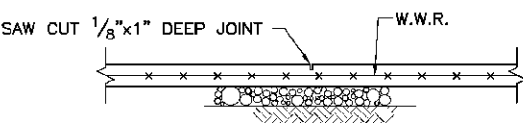
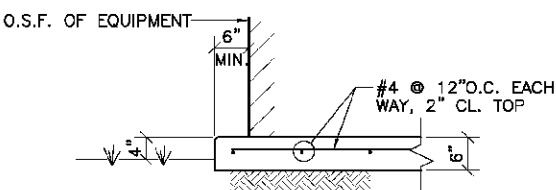


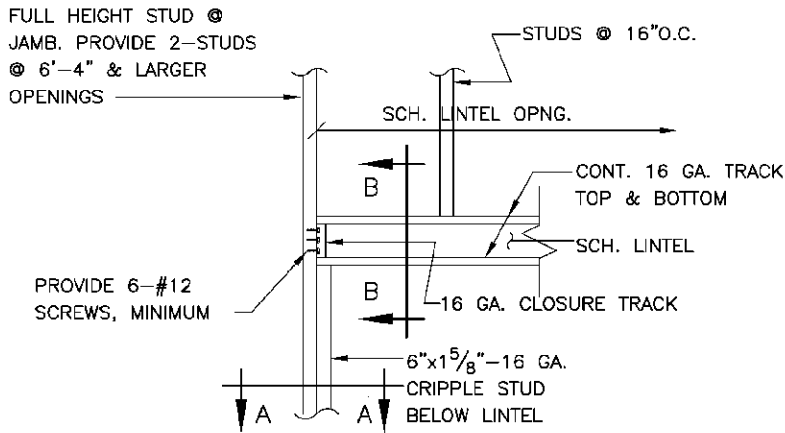
TYPICAL SLAB CONSTRUCTION JOINT DETAIL - (C.J.)



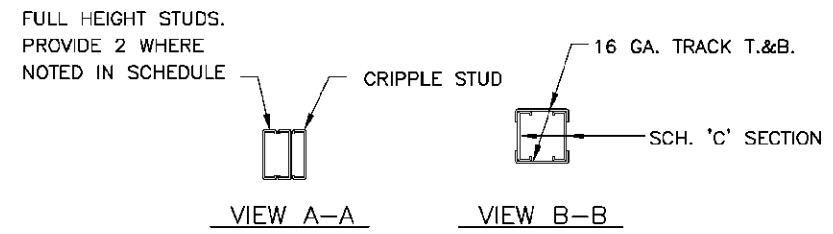
TYPICAL SAW CUT CONTROL JOINT DETAIL - (S.J.)
CUT SAWN JOINTS AS SOON AS SLAB WILL SUPPORT WEIGHT OF EQUIPMENT



TYPICAL MECHANICAL EQUIPMENT PAD DETAIL



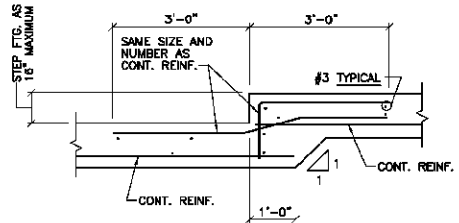
TYPICAL LINTEL DETAIL AT STEEL STUD WALLS



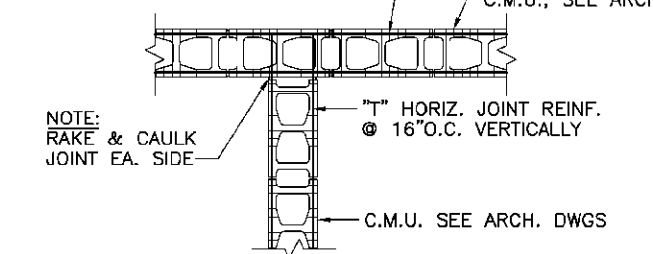
FULL HEIGHT STUDS. PROVIDE 2 WHERE NOTED IN SCHEDULE

INTERIOR STEEL STUD HEIGHT/SIZE/SPACING

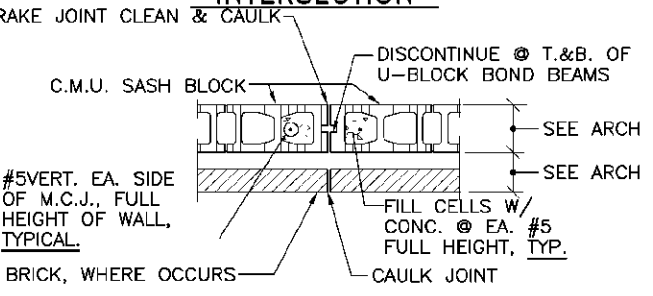
STUD SIZE INCHES	SPACING IN. O.C.	GAGE	MAX HEIGHT	BRIDGING	REMARKS
4	24	20	13'-10"	48" O.C. VERTICALLY	SEE NOTES
4	24	18	15'-8"	48" O.C. VERTICALLY	SEE NOTES
4	18	18	17'-9"	48" O.C. VERTICALLY	SEE NOTES
4	16	16	19'-0"	48" O.C. VERTICALLY	SEE NOTES
6	24	20	19'-6"	48" O.C. VERTICALLY	SEE NOTES
6	24	18	21'-4"	48" O.C. VERTICALLY	SEE NOTES
6	16	18	24'-4"	48" O.C. VERTICALLY	SEE NOTES
6	16	16	28'-2"	48" O.C. VERTICALLY	SEE NOTES



TYPICAL FOOTING STEP DETAIL - (F.S.)



