STRUCTURAL ABBREVIATIONS

AB	ANCHOR BOLT	L	LENGTH
ACI	AMERICAN CONCRETE INSTITUTE	LAB	LABORATORY
		LB	POUND
ADJ	ADJACENT	LDG	LANDING
AFF	ABOVE FINISH FLOOR	LG	LONG
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	LL	LIVE LOAD
		LLH	LONG LEG HORIZONTAL
AISI	AMERICAN IRON & STEEL INSTITUTE	LLV	LONG LEG VERTICAL
		LSH	LONG SIDE HORIZONTAL
ALT	ALTERNATE	LSV	LONG SIDE VERTICLE
ALUM	ALUMINUM	LT	LEFT
ANSI	AMERICAN NATIONAL STANDARDS INSTITUTE	LTL	LINTEL
		LOC	LOCATION
APPROX	APPROXIMATE	LW	LIGHTWEIGHT
ARCH	ARCHITECT(URAL)		
ASTM	AMERICAN SOCIETY FOR TESTING & MATERIALS	MAS, MSNRY	MASONRY
		MAX	MAXIMUM
AVG	AVERAGE	MBMA	METAL BUILDING MANUFACTURERS ASSOC
B, BOT	BOTTOM	MC	MOMENT CONNECTION
BF	BOTTOM OF	MECH	MECHANICAL
BFF	BELOW FINISHED FLOOR	MEZZ	MEZZANINE
BLDG	BUILDING	MFR	MANUFACTURER
BM	BEAM	MIN	MINIMUM
BRG	BEARING	MISC	MISCELLANEOUS
BRK	BRICK	MO	MASONRY OPENING
BSMT	BASEMENT	MSL	MEAN SEA LEVEL
BTB	BACK TO BACK	MTL	METAL
BTWN	BETWEEN		
		N	NORTH
CANT	CANTILEVER	NE	NORTHEAST
CHAM	CHAMFER	NF	NEAR FACE
CIP	CAST IN PLACE	NIC	NOT IN CONTRACT
CJ	CONTROL/CONSTRUCTION JOINT	NO	NUMBER
		NOM	NOMINAL
CL	CENTERLINE	NS	NEAR SIDE
CLG	CEILING	NTS	NOT TO SCALE
CLR	CLEAR(ANCE)	NW	NORTHWEST
CMU	CONCRETE MASONRY UNIT		
COL	COLUMN	OC	ON CENTER
COMB	COMBINATION	OD	OUTSIDE DIAMETER
CONC	CONCRETE	OF	OUTSIDE FACE
CONN	CONNECTION	OH	OPPOSITE HAND
CONST	CONSTRUCTION	OPNG	OPENING
CONT	CONTINUOUS	OPP	OPPOSITE
COORD	COORDINATE	OTO	OUT TO OUT
CRS	COLD ROLLED STEEL		
CRSI	CONCRETE REINFORCING STEEL INSTITUTE	PC	PRECAST, PILE CAP
		PCI	PRECAST/PRESTRESSED CONCRETE INSTITUTE
CRW	CANTILEVER RETAINING WALL	PCP	PRECAST CONCRETE PANEL
CTC	CENTER TO CENTER		
CY	CUBIC YARD	PEJ	PREFORMED EXPANSION JOINT
DBA	DEFORMED BAR ANCHOR	PL	PLATE
DBL	DOUBLE	PLBG	PLUMBING
DEG	DEGREE	PROJ	PROJECTION
DEMO	DEMOLISH, DEMOLITION	PSF	POUNDS/SQUARE FOOT
DEP	DEPRESS(ED)	PSI	POUNDS/SQUARE INCH
DIA	DIAMETER	PT	POINT, POST TENSIONING
DIAG	DIAGONAL	PVG	PAVING
DIM	DIMENSION		
DIR	DIRECTION	R, RAD	RADIUS
