON 3/4-INCH DIAMETER, 5-INCH S3L ANCHORS WITH FLUXED ENDS AS MANUFACTURED BY NELSON STUD WELDING COMPANY, UNLESS THE SIZE IS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS.
350.2 ALL HEADED CONCRETE ANCHORS SHALL BE MANUFACTURED FROM MATERIAL WHICH CONFORMS TO ASTM A108, GRADES 1015 THROUGH 1020, HEADED-STUD TYPE, COLD-FINISHED CARBON STEEL, AWS D1.1, TYPE B.
350.3 ALL WELDS SHALL BE MADE IN ACCORDANCE WITH STRUCTURAL WELDING CODE ANSI/AWS D1.1-88 OF THE AMERICAN WELDING SOCIETY AND THE RECOMMENDATIONS OF THE NELSON STUD WELDING COMPANY.
350.4 ALL ADHESIVE STUD ANCHORS SHALL BE "HILTI HIT-HY 150 ADHESIVE CONCRETE ANCHORS" AS MANUFACTURED BY HILTI FASTENING SYSTEMS, INC. (OR EQUAL).
350.5 ALL EXPANSION STUD ANCHORS SHALL BE "HILTI KWIK-BOLT II EXPANSION CONCRETE ANCHORS" AS MANUFACTURED BY HILTI FASTENING SYSTEMS, INC. (OR EQUAL).
350.6 THE "HAS ANCHOR ROD" SHALL CONFORM TO ASTM A36 STEEL, THE "HAS SUPER ANCHOR ROD" SHALL CONFORM TO ASTM A193 STEEL, THE "HAS STANDARD NUT" SHALL CONFORM TO ASTM A563, GRADE A, THE "HAS SUPER ANCHOR ROD NUT" SHALL CONFORM TO ASTM A 563, GRADE H.
350.7 THE "KWIK-BOLT II EXPANSION ANCHORS" STUD SHALL CONFORM TO ASTM A510 OR ASTM A108 STEEL AND THE NUT SHALL CONFORM TO ASTM A563, GRADE A.
350.8 ALL EPOXY ADHESIVE ANCHORS FOR ANCHORING TO HOLLOW MASONRY SHALL BE "HILTI HIT-HY20 ADHESIVE ANCHORS" AS MANUFACTURED BY HILTI FASTENING SYSTEMS, INC. (OR EQUAL).
350.9 ALL EXPANSION ANCHORS FOR ANCHORING TO MASONRY SHALL BE "HILTI HLG SLEVE ANCHORS" AS MANUFACTURED BY HILTI FASTENING SYSTEMS, INC. (OR EQUAL).
350.10 THE SPACING, MINIMUM EMBEDMENT AND INSTALLATION OF THE ANCHORS SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDED PROCEDURES.
420 MASONRY
420.1 ALL MASONRY WORK SHALL BE IN CONFORMANCE WITH THE LATEST EDITION OF "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" (ACI 530) AND THE "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI 530.1) OF THE AMERICAN CONCRETE INSTITUTE.
420.2 ALL MASONRY WORK TO BE EXECUTED IN COLD WEATHER SHALL BE IN CONFORMANCE WITH THE RECOMMENDATIONS FOR COLD WEATHER CONSTRUCTION OF THE LATEST EDITION OF "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" (ACI 530) AND THE "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI 530.1) OF THE AMERICAN CONCRETE INSTITUTE WITH THE FOLLOWING ADDITION TO THE REQUIREMENTS OF ACI 530.1 SECTION 1.8-B: FOR ALL CONDITIONS WHEN TEMPERATURES FALL BELOW 40 DEGREES F, THE TEMPERATURE OF THE NEWLY LAID MASONRY OR NEWLY GROUTED MASONRY SHALL BE MAINTAINED ABOVE 32 DEGREES (F) FOR A MINIMUM OF 24 HOURS USING THE METHODS DESCRIBED IN ACI 530.1.
420.3 MORTAR SHALL CONFORM TO THE PROPORTION SPECIFICATION OF ASTM C270, TYPE M OR S. PROVIDE TYPE M MORTAR AT ALL HIGH STRENGTH MASONRY NOTED AS Fm = 2500 PSI OR GREATER, PROVIDE TYPE S MORTAR AT ALL STRUCTURAL MASONRY AND REINFORCED MASONRY UNLESS NOTED OTHERWISE.
420.4 GROUT: a. Fc OF GROUT SHALL BE 8 TO 11 INCHES AS MEASURED b. ACCORDING TO ASTM C143. c. MAX. AGGREGATE SIZE SHALL BE 3/8" (AGGREGATE GRADED TO PRODUCE FINE GROUT IN CONFORMANCE WITH ASTM C478 AND C404).
420.5 LIMIT CEMENTITIOUS MATERIALS IN MORTAR TO: PORTLAND CEMENT CONFORMING TO ASTM C150 TYPE I; LIME CONFORMING TO ASTM C207; MORTAR CEMENT CONFORMING TO ASTM C1329
420.6 CONCRETE BLOCK UNITS: a. SOLID AND HOLLOW LOAD BEARING UNITS PER ASTM C90, TYPE N- II, AS REQUIRED TO PROVIDE Fm AS NOTED BELOW.
420.7 MINIMUM 28-DAY ULTIMATE COMPRESSIVE STRENGTH OF MASONRY: a. Fm 1500 PSI
420.8 HORIZONTAL JOINT REINFORCING FOR ALL EXTERIOR AND LOAD BEARING WALLS SHALL BE GALVANIZED TRUSS OR LADDER TYPE DUR-O-WALL OR EQUAL AS APPROVED BY THE ENGINEER WITH 2-9 GAGE LONGITUDINAL WIRE AND 9 GAGE CROSS WIRE, SPACED AT 18 INCHES CENTER TO CENTER, UNLESS NOTED OTHERWISE. PROVIDE ADDITIONAL LAYERS OF JOINT REINFORCEMENT IN THE FIRST TWO COURSES ABOVE AND BELOW A MASONRY OPENING. PROVIDE LAP AS RECOMMENDED BY MANUFACTURER WITH A MINIMUM OF 8". DISCONTINUE JOINT REINFORCING AT CONTROL JOINTS. PROVIDE "L" SHAPE AND "T" SHAPE DUR-O-WALL AT ALL INTERSECTION CORNERS WITH 8" MINIMUM LAP. SEE TYPICAL DETAILS.
420.9 FULL BED AND HEAD JOINTS SHALL BE USED.
420.10 REINFORCED MASONRY UNITS SHALL BE FILLED SOLID WITH 3000 PSI GROUT AT ALL CELLS CONTAINING REINFORCING AND WHERE SHOWN ON PLANS. GROUT AT ALL CELLS BELOW GRADE SOLID.
420.11 PROVIDE FINE GROUT PER ASTM C478 WHEN WIDTH OF GROUT SPACE IS LESS THAN 2". PROVIDE COARSE GROUT FOR GROUT SPACE WIDTHS 2" OR GREATER. PROVIDE FINE GROUT WHEN REINFORCING HAS LESS THAN 1/2" CLEARANCE.
420.12 DEFORMED BAR REINFORCEMENT SHALL CONFORM TO ASTM A615, GRADE 60. PROVIDE BAR SPACERS AS REQUIRED TO PROPERLY LOCATE REINFORCING. LAP SPLICES SHALL BE AS FOLLOWS: 8" CML
SINGLY REINFORCED DOUBLY REINFORCED
#3 (#10) Fm 1500 Fm 2000 Fm 1500 Fm 2000
#4 (#13) 19 20 19 18
#5 (#16) 25 21 22 22
#6 (#19) 31 27 27 27
#8 (#19) 57 50 47 41
#8 (#19) 57 50 47 41
510 STRUCTURAL STEEL
510.1 ALL STRUCTURAL STEEL WORK SHALL BE IN ACCORDANCE WITH THE "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS" (1989 EDITION, AS REVISED) OF THE AISC.
510.2 GRADE OF STEEL: a. STRUCTURAL "W" SHAPES: ASTM A992 b. STRUCTURAL "M", "S", "HP" SHAPES: ASTM A572, GRADE 50 c. CHANNELS: ASTM A572, GRADE 60 d. STEEL TUBES (HSS SHAPES): ASTM A500, GRADE B e. STEEL PIPE (ROUND SHAPES): ASTM A500, GRADE B f. ANGLES AND PLATES: ASTM A36
510.3 ALL BOLTS SHALL BE ASTM A325, 3/4-INCH DIAMETER, UNLESS NOTED OTHERWISE.
510.4 ALL WELDING SHALL BE IN ACCORDANCE WITH THE STRUCTURAL WELDING CODE, AWS D1.1, LATEST EDITION, OF THE AMERICAN WELDING SOCIETY. ELECTRODES SHALL BE E70XX FOR MANUAL ARC WELDING AND F7X-EXXX FOR SUBMERGED ARC WELDING.