TYPICAL DETAIL AT WALL INTERSECTION

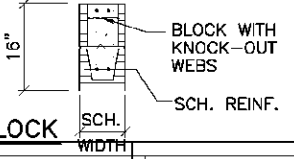


TYPICAL MASONRY CONTROL JOINT (M.C.J.)
SEE ARCH. FOR LOCATION OF JOINTS

LINTEL SCHEDULE

MARK OR LOCATION	MAX. SPAN	TYPE	SIZE	REINFORCEMENT	REMARKS
8" C.M.U.	4'-0"	U-BLOCK	8x16x8	#5 TOP & BOT.	8" HI U-BLK.
8" C.M.U.	6'-4"	U-BLOCK	8x16x16	2-#5 T.&B.	16" HI U-BLK.
6" C.M.U.	4'-0"	U-BLOCK	6x16x8	#5 TOP & BOT.	8" HI U-BLK.
4" BRICK	4'-0"	STEEL ANGLE	L4x4x1/4	---	BEAR 8" EA. END
4" BRICK	6'-4"	STEEL ANGLE	L6x4x3/8	---	BEAR 8" EA. END L.L.V.
L-1	9'-0"	U-BLOCK	8x16x16	2-#5 T.&B.	BEAR 16" EA. END

- NOTES:
- 1 - BEAR 8" HIGH U-BLOCKS 8" EACH END & 16" HIGH U-BLOCKS 16" EACH END.
 - 2 - FILL CELLS W/ CONCRETE FULL HEIGHT @ U-BLOCK BEARING, FOR ENTIRE LENGTH OF BEARING & REINF W/ #5 EA CELL.