DJ	DOUBLE JOIST	RECT	RECTANGULAR
DK	DECK	REF	REFERENCE
DL	DEAD LOAD	RFNF	REINFORCING, REINFORCEMENT
DN	DOWN		
DP	DEEP	REM	REMAINDER
DTL	DETAIL	REQ'D	REQUIRED
DWG	DRAWING	RET	RETAINING
DWL	DOWEL	REV	REVISION
		RM	ROOM
E	EAST	RT	RIGHT
E/	EDGE OF	RTU	ROOF TOP UNIT
EA	EACH		
EB	EXPANSION BOLT	S	SOUTH
EF	EACH FACE	SC	SLIP CRITICAL
EJ	EXPANSION JOINT	SCHED	SCHEDULE
EL	ELEVATION	SDI	STEEL DECK INSTITUTE
ELEC	ELECTRIC(AL)	SE	SOUTHEAST
ELEV	ELEVATOR	SEC	SECOND
EMBED	EMBEDMENT	SECT	SECTION
ENGR	ENGINEER	SF	SQUARE FOOT
ENTR	ENTRANCE	SHT	SHEET
EQ	EQUAL	SIM	SIMILAR
EQUIP	EQUIPMENT	SL	SLAB
ES	EACH SIDE	SLH	SHORT LEG HORIZONTAL
EW	EACH WAY	SLV	SHORT LEG VERTICAL
EXIST	EXISTING	SPA	SPACING
EXP	EXPANSION	SPECS	SPECIFICATIONS
EXT	EXTERIOR	SQ	SQUARE
		SOC	SLAB ON GRADE
F/	FACE OF	SSH	SHORT SIDE HORIZONTAL
FDN	FOUNDATION	SST	STAINLESS STEEL
FFE	FINISHED FLOOR ELEVATION	SSV	SHORT SIDE VERTICAL
		ST	STREET
FIN	FINISH	STA	STATION
FJ	FLOOR JOINT	STD	STANDARD
FLR	FLOOR/FLOORING	STL	STEEL
FS	FAR SIDE, FOOTING STEP	STRUCT	STRUCTURAL
FT	FOOT, FEET	SW	SOUTHWEST
FTF	FACE TO FACE	SYM	SYMMETRICAL
FTG	FOOTING		
		T	TOP
GA	GAUGE	T/	TOP OF
GALV	GALVANIZED	T/C	TOP OF CONCRETE
GC	GENERAL CONTRACT(OR)	TD	TURNED-DOWN SLAB
GR	GRADE	TEMP	TEMPORARY
GRD	GROUND	TFE	TOP OF FOOTING ELEVATION
GRV	GRAVEL		
		THK	THICK(NESS)
HCA	HEADED CONCRETE ANCHOR	THRU	THROUGH
HD	HEAVY DUTY	TFE	TOP OF PIER ELEVATION
HDR	HEADER	TS	TUBE STEEL
HEF	HORIZONTAL EACH FACE	T/W	TOP OF WALL
HIF	HORIZONTAL INSIDE FACE	TYP	TYPICAL
HK	HOOK	T&B	TOP & BOTTOM
HOF	HORIZONTAL OUTSIDE FACE		
HORIZ	HORIZONTAL	UNO	UNLESS NOTED OTHERWISE
HR	HOUR		
HS	HEADED STUD		
HT	HEIGHT	V	VAPOR BARRIER
		VEF	VERTICAL EACH FACE
ID	INSIDE DIAMETER	VERT	VERTICAL
IN	INCH	VIF	VERTICAL INSIDE FACE
INCL	INCLUDING	VOF	VERTICAL OUTSIDE FACE
INSUL	INSULATION		
INT	INTERIOR	W	WEST, WIDTH
		W/	WITH
JST	JOIST	WD	WIDTH
JT	JOINT	WF	WIDE FLANGE
		W/O	WITHOUT
K	KIP (1000 POUNDS)	WP	WORK POINT
		WT	WEIGHT
		WWF	WELDED WIRE FABRIC
		WWR	WELDED WIRE REINFORCEMENT
		YD	YARD