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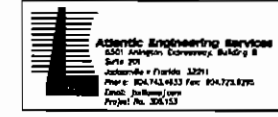
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GENERAL NOTES
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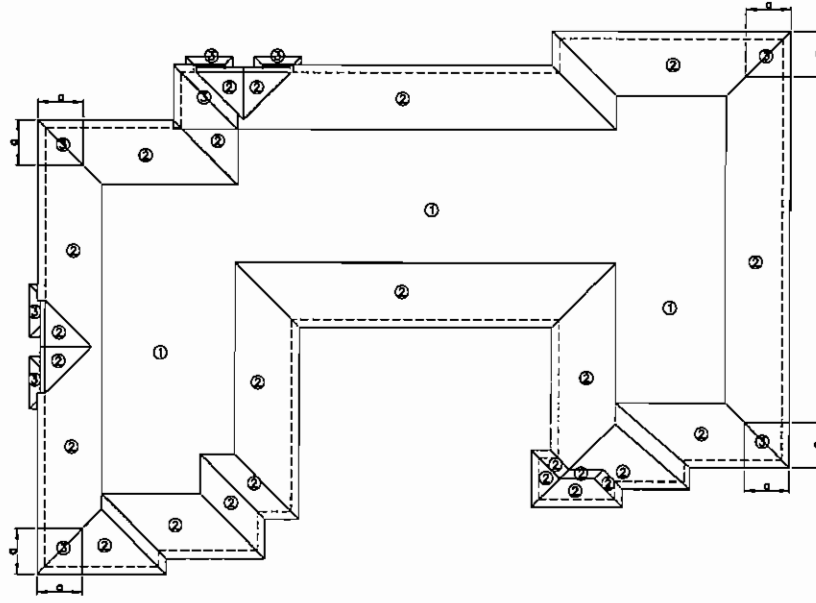
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- 510.5 ALL BEAM TO COLUMN CONNECTIONS SHALL BE DESIGNED FOR THE SUMMATION OF THE FOLLOWING LOADS:
- FOR ONE HALF THE UNIFORM LOAD CAPACITY OF THE MEMBER IN ACCORDANCE WITH AISC SPECIFICATIONS, BUT NOT LESS THAN 8 KIIPS.
  - A 10.0 KIP AXIAL FORCE (ACTING IN TENSION AND COMPRESSION), THE EFFECTS OF CONCENTRATED LOADS OCCURRING CLOSE TO THE ENDS OF THE BEAMS.
- 510.6 ALL NON-COMPOSITE BEAM END CONNECTIONS SHALL BE DESIGNED, UNLESS NOTED OTHERWISE, FOR AN END REACTION "R" EQUAL TO NOT LESS THAN ONE HALF THE UNIFORM LOAD CAPACITY OF THE MEMBER IN ACCORDANCE WITH AISC SPECIFICATIONS, BUT NOT LESS THAN 8 KIIPS. THE EFFECTS OF CONCENTRATED LOADS OCCURRING CLOSE TO THE ENDS OF THE BEAMS SHALL BE CONSIDERED IN THE CONNECTION DESIGN.
- 510.7 CUTS, HOLES AND COPING, ETC. REQUIRED FOR OTHER TRADES SHALL BE SHOWN ON THE SHOP DRAWING AND MADE IN THE SHOP. CUTS OR BURNING OF HOLES IN STRUCTURAL STEEL IN THE FIELD WILL NOT BE PERMITTED.
- 510.8 ALTERNATE CONNECTION DETAILS MAY BE USED IF SUCH DETAILS ARE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL. HOWEVER, THE ENGINEER SHALL BE THE SOLE JUDGE OF ACCEPTANCE AND THE CONTRACTOR'S BID SHALL ANTICIPATE THE USE OF THOSE SPECIFIED DETAILS SHOWN ON THE DRAWINGS. THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN OF SUCH ALTERNATE DETAILS WHICH HE PROPOSES.
- 510.9 ALL STRUCTURAL STEEL WORK, EXCEPT PORTIONS OF MEMBERS TO BE WELDED, FIELD BOLTED OR FIREPROOFED, SHALL BE SHOP PAINTED WITH PAINT CONFORMING TO STEEL STRUCTURES PAINTING COUNCIL (SSPC) PAINT 23. APPLY PRIME PAINT ACCORDING TO SSPC PAINT SYSTEM GUIDE NO. 7.00. CLEAN STEEL FREE OF LOOSE SCALE, RUST, OIL AND GREASE. ADDITIONAL AREAS SHALL BE FIELD PAINTED AFTER WELDING.
- 510.10 BOLT AND WELD TESTING:
- ALL SHOP AND FIELD BOLTS SHALL BE TESTED PER AISC REQUIREMENTS.
  - ALL WELDS SHOULD BE VISUALLY INSPECTED.
  - TEN PERCENT OF ALL WELDS AT BEAM AND GIRDER SHEAR CONNECTIONS SHALL BE RANDOMLY INSPECTED BY MAGNETIC PARTICLE METHOD, COMPLYING WITH ASTM E109, PERFORMED ON ROOT PASS AND ON FINISHED WELD.
  - ONE HUNDRED PERCENT OF FULL PENETRATION WELDS SHALL HAVE ULTRASONIC INSPECTION, COMPLYING WITH ASTM E164.
  - ONE HUNDRED PERCENT OF WELDS IN BEAM AND COLUMN MOMENT CONNECTIONS SHALL HAVE ULTRASONIC INSPECTION, COMPLYING WITH ASTM E164.
- 610.11 ALL EXTERIOR STRUCTURAL STEEL SHALL BE NOT-DIP GALVANIZED IN ACCORDANCE WITH THE GENERAL NOTES.
- 510.12 LEVELING GROUT SHALL BE NON-SHRINK, NON-METALLIC TYPE, FACTORY PRE-MIXED GROUT TESTED IN ACCORDANCE WITH CE-CRD-0821 OR ASTM C106, WITH  $f_c$  OF NOT LESS THAN 5000 PSI.
- 510.13 ALL ANCHOR BOLTS SHALL BE ASTM A307 OR ASTM F1554, GRADE 36 AND A MINIMUM 3/4" DIAMETER UNLESS NOTED OTHERWISE.
- 510.14 PROVIDE TAPERED SHIMS, ANGLES, BENT PLATES, OR OTHER STEEL ACCESSORIES TO FACILITATE BEARING CONNECTIONS AND DECK SUPPORT FOR SLOPING OR GATED STRUCTURAL STEEL AS REQUIRED.
- 530 METAL DECKING
- 530.1 ALL STEEL ROOF DECK SHALL BE IN CONFORMANCE WITH THE "STEEL ROOF DECK SPECIFICATIONS AND LOAD TABLES" OF THE "STEEL DECK INSTITUTE, LATEST EDITION.
- 530.2 ALL STEEL ROOF DECK SHALL BE, AS A MINIMUM, 1-1/2 INCH, 22-GAUGE, WIDE-RIBBED STEEL DECK, WITH A YIELD STRENGTH OF NOT LESS THAN 33,000 PSI AND SHALL BE NOT-DIPPED GALVANIZED.
- 530.3 STEEL ROOF DECK SHALL BE CAPABLE OF SUPPORTING 35 POUNDS PER SQUARE FOOT AT THE SPANS INDICATED ON THE DRAWINGS OR AT A SPAN OF 8'-0", WHICHEVER IS GREATER.
- 530.4 ALL STEEL ROOF DECK SHALL BE FASTENED TO THE SUPPORTING LIGHT-GAUGE STRUCTURAL STEEL WITH #12 TEK SCREWS AT THE ENDS OF UNITS AND AT ALL INTERMEDIATE SUPPORTS WITH A MAXIMUM SPACING OF 12 INCHES ON CENTER ACROSS THE WIDTH OF THE DECK. SIDE DECK LAPS SHALL BE FASTENED WITH #10 TEK SCREWS AT THIRD POINTS BETWEEN SUPPORTS WITH A SPACING NOT TO EXCEED 2'-0" ON CENTER. ATTACHMENT OF DECK TO LIGHT-GAUGE STRUCTURE SHALL BE CAPABLE OF RESISTING A DIAPHRAGM SHEAR FORCE OF 140 PLF. ANY SPLIT OR PARTIAL PANELS SHALL BE FASTENED TO THE SUPPORTING STRUCTURE IN EVERY VALLEY REGARDLESS OF ADJACENT FASTENER PATTERNS.
- 530.5 ALL DECK SHALL BE A MINIMUM OF THREE SPANS CONTINUOUS.
- 530.6 DECKING CONTRACTOR SHALL COORDINATE OPENING SIZES AND LOCATIONS IN FLOORS AND ROOFS FROM ARCHITECTURAL AND MECHANICAL DRAWINGS. HE SHALL PROVIDE HEADER MEMBERS IF REQUIRED AS PER THE TYPICAL DETAILS.
- 530.7 DECKING CONTRACTOR SHALL PROVIDE SCREED ANGLES AND CLOSURE PLATES AS REQUIRED AT THE EDGES OF ALL FLOOR OPENINGS AND AT ALL SLAB DEPRESSIONS OR CHANGES OF DECK DIRECTION, WHICH HAVE NOT BEEN DETAILED.
- 540 COLD FORMED STEEL FRAMING
- 540.1 THE LIGHT GAUGE FRAMING SYSTEMS INCLUDING CONNECTIONS SHALL BE DESIGNED, ENGINEERED AND CONSTRUCTED TO WITHSTAND, AS A MINIMUM, LOADS FROM DRAVITY, SNOW, WIND, HANDLING AND ERECTION, SHORT TERM AND LONG TERM DEFLECTION OF THE BUILDING, AND THERMAL MOVEMENT. THE ATTACHMENT OF THE LIGHT GAUGE FRAMING TO THE STRUCTURE SHALL ALSO BE DESIGNED BY THE DESIGNATED LIGHT GAUGE FRAMING ENGINEER AND IS SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER.
- 540.2 ALL SUPPORTS, BRACING, AND CONNECTIONS REQUIRED FOR THE INSTALLATION THE LIGHT GAUGE FRAMING SYSTEMS THAT ARE NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED AND PROVIDED BY THE LIGHT GAUGE FRAMING SYSTEMS FABRICATOR AND ARE SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER.
- 540.3 THE LIGHT GAUGE FRAMING FABRICATOR SHALL SUBMIT STRUCTURAL CALCULATIONS AND SHOP DRAWINGS SIGNED AND SEALED BY A DESIGNATED PROFESSIONAL ENGINEER FOR THE "LIGHT GAUGE FRAMING SYSTEMS". THE DESIGNATED LIGHT GAUGE FRAMING SYSTEMS ENGINEER SHALL BE LICENSED TO PRACTICE IN THE STATE OF FLORIDA AND HAVE 5 YEARS OF EXPERIENCE IN THE SUCCESSFUL DESIGN OF LIGHT GAUGE FRAMING.
- 540.4 LIGHT-GAUGE STEEL FRAMING MEMBERS AND CONNECTIONS SHALL CONFORM TO THE MOST CURRENT VERSION OF "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" BY THE AMERICAN IRON AND STEEL INSTITUTE.
- 540.5 ALL WELDING SHALL BE IN ACCORDANCE WITH THE "STRUCTURAL WELDING CODE-SHEET METAL: AWS D.1.3, MOST CURRENT EDITION, OF THE AMERICAN WELDING SOCIETY.
- 540.6 STEEL USED IN THE MANUFACTURE SHALL BE HOT-DIPPED GALVANIZED STEEL, G-80 MINIMUM COATING WEIGHT AND SHALL CONFORM TO ASTM A953 GRADE D, MINIMUM YIELD POINT OF 50,000 PSI FOR 12-, 14- AND 18- GAUGE MEMBERS AND ASTM A953 GRADE A, MINIMUM YIELD POINT OF 40,000 PSI FOR 18- AND 20-GAUGE MEMBERS.
- 540.7 PROVIDE STUD BRIDGING IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS FOR LATERALLY LOADED WALLS, BUT AT SPACING NOT TO EXCEED 4'-0" VERTICALLY.
- 540.8 ALL FRAMING COMPONENTS SHALL BE CUT SQUARELY FOR ATTACHMENT TO PERPENDICULAR MEMBERS OR AS REQUIRED FOR AN ANGULAR FIT AGAINST ADJUTING MEMBERS. MEMBERS SHALL BE HELD POSITIVELY IN PLACE UNTIL PROPERLY FASTENED.