16" HIGH U-BLOCK

GENERAL NOTES

- FOUNDATION:
1. SEE SOILS REPORT FOR ADDITIONAL INFORMATION AND DESIGN REQUIREMENTS AND PARAMETERS.
 2. FOOTINGS WERE DESIGNED FOR AN ALLOWABLE SOIL BEARING OF $F_c = 1500$ PSF. FINAL GEOTECHNICAL INVESTIGATION SHALL GOVERN BEARING CAPACITY AND WHICH SHALL BE CONFIRMED BY GC BEFORE CONSTRUCTION.
 3. DEPTHS SHOWN IN SECTIONS TO TOP OF FOOTING ELEVATIONS SHOWN ARE MINIMUM DEPTHS. DIFFERENT OR UNUSUAL CONDITIONS SHALL BE REPORTED TO THE ARCHITECT AND/OR ENGINEER.
 4. ALL FOOTING REINFORCEMENT SHALL BE HELD SECURELY FROM THE GROUND. CONCRETE BLOCK AND BROKEN TILE SHALL NOT BE USED. CONCRETE OR CLAY BRICK MAY BE USED.
 5. DOWEL ALL FOOTINGS AND WALLS WHERE THEY ABUT WITH SAME STEEL AS VERTICAL.
 6. PROVIDE PREFORMED EXPANSION JOINT WITH SEALANT WHERE SHOWN. IN FOOTINGS AND GRADE BEAMS 3" CLEAR BOTTOM AND SIDES. 1 1/2" CLEAR TOP. CONCRETE SLABS 3/4" CLEAR. CONCRETE PIERS AND WALLS 1 1/2" CLEAR SIDES.
 7. 1/4" X 4" CONTINUOUS BARS WITH CLASS B SPIGOTS (5# DIAMETERS) UNLESS OTHERWISE NOTED.
 8. PLACING PLANS AND DETAILS SHALL BE IN ACCORDANCE WITH THE LATEST "A.C.I." DETAILING MANUAL.
 9. STEEL FABRICATOR SHALL SUBMIT SHOP DRAWINGS FOR THE ARCHITECT AND/OR ENGINEER'S REVIEW.
 10. THERE SHALL BE NO REINFORCING STEEL WELDED.
 11. COMPRESSIVE STRENGTH TESTS SHALL BE AVAILABLE ON JOB SITE FOR REVIEW OF THE INSPECTOR.
- CONCRETE:
1. ALL CONCRETE SHALL HAVE A COMPRESSIVE STRENGTH AT 28 DAYS OF $F_c = 3000$ PSI. A MAXIMUM WATER-CEMENT RATIO OF 0.53 AND SHALL CONTAIN ENTRAINED AIR. SEE SPECS FOR ADDITIONAL INFORMATION.
 2. ALL DEFORMED REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60.
 3. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
 4. PROTECTIVE COVERING OF REINFORCEMENT (SEE DETAILS) SHALL BE AS FOLLOWS: FOOTINGS AND GRADE BEAMS 3" CLEAR BOTTOM AND SIDES. 1 1/2" CLEAR TOP. CONCRETE SLABS 3/4" CLEAR. CONCRETE PIERS AND WALLS 1 1/2" CLEAR SIDES.
 5. 1/4" X 4" CONTINUOUS BARS WITH CLASS B SPIGOTS (5# DIAMETERS) UNLESS OTHERWISE NOTED.
 6. PLACING PLANS AND DETAILS SHALL BE IN ACCORDANCE WITH THE LATEST "A.C.I." DETAILING MANUAL.
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 8. THERE SHALL BE NO REINFORCING STEEL WELDED.
 9. COMPRESSIVE STRENGTH TESTS SHALL BE AVAILABLE ON JOB SITE FOR REVIEW OF THE INSPECTOR.
- STRUCTURAL STEEL:
1. ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM A992, GRADE B, $F_y = 50$ KSI LATEST EDITION (EXCEPT MISCELLANEOUS SECTIONS AND TUBE SECTIONS).
 2. STEEL FABRICATOR SHALL SUBMIT SHOP DRAWINGS FOR THE ARCHITECT AND/OR ENGINEER'S REVIEW. ALL SHOP AND ERECTION DRAWINGS SHALL BE REVIEWED AND SEALED BY THE FABRICATOR'S ENGINEER.
 3. THE CONTRACTOR SHALL VERIFY ALL SHOP DRAWINGS DIMENSIONS WITH STRUCTURAL AND ARCHITECTURAL PLANS AND DETAILS.
 4. HOLLOW STRUCTURAL STEEL (HSS) SECTIONS SHALL CONFORM TO ASTM A500, GRADE B, $F_y = 48.0$ KSI. MISCELLANEOUS SHAPES (ANGLES, PLATES, ETC.) SHALL CONFORM TO ASTM A36.
 5. BOLTED CONNECTIONS SHALL BE MADE WITH HIGH STRENGTH BOLTS CONFORMING TO ASTM A325. USE 3/4" INCH DIAMETER MINIMUM.