WOOD-RELATED ABBREVIATIONS

AF&PA	AMERICAN FOREST & PAPER ASSOCIATION
APA	APA-ENGINEERED WOOD ASSOCIATION (AMERICAN PLYWOOD ASSOCIATION)
AWPA	AMERICAN WOOD-PRESSERVERS' ASSOCIATION
NDS	NATIONAL DESIGN SPECIFICATION

STRUCTURAL NOTES

DESIGN LOADS & MISCELLANEOUS NOTES

G1. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE 2006 INTERNATIONAL BUILDING CODE & GEORGIA AMENDMENTS & ASCE 7-05, "MINIMUM DESIGN LOADS FOR BUILDINGS & OTHER STRUCTURES."

G2. DESIGN LOADS ARE AS FOLLOWS:

DEAD LOADS	
15 PSF 10 PSF 10 PSF	SUPERIMPOSED ELEVATED FLOOR SUPERIMPOSED ROOF TRUSS BOTTOM CHORD SUPERIMPOSED ROOF TRUSS TOP CHORD EQUIPMENT OPERATING WEIGHTS SHALL BE PROVIDED BY CONTRACTOR FOR EQUIPMENT SELECTED
LIVE LOADS	
60 PSF 20 PSF OR 300* 100 PSF	ELEVATED FLOORS ROOF SLABS-ON-GRADE U.N.O.
OCCUPANCY CATEGORY & IMPORTANCE FACTORS	
I = II Is = 1.00 Iw = 1.00 Ie = 1.00 C	OCCUPANCY CATEGORY SNOW IMPORTANCE FACTOR WIND IMPORTANCE FACTOR SEISMIC IMPORTANCE FACTOR EXPOSURE (TERRAIN) CATEGORY FOR SNOW & WIND
SNOW LOAD	
Pg = 5 PSF Pf = 3.9 PSF Ce = 1.0 Ct = 1.1 Cs = 0.88 Ps = 8.3 PSF	GROUND SNOW LOAD MINIMUM ROOF SNOW LOAD SNOW EXPOSURE FACTOR THERMAL FACTOR ROOF SLOPE FACTOR SLOPED-ROOF SNOW LOAD
WIND LOAD	
V = 90 MPH G = 0.85 Kzt = 1 Kd = 0.85 GCpi = +/- 0.18 3.5 FT -24.0 PSF -15.0 PSF -36.5 PSF -20.0 PSF -15.0 PSF	MAIN WIND FORCE RESISTING SYSTEMS SHALL BE THE SAME AS THOSE LISTED FOR EARTHQUAKE LOADS BASIC WIND SPEED GUST EFFECT FACTOR TOPOGRAPHIC FACTOR DIRECTIONALITY FACTOR INTERNAL PRESSURE COEFFICIENT COMPONENTS & CLADDING DESIGN PRESSURES: EDGE STRIP WIDTH ROOF EDGE STRIP BALANCE OF ROOF ROOF OVERHANG WALL EDGE STRIP BALANCE OF WALLS
EARTHQUAKE DESIGN DATA	
SS = 24.5 S1 = 9.00 0 SDS = 0.261 SD1 = 0.144 SDC = B R = 2 omegaO = 2 Cd = 1.75 V = 0.13W	MAPPED SPECTRAL ACCELERATION (PER MAPS) SHORT PERIODS 1-SECOND PERIODS SITE CLASSIFICATION (ESTIMATED) DESIGN SPECTRAL RESPONSE COEFFICIENT SHORT PERIODS 1-SECOND PERIODS SEISMIC DESIGN CATEGORY BASIC SEISMIC-FORCE-RESISTING SYSTEM -- ORDINARY REINFORCED MASONRY SHEAR WALLS RESPONSE MODIFICATION FACTOR SYSTEM OVERSTRENGTH FACTOR DEFLECTION AMPLIFICATION FACTOR BASE SHEAR
	ANALYSIS: EQUIVALENT LATERAL FORCE PROCEDURE

G3. THE SLABS-ON-GRADE HAVE BEEN DESIGNED ONLY FOR THE LOADS SHOWN ON THESE CONSTRUCTION DOCUMENTS. GREATER LOADS DUE TO CONSTRUCTION ACTIVITIES, SUCH AS CRANE LOADS, CONCRETE TRUCKS & DELIVERY TRUCKS - MAY DAMAGE THE SLABS. IF SUCH CONSTRUCTION LOADINGS WILL OCCUR, THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN, DETAILING, MATERIALS, & INSTALLATION OF THE UPGRADED SLABS-ON-GRADE NEEDED TO SUPPORT THESE LOADINGS.