- 540.9 ALL FIELD CUTTING OF STUDS MUST BE DONE BY SAWING OR SHEARING. TORCH CUTTING OF COLD-FORMED MEMBERS IS UNACCEPTABLE.
- 540.10 NO SPLICES IN STUDS, JOISTS, OR OTHER LOAD CARRYING MEMBERS MAY BE MADE WITHOUT PRIOR REVIEW BY STRUCTURAL ENGINEER AND SPECIFIC DETAILS FOR ANY SUCH SPLICE(S).
- 540.11 FRAME BOTH SIDES OF EXPANSION JOINTS WITH SEPARATE STUDS. DO NOT BRIDGE THE EXPANSION JOINTS WITH STUD SYSTEM COMPONENTS.
- 540.12 DEFLECTION OF ALL LIGHT GAUGE STEEL FRAMING MEMBERS WHICH SUPPORT MASONRY VENEER SHALL NOT EXCEED THE STUD/JOIST HEIGHT/SPAN IN INCHES DIVIDED BY 720.
- 540.13 PROVIDE VERTICAL SLIDE CONNECTION OR DEFLECTION TRACK AT THE TOP OF EACH STUD AS REQUIRED FOR DEFLECTION OF THE STRUCTURE AND THERMAL MOVEMENT.
- 540.14 PROVIDE DOUBLE STUDS, OR DOUBLE PAIRS OF STUDS WHERE APPLICABLE, AT THE JAMBS OF ALL DOOR AND WINDOW OPENINGS WHICH EXCEED 24 INCHES. A MINIMUM OF A STUD AND A TRACK SHALL BE PROVIDED AT ALL SILLS AND HEADS OF OPENINGS.
- 540.15 THE DEFLECTION OF THE LIGHT GAUGE FRAMING MEMBERS DUE TO THE TOTAL LOADS SHALL NOT EXCEED THE SPAN LENGTH DIVIDED BY 600.
- 540.16 REQUESTS FOR ALLOWABLE SUBSTITUTIONS FOR THE ABOVE NOTED LIGHT GAUGE FRAMING SYSTEMS SHALL BE SUBJECT TO THE REVIEW OF THE ARCHITECT AND ENGINEER.
- 545 LIGHT GAUGE METAL TRUSSES
- 545.1 LIGHT GAUGE METAL TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE AMERICAN IRON AND STEEL INSTITUTE (AISI) "SPECIFICATION FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS".
- 545.2 LIGHT GAUGE METAL TRUSSES SHALL BE DESIGNED TO SUPPORT THE LOADS INDICATED BELOW AT THE SPACING INDICATED ON THE DRAWINGS.
- DEAD LOADS:
    - UNIFORM TOP CHORD DEAD LOAD 15 PSF
    - UNIFORM BOTTOM CHORD DEAD LOAD 3 PSF
  - LIVE LOADS: SEE DESIGN CRITERIA GENERAL NOTES
  - LATERAL LOADS AND UPLIFT: SEE DESIGN CRITERIA GENERAL NOTES
- 545.3 THE DEFLECTION OF THE ROOF TRUSSES UNDER THE INDICATED LOADS AT THE SPAN AND SPACINGS SHOWN ON THE CONTRACT DRAWINGS SHALL MEET THE FOLLOWING CRITERIA:
- THE DEFLECTION DUE TO LIVE LOAD SHALL NOT EXCEED THE SPAN LENGTH/360.
  - THE DEFLECTION DUE TO THE TOTAL LOADS SHALL NOT EXCEED THE SPAN LENGTH/240.
- 545.4 STEEL USED TO FABRICATE THE LIGHT GAUGE METAL TRUSSES SHALL BE HOT-DIPPED GALVANIZED STEEL, G-80 MINIMUM COATING WEIGHT AND SHALL CONFORM TO ASTM A448 GRADE D, MINIMUM YIELD POINT OF 50,000 PSI FOR 12, 14 AND 18 GAUGE MEMBERS AND ASTM A448 GRADE A, MINIMUM YIELD POINT OF 40,000 PSI FOR 18 AND 20 GAUGE MEMBERS.
- 545.5 THE LIGHT GAUGE METAL TRUSS MANUFACTURER SHALL SPECIFY AND PROVIDE ALL BRACING AT TOP AND BOTTOM CHORDS REQUIRED TO STABILIZE THE ROOF STRUCTURE DURING AND AFTER CONSTRUCTION, IN ADDITION TO THE BRACING INDICATED ON THE STRUCTURAL DRAWINGS.
- 545.6 THE LIGHT GAUGE METAL TRUSS MANUFACTURER SHALL SUBMIT STRUCTURAL CALCULATIONS STAMPED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED TO PRACTICE IN THE STATE OF FLORIDA FOR ALL TRUSS TYPES, WHICH INDICATE TRUSS CAPACITIES AND DEFLECTIONS.
- 545.7 TRUSSES MUST BE SHOP FABRICATED
- 545.8 ALL WELDING SHALL BE IN ACCORDANCE WITH THE "STRUCTURAL WELDING CODE-SHEET METAL: AWS D.1.3, MOST CURRENT EDITION, OF THE AMERICAN WELDING SOCIETY.
- 545.9 NO SPLICES IN WEBS, CHORDS, OR OTHER LOAD CARRYING MEMBERS MAY BE MADE WITHOUT REVIEW AND APPROVAL ON THE FINAL TRUSS SHOP DRAWINGS WHICH INCLUDE SPECIFIC LOCATIONS AND DETAILS FOR ANY SUCH SPLICE(S).
- 545.10 FRAME BOTH SIDES OF EXPANSION JOINTS WITH SEPARATE TRUSS AND BRACING SYSTEMS. DO NOT BRIDGE EXPANSION JOINTS WITH TRUSS AND BRACING SYSTEMS.
- 545.11 THE LIGHT GAUGE METAL TRUSS MANUFACTURER SHALL DESIGN / SPECIFY ALL CONNECTIONS AND CONNECTION COMPONENTS BETWEEN LIGHT GAUGE METAL TRUSSES AND BEARING CONNECTIONS OF THE LIGHT GAUGE METAL TRUSSES TO THE SUPPORTING STRUCTURE. ALL CONNECTIONS AND CONNECTION COMPONENTS SHALL BE INDICATED ON THE FINAL SHOP DRAWINGS.



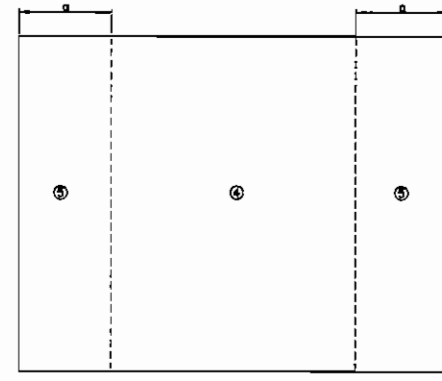
COMPONENTS AND CLADDING ROOF DIAGRAM  
SCALE: 1/8"=1'-0"

ROOF OVERHANG COMPONENT AND CLADDING DESIGN WIND PRESSURES

COMPONENTS AND CLADDING WIND LOADS FOR EXPOSURE C		
ZONE	EFFECTIVE WIND AREA (FT <sup>2</sup> )	WIND PRESSURES
ROOF ANGLE 10-30 DEGREES		120MPH
2	10.0	-67.8
2	20.0	-67.8
2	100.0	-67.8
3	10.0	-113.7
3	20.0	-100.8
3	100.0	-70.7

STRUCTURAL ABBREVIATIONS:

- |                                   |                                     |
|-----------------------------------|-------------------------------------|
| ARCH — ARCHITECT(URAL)            | NS — NEAR SIDE                      |
| CJ — CONSTRUCTION JOINT           | ND/# — NUMBER                       |
| CMU — CONCRETE MASONRY UNIT       | NTS — NOT TO SCALE                  |
| CL — CENTER LINE                  | NW — NORMAL WEIGHT                  |
| CLR — CLEAR                       | OC — ON CENTER                      |
| CONT — CONTINUOUS                 | OPP — OPPOSITE                      |
| COORD — COORDINATE                | PERP — PERPENDICULAR                |
| DA — DIAMETER                     | PLF — POUNDS PER LINEAR FOOT        |
| EF — EACH FACE                    | PSF — POUNDS PER SQUARE FOOT        |
| EJ — EXPANSION JOINT              | PSI — POUNDS PER SQUARE INCH        |
| EO — EDGE OF DECK                 | PT — PRESSURE TREATED/ POST TENSION |
| EOS — EDGE OF SLAB                | REINF — REINFORCED/REINFORCING      |
| EL — ELEVATION                    | REF — REFERENCE                     |
| EMBED — EMBEDMENT/EMBEDDED        | SF — SQUARE FEET                    |
| EQ — EQUAL                        | SM — SIMILAR                        |
| ETC — ETCETERA                    | SJ — SAW CUT CONTRACTION JOINT      |
| EXIST — EXISTING                  | SPEC — SPECIFICATION                |
| FS — FAR SIDE                     | SS — STAINLESS STEEL                |
| GC — GENERAL CONTRACTOR           | STD — STANDARD                      |
| GA — GAGE                         | SYM — SYMMETRICAL                   |
| GALV — GALVANIZED                 | TD — TURNDOWN                       |
| HORIZ — HORIZONTAL                | TOB — TOP OF BEAM                   |
| HK — HOOK                         | TOC — TOP OF CONCRETE               |
| K — KIIPS                         | TOF — TOP OF FOOTING                |
| KLF — KIIPS PER LINEAR FOOT       | TOJ — TOP OF JOIST                  |
| KSF — KIIPS PER SQUARE FOOT       | TOS — TOP OF STEEL                  |
| KSI — KIIPS PER SQUARE INCH       | TOW — TOP OF WALL                   |
| LONG — LONG LEG                   | TS — TUBULAR STEEL                  |
| LLH — LONG LEG HORIZONTAL         | TYP — TYPICAL                       |
| LLV — LONG LEG VERTICAL           | UNO — UNLESS NOTED OTHERWISE        |
| LW — LIGHTWEIGHT                  | VCJ — VERTICAL CONTROL JOINT        |
| MBM — METAL BUILDING MANUFACTURER | VERT — VERTICAL                     |
| MC — MOMENT CONNECTION            | WP — WORK POINT                     |
| MANUF — MANUFACTURER(ING)         | WWM — WELDED WIRE MESH              |
| MAX — MAXIMUM                     | XS — EXTRA STRONG                   |
| MEDH — MECHANICAL                 | XDS — DOUBLE EXTRA STRONG           |
| MIN — MINIMUM                     |                                     |
| MISC — MISCELLANEOUS              |                                     |
| MO — MASONRY OPENING              |                                     |



COMPONENTS AND CLADDING WALL DIAGRAM  
SCALE: 1/8"=1'-0"