 6. CONNECTIONS NOT SHOWN ON DRAWINGS SHALL BE DESIGNED BY THE FABRICATOR'S ENGINEER.
 7. COLD FORMED STRUCTURAL MEMBERS SHALL CONFORM TO THE FOLLOWING: 18 GAGE AND LIGHTER: ASTM 611, GRADE C. INTERIOR PARTITIONS 20 GA OR 1/4" OVER AND SHALL BE ANCHORED TO STRUCTURE ABOVE VIA GONGE 20 GA TRACK WITH HLT X-DIM AT 32" O.C. (1" PENETRATION) AND STEEL NETWORK VERTICAL SL EACH STUD. INSPECTION SHALL BE PERFORMED ON CONTINUOUS BASES PER IBC 1704.
 8. STRUCTURAL, SPECIALTY ITEMS INCLUDING BUT NOT LIMITED TO STOREFRONT GLAZING SYSTEMS, SHALL BE SEALED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF SOUTH CAROLINA AND SHALL BE AVAILABLE AT THE JOB SITE DURING PHASE OF INSPECTION.
 9. SPECIAL PERIODIC AND CONTINUOUS INSPECTIONS ARE REQUIRED FOR THIS PROJECT FOR WELDING OF STRUCTURAL COMPONENTS INCLUDING BUT NOT LIMITED TO FRAMING CONNECTIONS FOR SEISMIC-FORCE RESISTING SYSTEMS (BRACED FRAMES, MOMENT RESISTING FRAMES, ETC.) FOR STRUCTURES CLASSIFIED AS SEISMIC DESIGN CATEGORY C, D, E, OR F (IBC SECTION 1616.3), STRUCTURAL STEEL, FLOOR/ROOF DECK, WELDED STUDS, LIGHT GAUGE STEEL FRAMING, AND REINFORCING STEEL PER IBC SECTIONS INCLUDING BUT NOT LIMITED TO 1704.3, 1707.2, AND 2208.
 10. THE DESIGN OF SPECIAL CONNECTIONS BETWEEN STEEL FRAMING COMPONENTS BY OTHER THAN THE PROJECT STRUCTURAL ENGINEER OF RECORD SHALL BE PERFORMED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF SOUTH CAROLINA, INCLUDING BUT NOT LIMITED TO BRACE END CONNECTIONS, MOMENT RESISTING CONNECTIONS, MODIFIED BEAM SEAT CONNECTIONS, AND MEMBER SPLICE CONNECTIONS. SEE PLANS FOR APPLICABLE FORCES AND REACTIONS.
 11. ALL WELDS SHALL BE ACCOMPLISHED USING E70 ELECTRODES OR BETTER.
- INTERIOR STEEL STUD PARTITIONS:
1. ALL INTERIOR PARTITIONS SHALL BE OF THE SIZE AND SPACING SHOWN ON ARCHITECTURAL DRAWINGS AND OF 20 GAGE MINIMUM WITH 1/2" FLANGE AND CONTINUOUS TRACK TOP AND BOTTOM AND CONTINUOUS HORIZONTAL BRIDGING AT 48" O.C. VERTICAL OR CONTINUOUS SHEATHING FULL HEIGHT.
- MASONRY:
1. PROVIDE MASONRY HORIZONTAL JOINT REINFORCEMENT 16" O.C. VERTICAL IN ALL CONCRETE BLOCK WALLS. REINFORCEMENT SHALL BE FOR TOTAL WIDTH OF CAVITY AND MULTI-WYTHE WALLS.
 2. WHERE BEAMS OR LINTELS BEAR ON CONCRETE BLOCK WALLS, BLOCK CELLS SHALL BE FILLED WITH CONCRETE 1" MIN. WIDE TO FOUNDATION AND BE REINFORCED WITH 1-#5 EACH CELL.
 3. CONCRETE OR GROUT FOR BLOCK FILL SHALL HAVE 3/8" INCH MAXIMUM SIZE COARSE AGGREGATE AND SUFFICIENT WATER SO THE CONCRETE WILL FLOW INTO THE BLOCK CELLS WITHOUT LEAVING Voids. HEIGHT OF LIFT WHEN FILLING BLOCKS SHALL NOT EXCEED 6'-0".
 4. ANCHOR ALL MASONRY WALLS TO STEEL COLUMNS WITH STRAP ANCHORS AT 16" O.C. VERTICALLY UNLESS SHOWN OTHERWISE.
 5. UNLESS INDICATED OTHERWISE, PROVIDE KEYED RUBBER MASONRY CONTROL JOINTS AT A MAXIMUM SPACING OF 30 FEET. JOINT SHALL BE DISCONTINUOUS AT BOND BEAMS.
 6. PROVIDE REINFORCING BAR SUPPORTS TO CENTER VERTICAL REINFORCING IN MASONRY WALLS.
 7. PROVIDE 30 INCH LAP SPLICE IN VERTICAL MASONRY REINFORCING.
 8. PROVIDE 4'-0" X 4'-0" CORNER BARS IN ALL U-BLOCK CORNERS AND REINFORCED WITH A #5 EACH CELL, UNLESS OTHERWISE SHOWN.