G4. THE STRUCTURE & ITS COMPONENTS HAVE BEEN DESIGNED FOR THE FINAL CONFIGURATIONS SHOWN ON THESE DOCUMENTS. DURING CONSTRUCTION, THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN, ADEQUACY, & SAFETY OF ERECTION BRACING, SHORING, RESHORING, TEMPORARY SUPPORTS, & METHODS & SEQUENCING OF CONSTRUCTION.

G5. THE LOCATIONS OF SLAB DEPRESSIONS, FLOOR SLOPES & FLOOR DRAINS ARE APPROXIMATE. THE CONTRACTOR SHALL COORDINATE EXACT LOCATIONS WITH THE ARCHITECTURAL & PLUMBING DRAWINGS.

G6. TYPICAL & CERTAIN SPECIFIC CONDITIONS HAVE BEEN DETAILED ON THE DRAWINGS. FOR CONDITIONS NOT SPECIFICALLY SHOWN, THE CONTRACTOR SHALL PREPARE DETAILS SIMILAR TO THOSE SHOWN & SUBMIT THEM WITH THE RELEVANT SHOP DRAWINGS FOR APPROVAL.

G7. THE CONTRACTOR SHALL USE THESE STRUCTURAL DRAWINGS ALONG WITH THE ARCHITECTURAL & OTHER DRAWINGS FOR DETERMINING DIMENSIONS, ELEVATIONS, & DETAILS. FEATURES SHOWN ON THE CONTRACT DRAWINGS SHALL NOT BE SCALED FOR THE PURPOSE OF ESTABLISHING THEIR CORRECT DIMENSIONS.

G8. THE CONTRACTOR SHALL MAKE AVAILABLE AT ALL TIMES AT THE JOB SITE COMPLETE SHOP DRAWINGS OF ALL APPLICABLE SPECIALTY ITEMS. THESE SHOP DRAWINGS & THEIR CALCULATIONS SHALL BE STAMPED & SIGNED BY PROFESSIONAL ENGINEERS LICENSED IN THE STATE OF TEXAS. THE SPECIALTY ITEMS INCLUDE BUT ARE NOT LIMITED TO CURTAIN WALL GLAZING SYSTEMS, COLD-FORMED STEEL FRAMING, GUARDRAILS, HANDRAILS, & STAIRS, AS WELL AS OTHER ITEMS THAT MAY BE INDICATED IN THE CONSTRUCTION DOCUMENTS.

FOUNDATION NOTES

F1. NO GEOTECHNICAL REPORT WAS MADE AVAILABLE. ASSUMPTIONS MADE IN FOUNDATION & SEISMIC DESIGN ARE BASED ON THOSE VALUES PERMITTED BY BUILDING CODE FOR PROJECTS LACKING GEOTECHNICAL DATA.

F2. FOUNDATIONS CONSIST OF SPREAD OR STRIP FOOTINGS & SHALL BEAR ON VIRGIN SOIL OF STRUCTURAL FILL. FOUNDATIONS HAVE BEEN DESIGNED FOR A NET SOIL BEARING PRESSURE OF 1500 PSF. THE ALLOWABLE SOIL BEARING PRESSURE SHALL BE VERIFIED BY THE TESTING AGENCY DURING CONSTRUCTION & PRIOR TO PLACING FOOTING CONCRETE. IF THE ACTUAL JOBSITE CONDITIONS DICTATE A DIFFERENT BEARING CAPACITY, CONTACT THE ARCHITECT FOR ADDITIONAL INSTRUCTIONS, WHICH MAY INCLUDE FOUNDATION REDESIGN.