COMPONENTS AND CLADDING WIND LOADS FOR EXPOSURE C

COMPONENTS AND CLADDING WIND LOADS FOR EXPOSURE C		
ZONE	EFFECTIVE WIND AREA (FT <sup>2</sup> )	WIND PRESSURES
ROOF ANGLE 10-30 DEGREES		120MPH
1	10.0	+26.3 -41.8
1	20.0	+24.0 -40.6
1	50.0	+20.9 -39.1
1	100.0	+18.6 -37.9
2	10.0	+26.3 -72.6
2	20.0	+24.0 -67.0
2	50.0	+20.9 -58.2
2	100.0	+18.6 -53.4
3	10.0	+26.3 -107.6
3	20.0	+24.0 -100.6
3	50.0	+20.9 -91.3
3	100.0	+18.6 -84.4
WALL		
4	10.0	+46.7 -46.5
4	20.0	+43.7 -47.6
4	50.0	+41.0 -44.0
4	100.0	+38.7 -42.6
5	10.0	+45.7 -81.1
5	20.0	+43.7 -58.6
5	50.0	+41.0 -51.5
5	100.0	+38.7 -47.8

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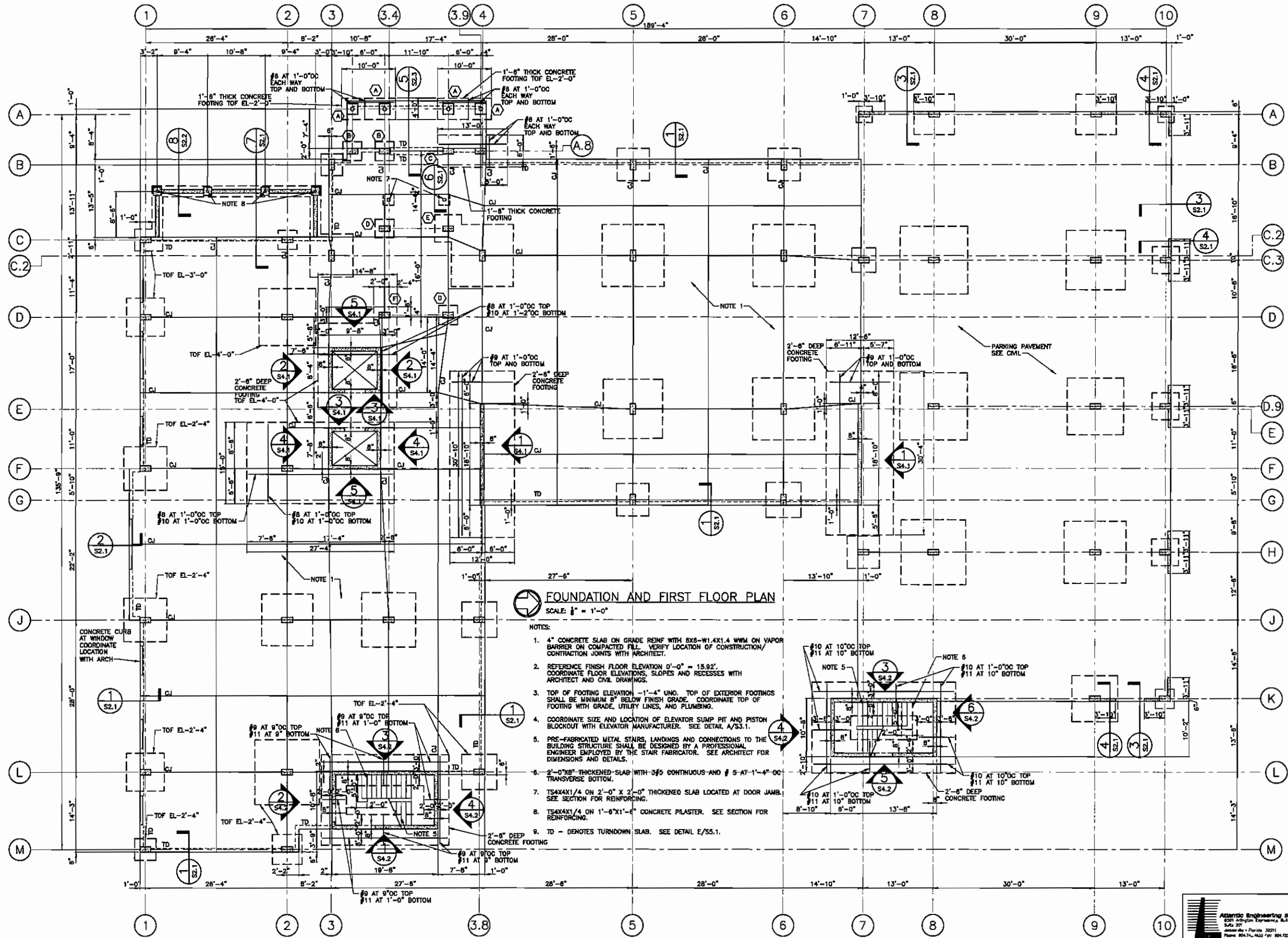
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GENERAL NOTES AND COMPONENTS AND CLADDING  
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JACKSONVILLE BEACH, FLORIDA

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S0.2



**FOUNDATION AND FIRST FLOOR PLAN**  
 SCALE: 1/8" = 1'-0"

- NOTES:
- 4" CONCRETE SLAB ON GRADE REINF WITH 6X8-W1.4X1.4 W/M ON VAPOR BARRIER ON COMPACTED FILL. VERIFY LOCATION OF CONSTRUCTION/CONTRACTION JOINTS WITH ARCHITECT.
  - REFERENCE FINISH FLOOR ELEVATION 0'-0" = 15.92'. COORDINATE FLOOR RECESSES, SLOPES AND RECESSES WITH ARCHITECT AND CIVIL DRAWINGS.
  - TOP OF FOOTING ELEVATION -1'-4" UNO. TOP OF EXTERIOR FOOTINGS SHALL BE MINIMUM 8" BELOW FINISH GRADE. COORDINATE TOP OF FOOTING WITH GRADE, UTILITY LINES, AND PLUMBING.
  - COORDINATE SIZE AND LOCATION OF ELEVATOR SUMP PIT AND PISTON BLOCKOUT WITH ELEVATOR MANUFACTURER. SEE DETAIL A/S3.1.
  - PRE-FABRICATED METAL STAIRS, LANDINGS AND CONNECTIONS TO THE BUILDING STRUCTURE SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER EMPLOYED BY THE STAIR FABRICATOR. SEE ARCHITECT FOR DIMENSIONS AND DETAILS.
  - 2'-0" THICKENED SLAB WITH 3#5 CONTINUOUS AND #5 AT 1'-4" OC TRANSVERSE BOTTOM.
  - TS4X4X1/4 ON 2'-0" X 2'-0" THICKENED SLAB LOCATED AT DOOR JAMB. SEE SECTION FOR REINFORCING.
  - TS4X4X1/4 ON 1'-6" X 1'-6" CONCRETE PILASTER. SEE SECTION FOR REINFORCING.
  - TD - DENOTES TURNDOWN SLAB. SEE DETAIL E/S5.1.

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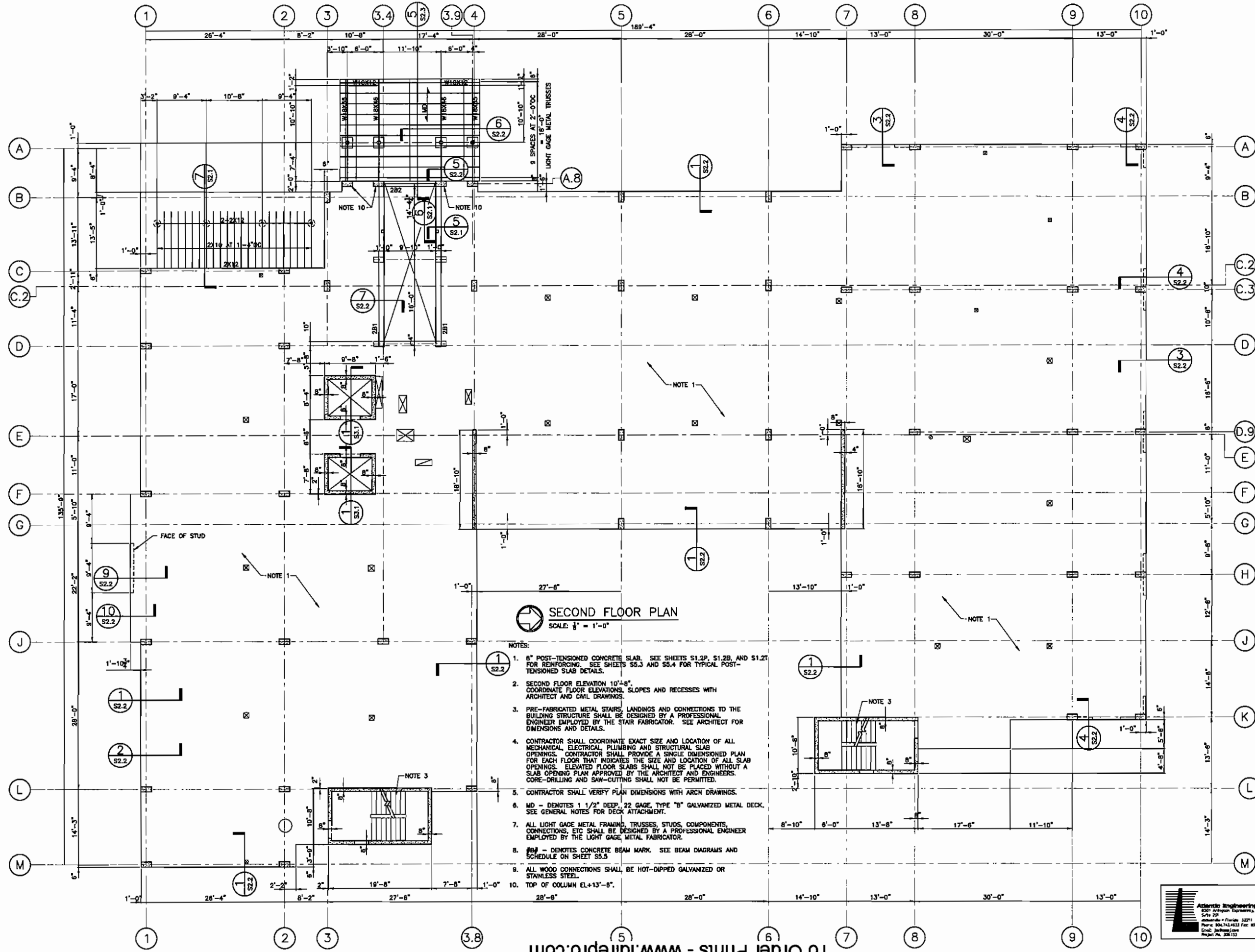
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**FOUNDATION AND FIRST FLOOR PLAN**  
**Holiday Inn EXPRESS**  
 JACKSONVILLE BEACH, FLORIDA

3748 PE  
 MARK J. KEISTER

PROJECT NO. 025  
 ISSUE DATE: MAR 2, 2007  
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**S1.1**