- GENERAL DESIGN INFORMATION:
1. BUILDING AND COMPONENTS DESIGNED IN COMPLIANCE WITH 2006 INTERNATIONAL BUILDING CODE AND STATE AMENDMENTS.
 2. MANUFACTURERS OF BUILDING COMPONENTS SUCH AS CURTAIN WALL SYSTEMS, ETC. SHALL SUBMIT SHOP DRAWINGS SEALED BY A REGISTERED ENGINEER AS REQUIRED BY LOCAL ORDINANCES. SHOP DRAWINGS SHALL BE AVAILABLE AT JOB SITE DURING TIMES OF INSPECTION.
- 2006 INTERNATIONAL BUILDING CODE:
1. THE COMPLETE DESIGN OF METAL BUILDING INCLUDING ALL COMPONENTS SHOWN OR NOT SHOWN ON THE DRAWINGS SHALL BE ACCOMPLISHED BY THE BUILDING MANUFACTURER.
 2. THE DESIGN SHALL BE MADE BY A REGISTERED ENGINEER REGISTERED IN THE STATE OF SOUTH CAROLINA AND HE SHALL AFFIX HIS REGISTRATION NUMBER TO ALL SHOP DRAWINGS AND CALCULATIONS.
 3. THE BUILDING AND ALL OF HIS COMPONENTS SHALL BE DESIGNED FOR THE FOLLOWING DEAD AND LIVE LOADS:
 - a. ACTUAL WEIGHT OF STEEL STRUCTURE.
 - b. 12 PSF DEAD LOAD IN ADDITION TO ACTUAL WEIGHT.
 - c. 20 PSF ROOF LIVE LOAD, REDUCIBLE.
 - d. ANY ADDITIONAL LOADS AND REACTIONS THAT ARE SHOWN ON THE DRAWINGS.
 4. WHERE MEMBER SIZES AND GAGES ARE SHOWN THEY SHALL BE CONSIDERED A MINIMUM SIZE. THE MANUFACTURER SHALL NOT USE SMALLER SIZE OF LOWER GAGES OR LIGHT FRAMING UNLESS INDICATED. HE SHALL USE ONLY LARGER SIZE AND HEAVIER GAGES IF HIS DESIGN INDICATES THESE ARE REQUIRED TO MEET THE LOADING CRITERIA.
 5. THE EFFICIENCY OF GIRTS SHALL BE LIMITED TO 1/240 OF THE SPAN AND DEFLECTION OF PURLINS SHALL BE LIMITED TO 1/240 OF THE SPAN. DEFLECTION OF ROOF FRAMES SHALL BE LIMITED TO 1/240 OF THE SPAN. DEFLECTIONS SHALL BE BASED ON TOTAL LOAD (DEAD PLUS LIVE LOADS).
 6. COLUMN BASES SHALL BE DESIGNED AS PINNED CONNECTIONS UNLESS AT COLUMN BASE PLATES ARE NOT ACCEPTED.
 7. BUILDING MANUFACTURER SHALL DESIGN AND SUPPLY DIRTS AND WIND OTHER CONNECTIONS INCLUDING BUT NOT LIMITED TO COMPRES. MECHANICAL OPENINGS & ENCLOSURES, ETC.
- PRE-ENGINEERED PREFABRICATED JOIST GAGE STEEL TRUSSES:
1. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS FOR LIGHT GAGE STEEL TRUSSES.
 2. PROVIDE PRE-ENGINEERED PREFABRICATED LIGHT GAGE STEEL TRUSSES WHERE INDICATED ON PLAN, AND AS SPECIFIED.
 3. ALL FRAMING SHALL BE DESIGNED AND MANUFACTURED TO MEET THE FOLLOWING WORKING LOADS AND CODES:
 - MINIMUM LOADS (IN ADDITION TO ROOF SYSTEM FRAMING WEIGHT):
 - TOP CHORD DEAD LOAD..... 5 PSF
 - BOTTOM CHORD DEAD LOAD..... 2 PSF
 - TOP CHORD LIVE LOAD..... 20 PSF
 - TRANSIENT LOADS..... PER BELOW.
 - TRUSS SYSTEM CONNECTIONS AND CONNECTIONS OF THE TRUSS SYSTEM TO THE STRUCTURE SHALL BE DESIGNED AND SUPPLIED BY THE TRUSS SYSTEM MANUFACTURER.
 - MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND DESIGN CALCULATIONS FOR ALL COMPONENTS OF THE TRUSS SYSTEM. DESIGNS SHALL BE SIGNED BY A REGISTERED PROFESSIONAL ENGINEER, REGISTERED IN THE STATE OF SOUTH CAROLINA.
 - MECHANICAL CONTRACTOR SHALL PROVIDE WEIGHTS AND LOCATIONS OF ALL MECHANICAL EQUIPMENT TO TRUSS SYSTEM AND JOIST MANUFACTURERS.
 - SEE ARCHITECTURAL DRAWINGS FOR EAVE AND OVERHANG DIMENSIONS, FINISHES, AND OTHER DETAILS.
 - UPLIFT ANCHORAGE SHALL PROVIDE STRENGTH EQUAL TO OR GREATER THAN THAT SHOWN IN SECTIONS AND DETAILS, AND AS SHOWN ON TRUSS SHOP DRAWINGS, TYPICAL SEE MECHANICAL DRAWINGS FOR EQUIPMENT AND DUCTWORK INFORMATION. COORDINATE ALL FRAMING WITH MECHANICAL EQUIPMENT.
 - SEE MECHANICAL DRAWINGS FOR DUCTWORK AND OTHER LOADS TO BE SUPPORTED BY ROOF FRAMING. CONTRACTOR(S) SHALL COORDINATE PLUMBING AND OTHER EQUIPMENT WITH TRUSS SYSTEM MANUFACTURER.
 4. PROVIDE FRAMING AS PER DRAWINGS AND SPECIFICATIONS. DETAILS OF CONNECTIONS AND MEMBER SIZES SHALL BE CONSIDERED APPLICABLE TO ALL SIMILAR AND TYPICAL CONDITIONS. REGARDS OF INFORMATION SPECIFIED IN ANY PARTICULAR AREA. CONTACT ENGINEER IF CONDITIONS ARE FOUND TO DIFFER FROM GENERAL ASSUMPTIONS INDICATED ON PLANS. CONTRACTOR(S) SHALL CAREFULLY COORDINATE ALL DIMENSIONS WITH CONDITIONS AND ARCHITECTURAL DRAWINGS.
- DESIGN DEAD LOADS:
- MECHANICAL, ELECTRICAL, PLUMBING..... 4 PSF.
 CEILING..... 3 PSF. PLUS JOIST WT (3 PSF.)
 LOW PITCH ROOF..... 12 PSF. PLUS JOIST AND MECHANICAL UNITS
- SEISMIC:
- SEISMIC USE GROUP = GROUP 1
 SEISMIC IMPORTANCE CATEGORY I = 1.0
 SEISMIC SITE CLASS = D
 SEISMIC DESIGN CATEGORY = B (IBC 1616.3(2))
 EQUIVALENT LATERAL FORCE PROCEDURE
 $S_e = 1.00, S_1 = 0.30$
 RESPONSE MODIFICATION FACTOR, $R = 6$
 BASE SHEAR = 20 KIIPS. EACH ORTHOGONAL DIRECTION
 SEISMIC FORCES ARE TRANSMITTED TO SHEAR WALLS.
 SEE PLANS FOR LOCATIONS OF EACH ELEMENT.
- DESIGN LIVE LOADS:
- ROOF..... 20 PSF.
 GROUND SNOW..... 10 PSF.
 FLOOR..... 250 PSF.
 PLATFORMS..... 125 PSF.
 WIND..... IMPORTANCE FACTOR $I = 1.0$, CATEGORY 1
 EXPOSURE CATEGORY B
 INTERNAL PRESSURE COEFFICIENT = +/- 0.18
 COMPONENTS AND CLADDING PRESSURES:
 ROOF: 12 PSF NET UPLIFT
 WALL: +18/-20 TO 23 (SLCTION) PSF HORIZONTAL
 SEISMIC..... INTERNATIONAL BUILDING CODE/IBC REQUIREMENTS
- CODES:
- ALL PARTS SHALL BE FURNISHED AND ERECTED ACCORDING TO THE ENFORCED APPLICABLE CODES AND SPECIFICATIONS OF THE FOLLOWING:
- AMERICAN CONCRETE INSTITUTE (ACI)
 - AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC)
 - AMERICAN WELDING SOCIETY (AWS D1.1)
 - STEEL JOIST INSTITUTE (SJI)
 - AMERICAN IRON & STEEL INSTITUTE (AISI)