F3. WHEREVER FOOTINGS BEAR ON STRUCTURAL FILL, THE STRUCTURAL FILL SHALL EXTEND LATERALLY BEYOND THE EDGES OF THE FOOTINGS FOR A MINIMUM DISTANCE EQUAL TO THE DEPTH OF THE STRUCTURAL FILL BENEATH THE FOOTINGS.

F4. FOOTINGS SHALL BE CENTERED ON COLUMN LINES & WALLS UNLESS NOTED OTHERWISE.

F5. EXTERIOR GRADES ADJACENT TO THE BULDING SHALL BE SLOPED TO DRAIN AWAY FROM THE FOUNDATION. ALL ROOF DOWNSPOUTS & DRAINS SHALL DISCHARGE WELL BEYOND THE LIMITS OF ALL BACKFILL & SHALL NOT BE ALLOWED TO POND ADJACENT TO THE BUILDING.

CAST-IN-PLACE CONCRETE NOTES

C1. MATERIAL, CONSTRUCTION & TESTING OF ALL CAST-IN-PLACE CONCRETE SHALL BE IN ACCORDANCE WITH APPLICABLE PROVISIONS OF ACI 301, ACI 315, & ACI 318.

C2. ALL CAST-IN-PLACE CONCRETE SHALL CONTAIN TYPE I, TYPE II, OR TYPE III PORTLAND CEMENT & SHALL BE IN ACCORDANCE WITH THE SPECIFICATIONS AS FOLLOWS:

STRENGTH	DENSITY	SLUMP	LOCATION
3000 PSI	145 PCF	4" (+/- 1")	FOOTINGS, SLAB-ON-GRADE, SITE WALLS

A COPY OF CONCRETE CYLINDER TEST REPORTS SHALL BE AVAILABLE AT THE JOB SITE.

C3. USE OF CALCIUM CHLORIDE, CHLORIDE IONS, OR OTHER SALTS IN CONCRETE OR CONCRETE ADMIXTURES IS NOT PERMITTED. ALUMINUM MATERIAL SHALL NOT BE EMBEDDED IN OR USED IN CONCRETE PLACEMENT.

C4. REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A615, INCLUDING SUPPLEMENT S1, GRADE 60, DETAIL REINFORCING STEEL & PROVIDE BAR SUPPORTS IN ACCORDANCE WITH ACI DETAILING MANUAL. U.N.O., ALL REINFORCING BAR SPLICES SHALL BE CLASS "B" TENSION LAP SPLICES U.N.O. ALL HOOKS SHALL BE STANDARD HOOKS U.N.O. TIES & STIRRUPS SHALL HAVE 135-DEGREE HOOKS U.N.O.

C5. PROVIDE CONTINUOUS REINFORCEMENT WHEREVER POSSIBLE. REINFORCEMENT SPLICES SHALL BE MADE ONLY AS REQUIRED OR PERMITTED ON THE STRUCTURAL DESIGN DRAWINGS.

C6. WELDED WIRE REINFORCEMENT SHALL CONFORM TO ASTM A185 & SHALL BE PROVIDED IN FLAT SHEETS. ALL WELDED WIRE REINFORCEMENT SHALL BE LAPPED TWO FULL PANELS & TIED. DO NOT CONTINUE WELDED WIRE REINFORCEMENT THROUGH CONSTRUCTION & CONTROL/CONTRACTION JOINTS IN SLABS-ON-GRADE.

C7. FIELD CUTTING, WELDING, TACK WELDING, OR HEATING REINFORCEMENT IS PROHIBITED, U.N.O.

C8. REINFORCING STEEL PLACEMENT SHALL BE REVIEWED BY THE TESTING AGENCY, PRIOR TO PLACING CONCRETE.

C9. PROVIDE BAR SUPPORTS & SPACERS IN ACCORDANCE WITH ACI 315 & CRSI "MANUAL OF STANDARD PRACTICE". ALL BAR SUPPORTS IN AREAS WHERE CONCRETE WILL BE EXPOSED SHALL HAVE PLASTIC TIPPED FEET. THE CONTRACTOR IS RESPONSIBLE TO PREVENT EXPOSURE OF TIE WIRE OR OTHER MATERIAL WHICH MAY CAUSE STAINING OF CONCRETE. SPECIFIED COVER SHALL BE PROVIDED ON ALL REINFORCEMENT.