**SECOND FLOOR PLAN**  
SCALE: 1/8" = 1'-0"

- NOTES:
1. 8" POST-TENSIONED CONCRETE SLAB. SEE SHEETS S1.2P, S1.2B, AND S1.2T FOR REINFORCING. SEE SHEETS S5.3 AND S5.4 FOR TYPICAL POST-TENSIONED SLAB DETAILS.
  2. SECOND FLOOR ELEVATION 10'-4". COORDINATE FLOOR ELEVATIONS, SLOPES AND RECESSES WITH ARCHITECT AND CIVIL DRAWINGS.
  3. PRE-FABRICATED METAL STAIRS, LANDINGS AND CONNECTIONS TO THE BUILDING STRUCTURE SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER EMPLOYED BY THE STAIR FABRICATOR. SEE ARCHITECT FOR DIMENSIONS AND DETAILS.
  4. CONTRACTOR SHALL COORDINATE EXACT SIZE AND LOCATION OF ALL MECHANICAL, ELECTRICAL, PLUMBING AND STRUCTURAL SLAB OPENINGS. CONTRACTOR SHALL PROVIDE A SINGLE DIMENSIONED PLAN FOR EACH FLOOR THAT INDICATES THE SIZE AND LOCATION OF ALL SLAB OPENINGS. ELEVATED FLOOR SLABS SHALL NOT BE PLACED WITHOUT A SLAB OPENING PLAN APPROVED BY THE ARCHITECT AND ENGINEERS. CORE-DRILLING AND SAW-CUTTING SHALL NOT BE PERMITTED.
  5. CONTRACTOR SHALL VERIFY PLAN DIMENSIONS WITH ARCH DRAWINGS.
  6. MD - DENOTES 1 1/2" DEEP, 22 GAGE, TYPE "B" GALVANIZED METAL DECK. SEE GENERAL NOTES FOR DECK ATTACHMENT.
  7. ALL LIGHT GAGE METAL FRAMING, TRUSSES, STUDS, COMPONENTS, CONNECTIONS, ETC SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER EMPLOYED BY THE LIGHT GAGE METAL FABRICATOR.
  8. #B - DENOTES CONCRETE BEAM MARK. SEE BEAM DIAGRAMS AND SCHEDULE ON SHEET S5.5
  9. ALL WOOD CONNECTIONS SHALL BE HOT-DIPPED GALVANIZED OR STAINLESS STEEL.
  10. TOP OF COLUMN EL+13'-8".

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**S1.2**

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MARK J. WESTER

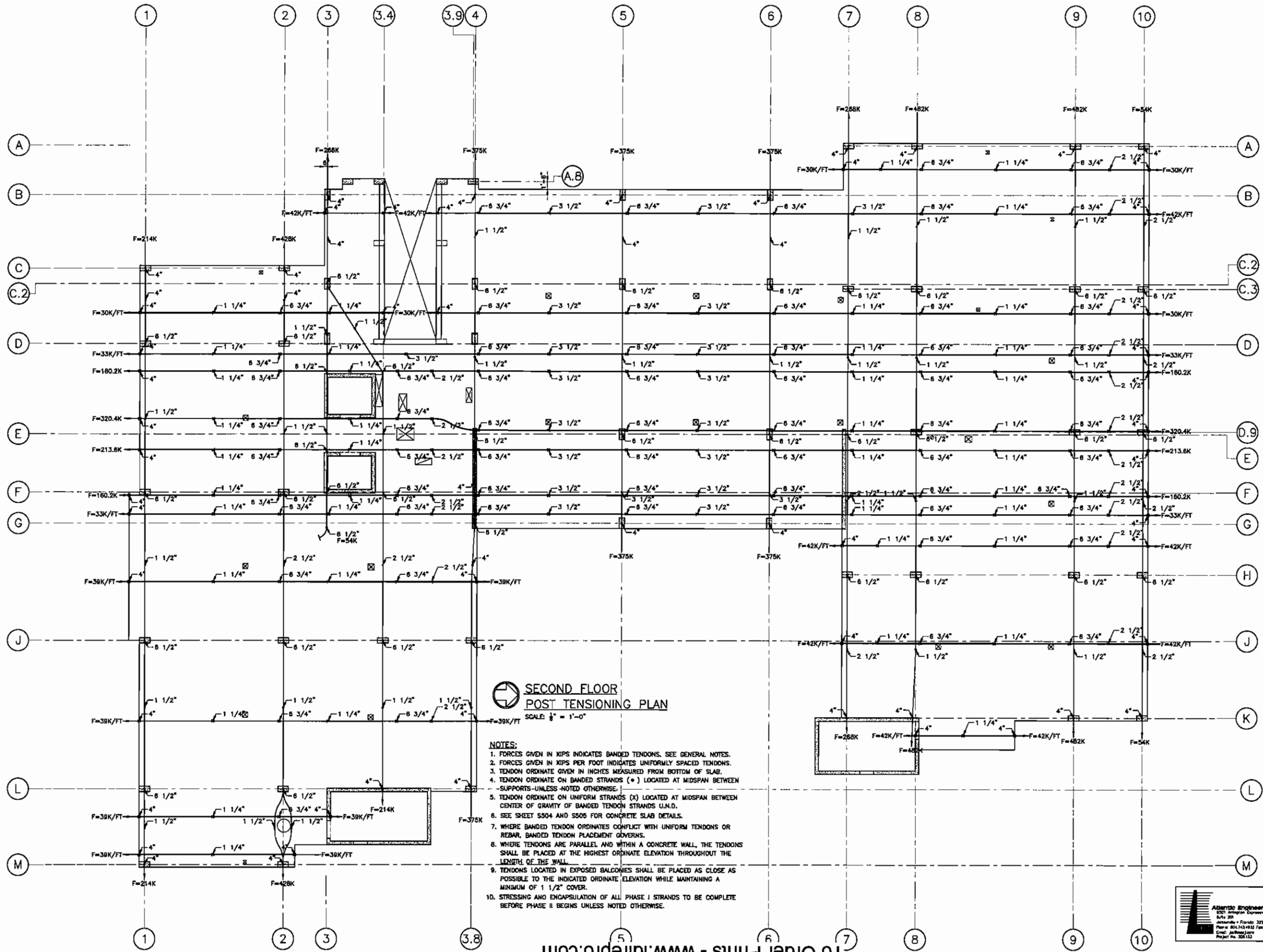
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ISSUE DATE: MAR. 2, 2007  
REVISIONS:  
DRAWN BY: EB  
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**SECOND FLOOR PLAN**  
**Holiday Inn EXPRESS**  
JACKSONVILLE BEACH, FLORIDA

**Ebert Norman Brady Architects**  
1361 13th Avenue South, Suite 200 Jacksonville Beach, Florida 32206  
Tel: 904.241.9887 Fax: 904.241.7828 www.enbradych.com



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**SECOND FLOOR  
POST TENSIONING PLAN**  
SCALE: 1/8" = 1'-0"

- NOTES:**
1. FORCES GIVEN IN KIPS INDICATES BANDED TENDONS. SEE GENERAL NOTES.
  2. FORCES GIVEN IN KIPS PER FOOT INDICATES UNIFORMLY SPACED TENDONS.
  3. TENDON ORDINATE GIVEN IN INCHES MEASURED FROM BOTTOM OF SLAB.
  4. TENDON ORDINATE ON BANDED STRANDS (•) LOCATED AT MIDSPAN BETWEEN SUPPORTS—UNLESS NOTED OTHERWISE.
  5. TENDON ORDINATE ON UNIFORM STRANDS (X) LOCATED AT MIDSPAN BETWEEN CENTER OF GRAVITY OF BANDED TENDON STRANDS U.N.D.
  6. SEE SHEET S504 AND S505 FOR CONCRETE SLAB DETAILS.
  7. WHERE BANDED TENDON ORDINATES CONFLICT WITH UNIFORM TENDONS OR REBAR, BANDED TENDON PLACEMENT GOVERNS.
  8. WHERE TENDONS ARE PARALLEL AND WITHIN A CONCRETE WALL, THE TENDONS SHALL BE PLACED AT THE HIGHEST ORDINATE ELEVATION THROUGHOUT THE LENGTH OF THE WALL.
  9. TENDONS LOCATED IN EXPOSED BALCONIES SHALL BE PLACED AS CLOSE AS POSSIBLE TO THE INDICATED ORDINATE ELEVATION WHILE MAINTAINING A MINIMUM OF 1 1/2" COVER.
  10. STRESSING AND ENCAPSULATION OF ALL PHASE I STRANDS TO BE COMPLETE BEFORE PHASE II BEGINS UNLESS NOTED OTHERWISE.

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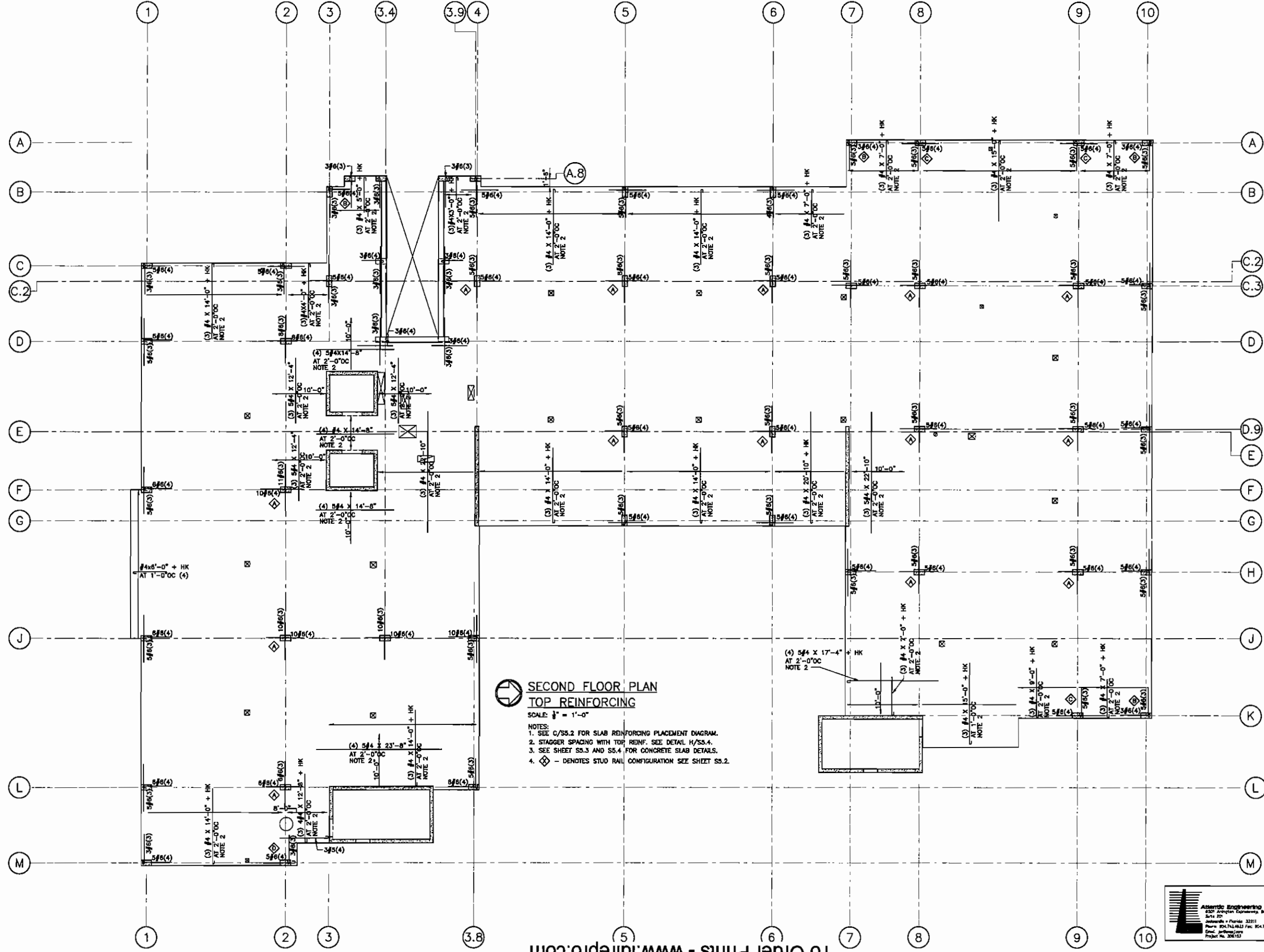
**SECOND FLOOR PT PLAN**  
**Holiday Inn EXPRESS**  
JACKSONVILLE BEACH, FLORIDA