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REV. NO.	DATE	BY
01	06/28/07	NOTED

0632-03-00012 **S1.0**

RE-F

ARMY & AIR FORCE EXCHANGE SERVICE
 DEPARTMENTS OF THE ARMY & AIR FORCE

PROJECT: FIRESTONE CAR CARE CENTER

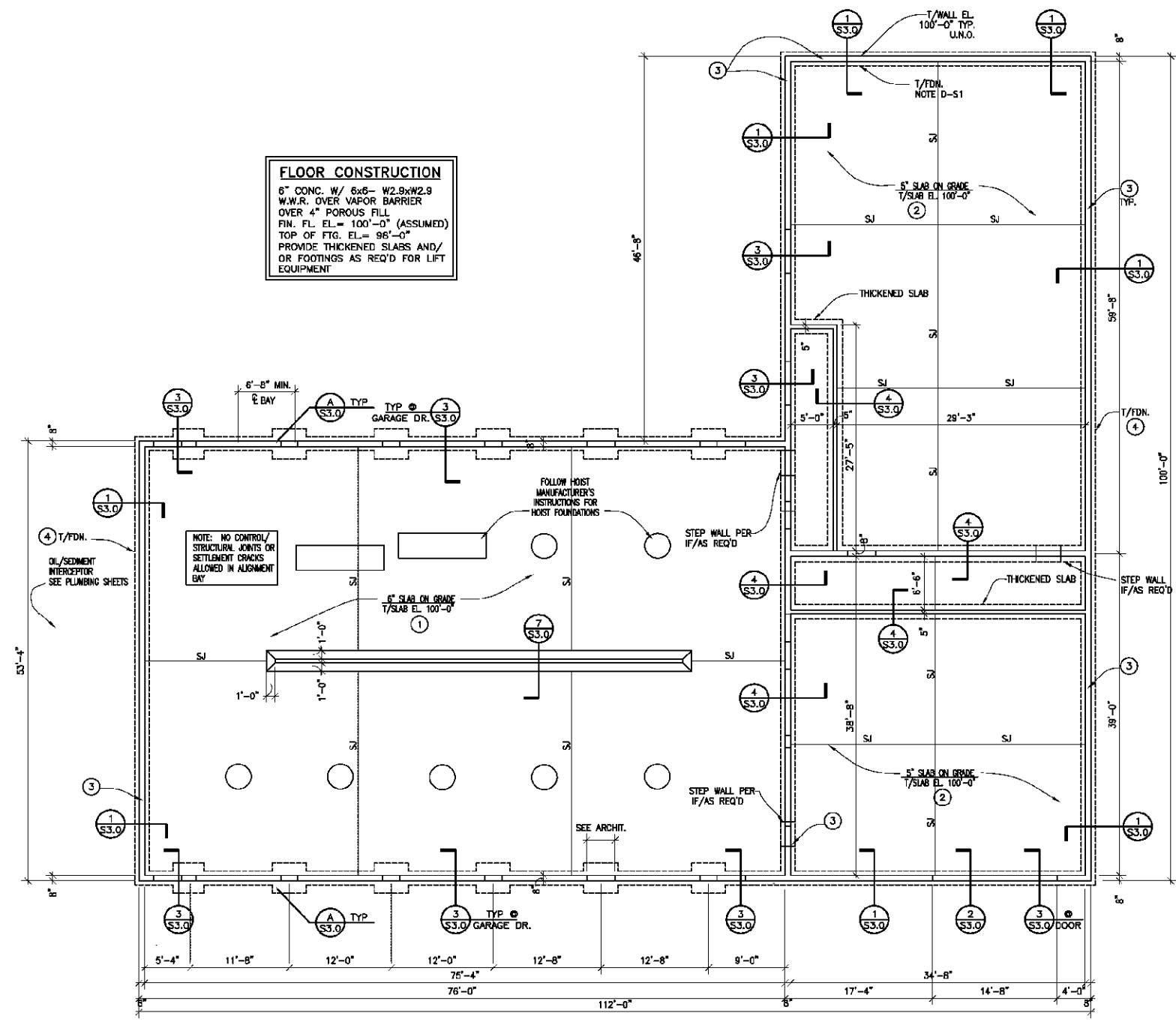
FT. JACKSON, SC

GENERAL NOTES & DETAILS

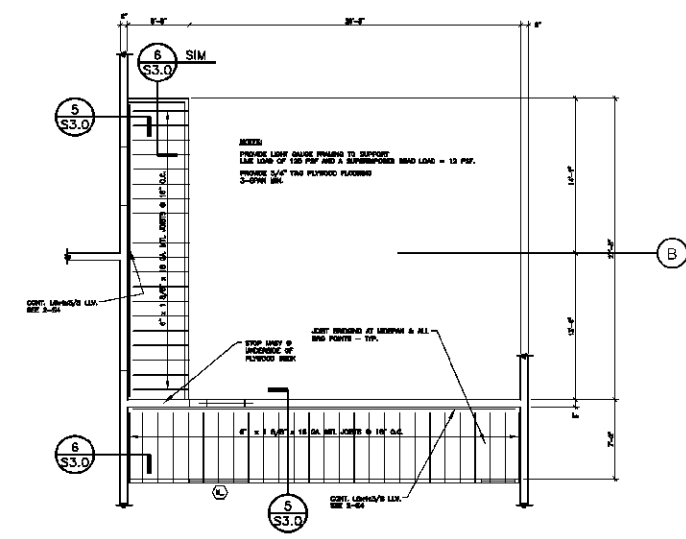


- KEYED PLAN NOTES**
- 1 PROVIDE 1" SLAB-ON-GRADE REINFORCED WITH WELDED WIRE FABRIC. PLACE SLAB ON VAPOR BARRIER OVER 4" OF COMPACTED GRANULAR FILL. SEE ARCHITECTURAL DRAWINGS FOR SLOPE OF FLOOR.
 - 2 PROVIDE 5" SLAB-ON-GRADE REINFORCED WITH WELDED WIRE FABRIC. PLACE SLAB ON VAPOR BARRIER OVER 4" OF COMPACTED GRANULAR FILL.
 - 3 PROVIDE 8" O.D. MASONRY REINF. FULL HEIGHT OF WALL IN GRouted CELLS. PROVIDE MATCHING CORNERS IN FOUNDATION WALLS.
 - 4 LOCATE BOTTOM OF FOOTING BELOW LOCAL FROST DEPTH IN ACCORDANCE WITH ALL LOCAL CODE REQUIREMENTS.
- (1) - INDICATES COLUMN MARK/PIER MARK REFER TO SCHEDULES THIS SHEET
 (2) - INDICATES FOOTING MARK REFER TO SCHEDULES THIS SHEET
 (3) - INDICATES TOP OF FOOTING ELEVATION