37435 PE  
MARK J. KEISTER

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**SECOND FLOOR PLAN**  
**TOP REINFORCING**  
 SCALE: 1/4" = 1'-0"  
 NOTES:  
 1. SEE C/SS.2 FOR SLAB REINFORCING PLACEMENT DIAGRAM.  
 2. STAGGER SPACING WITH TOP REINF. SEE DETAIL H/SS.4.  
 3. SEE SHEET SS.3 AND SS.4 FOR CONCRETE SLAB DETAILS.  
 4.  $\diamond$  - DENOTES STUD RAIL CONFIGURATION SEE SHEET SS.2.



**S1.2-T**

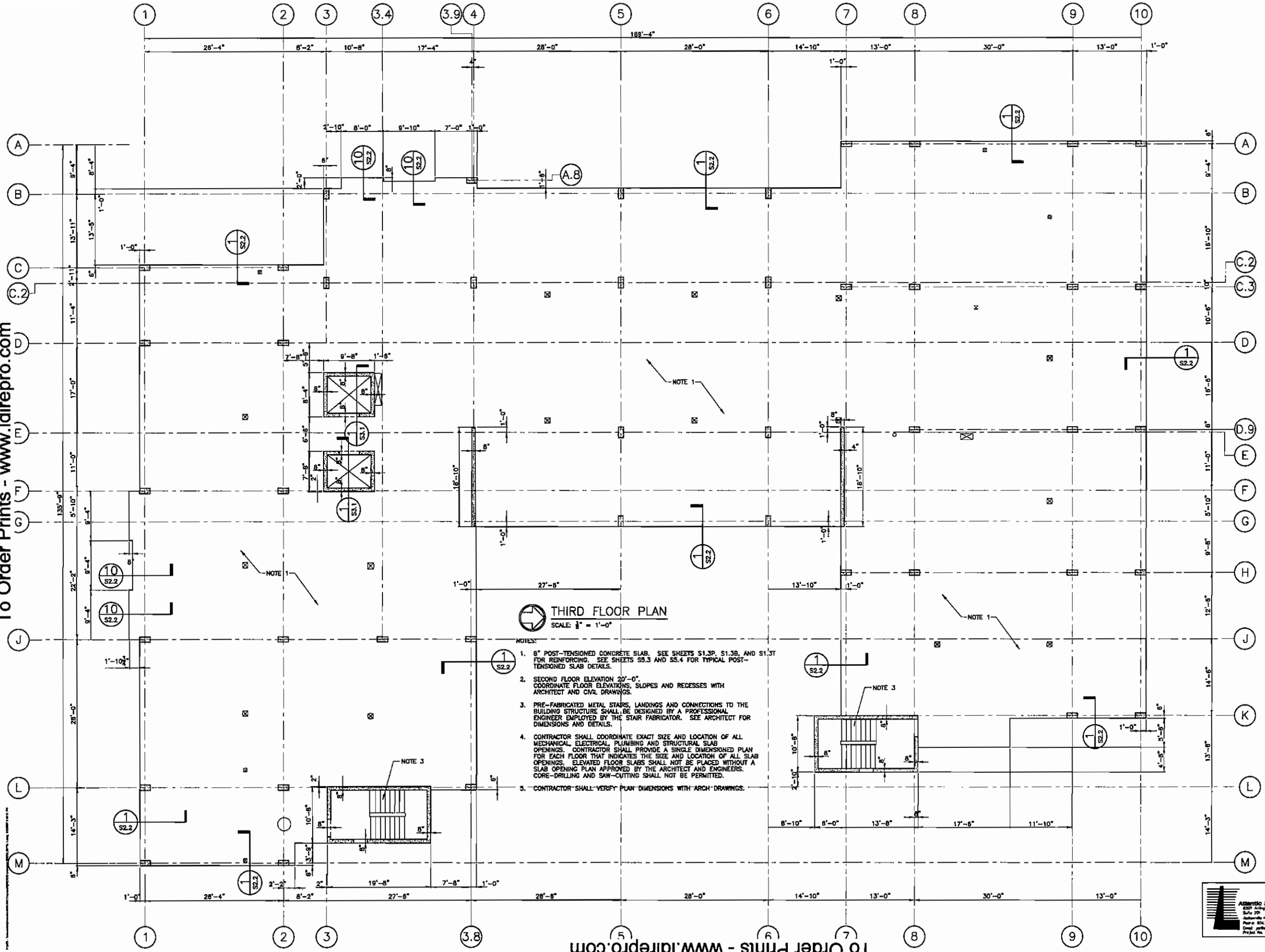
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**SECOND FLOOR TOP REINFORCING PLAN**  
  
 JACKSONVILLE BEACH, FLORIDA

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**THIRD FLOOR PLAN**  
SCALE: 1/4" = 1'-0"

- NOTES:
1. 8" POST-TENSIONED CONCRETE SLAB. SEE SHEETS S1.3P, S1.3B, AND S1.3T FOR REINFORCING. SEE SHEETS S5.3 AND S5.4 FOR TYPICAL POST-TENSIONED SLAB DETAILS.
  2. SECOND FLOOR ELEVATION 20'-0". COORDINATE FLOOR ELEVATIONS, SLOPES AND RECESSES WITH ARCHITECT AND CIVIL DRAWINGS.
  3. PRE-FABRICATED METAL STAIRS, LANDINGS AND CONNECTIONS TO THE BUILDING STRUCTURE SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER EMPLOYED BY THE STAIR FABRICATOR. SEE ARCHITECT FOR DIMENSIONS AND DETAILS.
  4. CONTRACTOR SHALL COORDINATE EXACT SIZE AND LOCATION OF ALL MECHANICAL, ELECTRICAL, PLUMBING AND STRUCTURAL SLAB OPENINGS. CONTRACTOR SHALL PROVIDE A SINGLE DIMENSIONED PLAN FOR EACH FLOOR THAT INDICATES THE SIZE AND LOCATION OF ALL SLAB OPENINGS. ELEVATED FLOOR SLABS SHALL NOT BE PLACED WITHOUT A SLAB OPENING PLAN APPROVED BY THE ARCHITECT AND ENGINEERS. CORE-DRILLING AND SAW-CUTTING SHALL NOT BE PERMITTED.
  5. CONTRACTOR SHALL VERIFY PLAN DIMENSIONS WITH ARCH. DRAWINGS.

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**S1.3**

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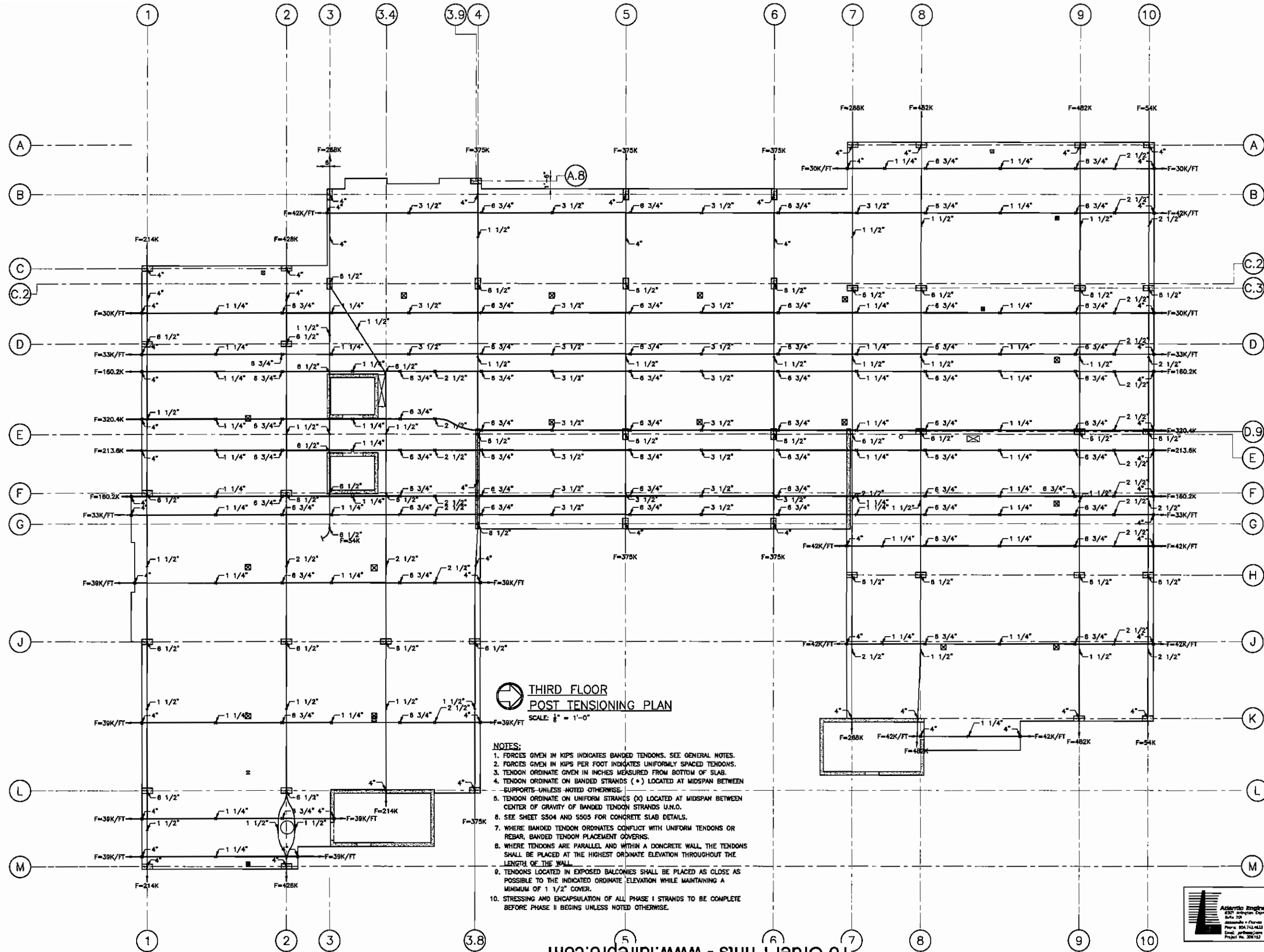
**THIRD FLOOR PLAN**  
**Holiday Inn EXPRESS**  
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**THIRD FLOOR  
POST TENSIONING PLAN**  
SCALE: 1/8" = 1'-0"

- NOTES:**
- FORCES GIVEN IN KIPS INDICATES BANDED TENDONS. SEE GENERAL NOTES.
  - FORCES GIVEN IN KIPS PER FOOT INDICATES UNIFORMLY SPACED TENDONS.
  - TENDON ORDINATE GIVEN IN INCHES MEASURED FROM BOTTOM OF SLAB.
  - TENDON ORDINATE ON BANDED STRANDS (•) LOCATED AT MIDSPAN BETWEEN SUPPORTS UNLESS NOTED OTHERWISE.
  - TENDON ORDINATE ON UNIFORM STRANDS (x) LOCATED AT MIDSPAN BETWEEN CENTER OF GRAVITY OF BANDED TENDON STRANDS U.N.O.
  - SEE SHEET S504 AND S505 FOR CONCRETE SLAB DETAILS.
  - WHERE BANDED TENDON ORDINATES CONFLICT WITH UNIFORM TENDONS OR REBAR, BANDED TENDON PLACEMENT GOVERNS.
  - WHERE TENDONS ARE PARALLEL AND WITHIN A CONCRETE WALL, THE TENDONS SHALL BE PLACED AT THE HIGHEST ORDINATE ELEVATION THROUGHOUT THE LENGTH OF THE WALL.
  - TENDONS LOCATED IN EXPOSED BALCONIES SHALL BE PLACED AS CLOSE AS POSSIBLE TO THE INDICATED ORDINATE ELEVATION WHILE MAINTAINING A MINIMUM OF 1 1/2" COVER.
  - STRESSING AND ENCAPSULATION OF ALL PHASE I STRANDS TO BE COMPLETE BEFORE PHASE II BEGINS UNLESS NOTED OTHERWISE.

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**S1.3-P**

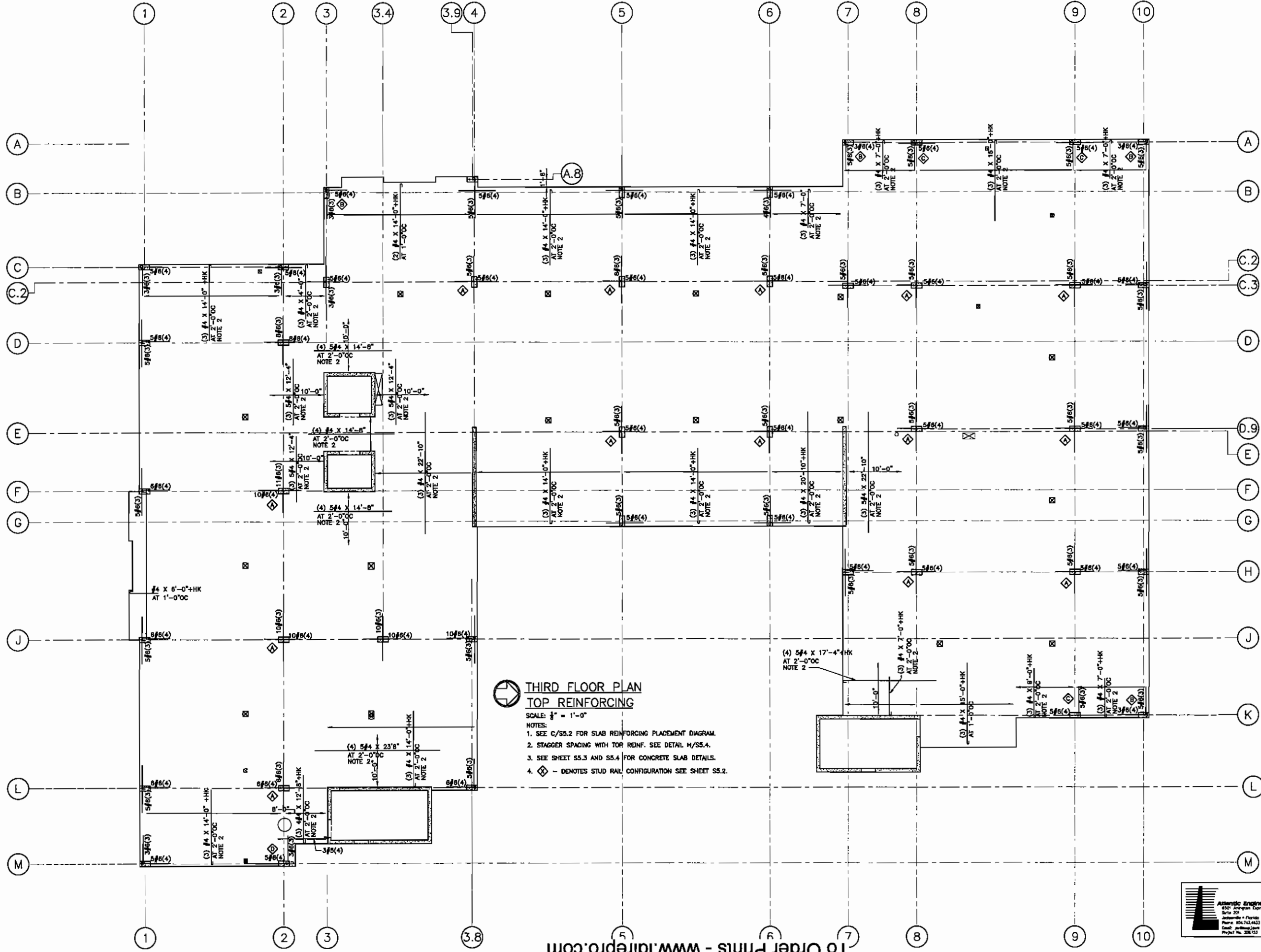
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**THIRD FLOOR PT PLAN**  
**Holiday Inn EXPRESS**  
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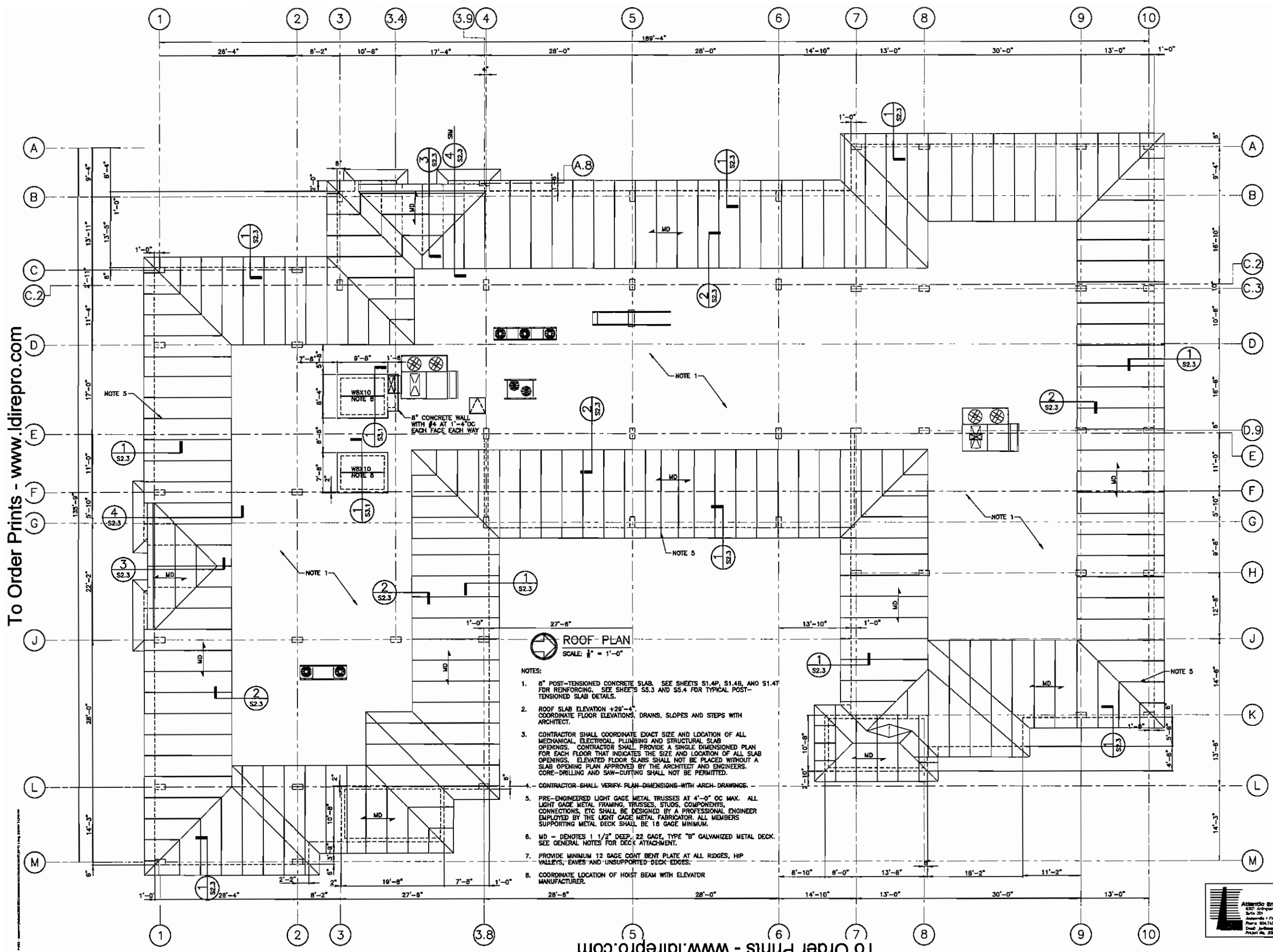
THIRD FLOOR TOP REINFORCING PLAN

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**ROOF PLAN**  
SCALE: 1/8" = 1'-0"