FLOOR CONSTRUCTION
 6" CONC. W/ 6x6 - W2.9xW2.9 W.W.R. OVER VAPOR BARRIER OVER 4" POROUS FILL
 FIN. FL. EL. = 100'-0" (ASSUMED)
 TOP OF FTG. EL. = 96'-0"
 PROVIDE THICKENED SLABS AND/OR FOOTINGS AS REQ'D FOR LIFT EQUIPMENT



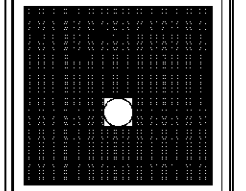
FOUNDATION PLAN
 SCALE: 1/8"=1'-0"
 NORTH



UTILITY PLATFORM PLAN
 SCALE: 1/4"=1'-0"
 NORTH



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DATE	REV. NO.	DESCRIPTION
	01	ISSUE FOR PERMIT
	02	REVISED PER COMMENTS
	03	REVISED PER COMMENTS
	04	REVISED PER COMMENTS

PROJECT NO.	0423
DATE	06.28.07
SCALE	AS SHOWN
PROJECT	FOUNDATION PLAN & NOTES
CLIENT	ARMY & AIR FORCE EXCHANGE SERVICE DEPARTMENTS OF THE ARMY & AIR FORCE
LOCATION	FT. JACKSON, SC
PROJECT NAME	FIRESTONE CAR CARE CENTER

NO.	DATE	DESCRIPTION
01	06.28.07	ISSUE FOR PERMIT
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03	06.28.07	REVISED PER COMMENTS
04	06.28.07	REVISED PER COMMENTS

RE-F

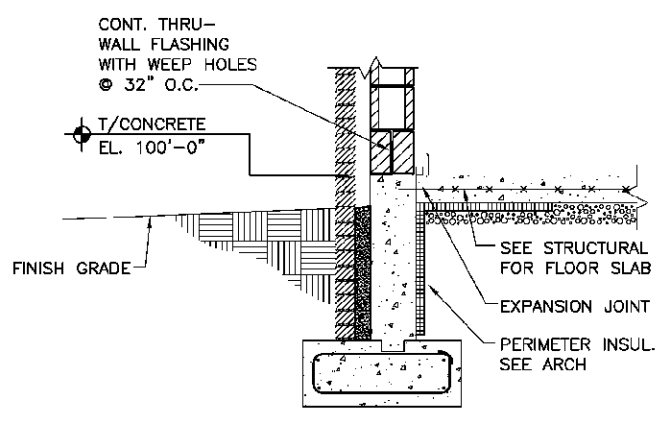
ARMY & AIR FORCE EXCHANGE SERVICE
 DEPARTMENTS OF THE ARMY & AIR FORCE

PROJECT: FIRESTONE CAR CARE CENTER

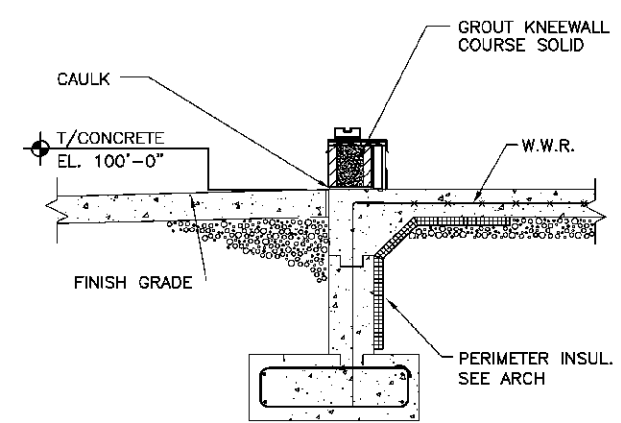
LOCATION: FT. JACKSON, SC

FOUNDATION PLAN & NOTES

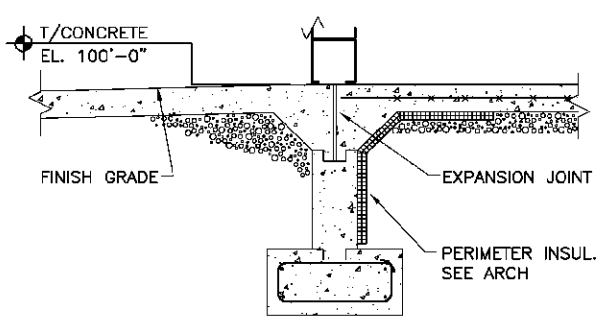
DATE	06.28.07	NOTED	BY
PROJECT NO.	0632-03-000012	SHEET	S2.0