- NOTES:
- 8" POST-TENSIONED CONCRETE SLAB. SEE SHEETS S1.4P, S1.4B, AND S1.4T FOR REINFORCING. SEE SHEETS S5.3 AND S5.4 FOR TYPICAL POST-TENSIONED SLAB DETAILS.
  - ROOF SLAB ELEVATION +20'-4". COORDINATE FLOOR ELEVATIONS, DRAWS, SLOPES AND STEPS WITH ARCHITECT.
  - CONTRACTOR SHALL COORDINATE EXACT SIZE AND LOCATION OF ALL MECHANICAL, ELECTRICAL, PLUMBING AND STRUCTURAL SLAB OPENINGS. CONTRACTOR SHALL PROVIDE A SINGLE DIMENSIONED PLAN FOR EACH FLOOR THAT INDICATES THE SIZE AND LOCATION OF ALL SLAB OPENINGS. ELEVATED FLOOR SLABS SHALL NOT BE PLACED WITHOUT A SLAB OPENING PLAN APPROVED BY THE ARCHITECT AND ENGINEERS. CORE-DRILLING AND SAW-CUTTING SHALL NOT BE PERMITTED.
  - CONTRACTOR SHALL VERIFY PLAN DIMENSIONS WITH ARCH. DRAWINGS.
  - PRE-ENGINEERED LIGHT GAGE METAL TRUSSES AT 4'-0" OC MAX. ALL LIGHT GAGE METAL FRAMING, TRUSSES, STUDS, COMPONENTS, CONNECTIONS, ETC SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER EMPLOYED BY THE LIGHT GAGE METAL FABRICATOR. ALL MEMBERS SUPPORTING METAL DECK SHALL BE 16 GAGE MINIMUM.
  - MD - DENOTES 1 1/2" DEEP, 22 GAGE, TYPE "B" GALVANIZED METAL DECK. SEE GENERAL NOTES FOR DECK ATTACHMENT.
  - PROVIDE MINIMUM 12 GAGE CONT BENT PLATE AT ALL RIDGES, HIP VALLEYS, EAVES AND UNSUPPORTED DECK EDGES.
  - COORDINATE LOCATION OF HOIST BEAM WITH ELEVATOR MANUFACTURER.

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**S1.4**

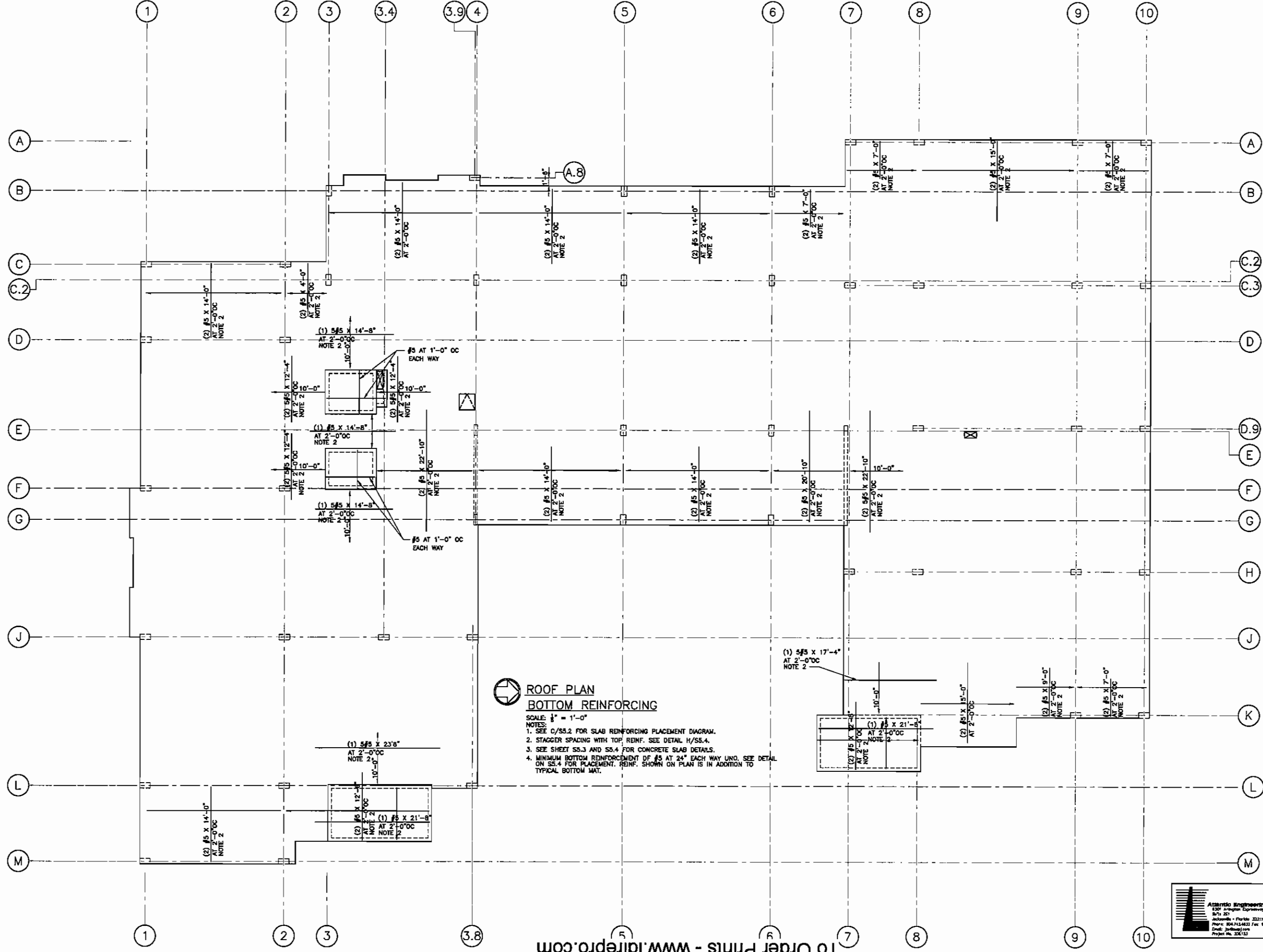
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**ROOF PLAN**  
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**ROOF PLAN  
BOTTOM REINFORCING**  
SCALE: 1/4" = 1'-0"  
NOTES:  
1. SEE C/S3.2 FOR SLAB REINFORCING PLACEMENT DIAGRAM.  
2. STAGGER SPACING WITH TOP REINF. SEE DETAIL H/SS.4.  
3. SEE SHEET SS.3 AND SS.4 FOR CONCRETE SLAB DETAILS.  
4. MINIMUM BOTTOM REINFORCEMENT OF #5 AT 24" EACH WAY UNO. SEE DETAIL ON SS.4 FOR PLACEMENT. REINF. SHOWN ON PLAN IS IN ADDITION TO TYPICAL BOTTOM MAT.

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**S1.4-B**

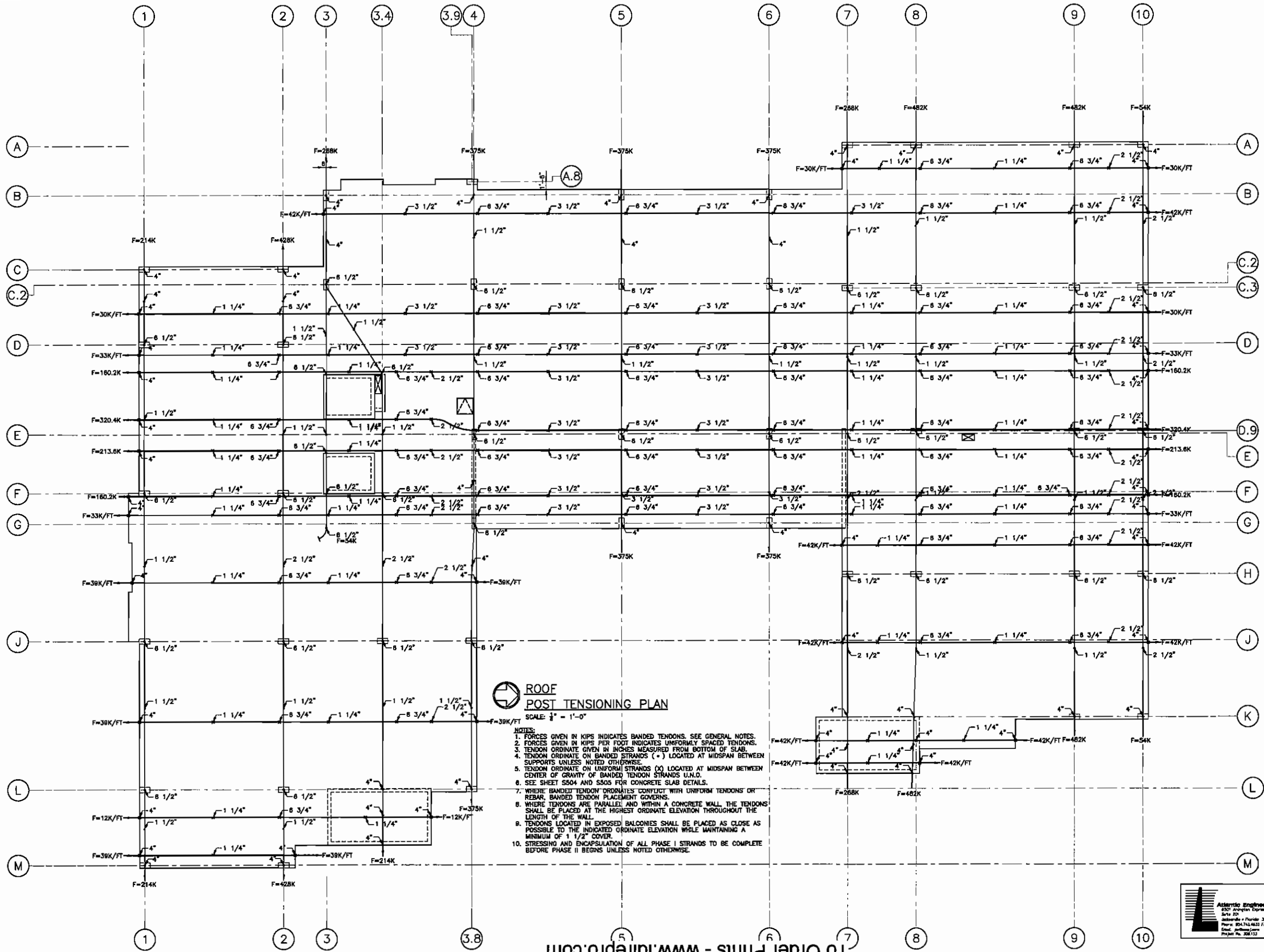
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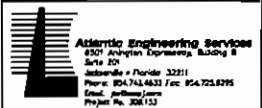
**ROOF BOTTOM REINFORCING PLAN**  
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