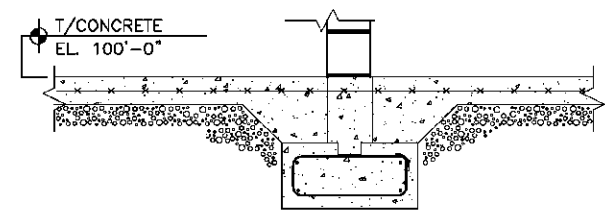
SECTION 1
3/4"=1'-0" S3.0



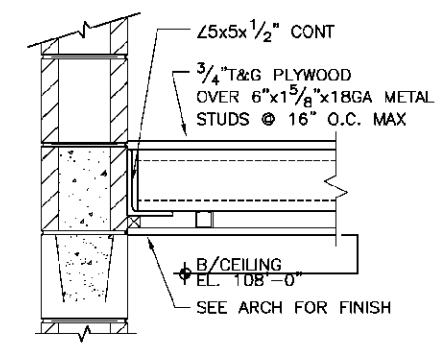
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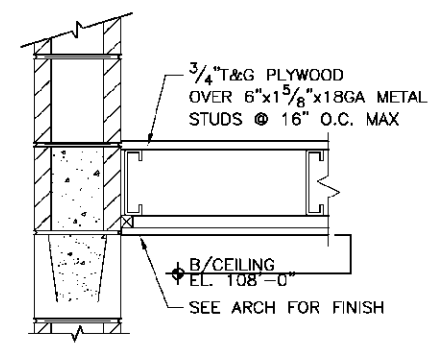
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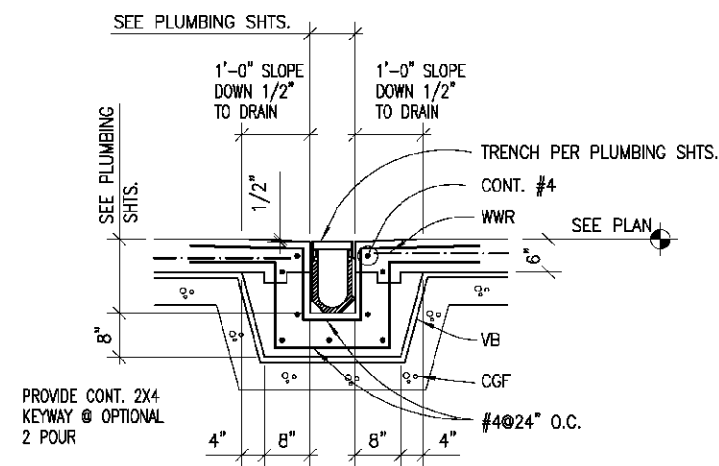
SECTION 4
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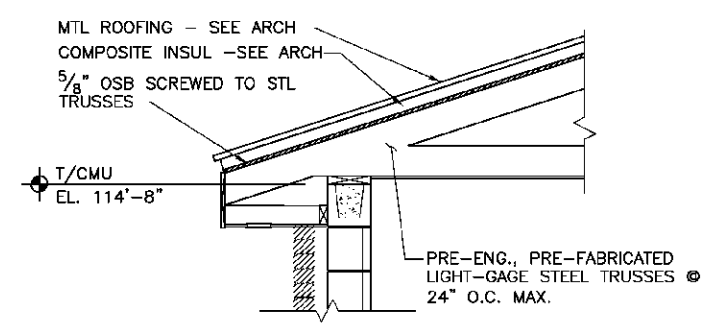
SECTION @ PLATFORM 5
1-1/2"=1'-0" S3.0



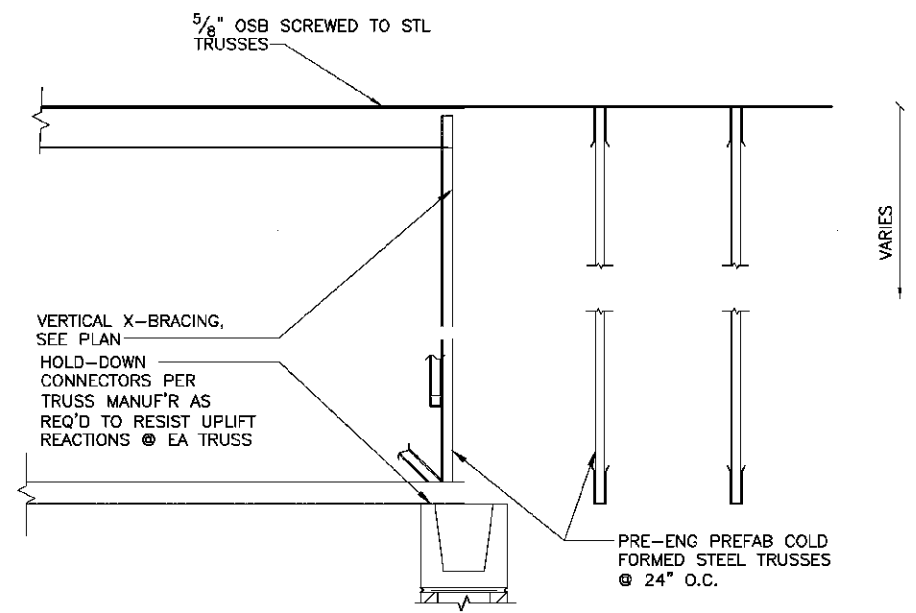
SECTION 6
3/4"=1'-0" S3.0



SECTION 7
3/4"=1'-0" S3.0



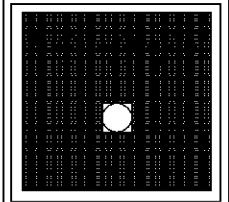
SECTION 8
3/4"=1'-0" S3.0



SECTION 9
3/4"=1'-0" S3.0



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06.28.07	1. JUNE 28, 2007	RDW
06.28.07	2. REVISED	RDW

NO.	0423
DATE	
BY	
CHKD	
APP'D	

RE-F

ARMY & AIR FORCE EXCHANGE SERVICE
 DEPARTMENTS OF THE ARMY & AIR FORCE

FIRESTONE CAR CARE CENTER

FT. JACKSON, SC

DETAILS

DATE	06.28.07	SCALE	NOTED	NO.	S3.0
PROJECT NO.	0632-03-000012				