C10. UNO MINIMUM CONCRETE COVER OVER REINFORCEMENT SHALL BE PROVIDED AS FOLLOWS

CONCRETE CAST AGAINST EARTH.....3"

CONCRETE CAST IN FORMS BUT EXPOSED TO WEATHER & EARTH.....2"

C11. NO HORIZONTAL JOINTS ARE PERMITTED IN SLABS, JOIST, WALLS OR BEAMS. ANY STOPPAGE IN CONCRETE WORK SHALL BE MADE AT THE CENTER OF THE SPAN OR SUPPORT WITH VERTICAL BULKHEAD & HORIZONTAL KEYS, UNO.

C12. CONDUITS, PIPES & SLEEVES EMBEDDED IN CONCRETE:
- A. SHALL NOT BE LARGER IN OUTSIDE DIMENSION THAN ONE-THIRD THE OVERALL THICKNESS OF THE SLAB, WALL OR BEAM IN WHICH THEY ARE EMBEDDED.
- B. SHALL BE NO CLOSER THAN 3 DIAMETERS ON CENTER.
- C. SHALL BE UNCOATED OR GALVANIZED STEEL NOT THINNER THAN STANDARD SCHEDULE 40 PIPE IN COLUMNS & BEAMS.
- D. WITH THEIR FITTINGS, THEIR MAXIMUM AREA EMBEDDED IN CONCRETE COLUMNS SHALL NOT EXCEED 4 PERCENT OF THE CROSS-SECTIONAL AREA OF THE COLUMN.
- E. SHALL BE SO FABRICATED & INSTALLED THAT CUTTING, BENDING OR DISPLACEMENT OF REINFORCEMENT FROM ITS PROPER LOCATIONS WILL NOT BE REQUIRED.
- F. SHALL NOT BE PLACED IN FOOTINGS.

REINFORCED MASONRY

RM1. FINISHED MASONRY CONSTRUCTION SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH (f'm) OF 1,500 PSI.

RM2. HOLLOW CONCRETE MASONRY SHALL CONFORM TO ASTM C90, TYPE N, WITH A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI BASED ON NET AREA PER ASTM C140.

RM3. ALL MORTARS SHALL BE TYPE S CONFORMING TO ASTM C270. ADMIXTURES SHALL NOT BE ADDED FOR ANY REASON UNLESS APPROVED BY THE ENGINEER. MORTAR SHALL NOT BE USED TO FILL BLOCK CELLS.

RM4. ALL REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60. ALL REINFORCING BAR SPLICES SHALL BE PER THE SCHEDULE OF LAP LENGTHS FOR CMU, & SHALL BE AT LOCATIONS APPROVED BY THE ARCHITECT. VERTICAL REINFORCING BARS RUN CONTINUOUSLY FROM TOP OF FOUNDATION TO TOPS OF WALLS BUT MAY BE SPLICED AS NOTED.

RM5. ALL HORIZONTAL & VERTICAL MASONRY REINFORCING SHALL BE SOLIDLY GROUTED IN PLACE USING LOW LIFT GROUTING TECHNIQUES. PROVIDE VERTICAL BAR POSITIONERS IN ALL REINFORCED & GROUTED WALLS. BAR POSITIONERS SHALL BE PLACED AT THE TOP OF THE FIRST COURSE, ONE COURSE BELOW THE TOP OF THE WALL & AT TEN FEET ON CENTER AT EACH VERTICAL REINFORCED BLOCK CORE.

RM6. FILL ALL VOIDS & BLOCK CELLS SOLIDLY WITH GROUT FOR A DISTANCE OF 24" BENEATH & 24" EACH SIDE OF ALL BEAM REACTIONS OR OTHER CONCENTRATED LOADS U.N.O.

RM7. ALL GROUT FOR BOND BEAMS & VERTICAL BLOCK WALL CORES SHALL BE STRUCTURAL QUALITY WITH 3/8" MAX COARSE AGGREGATE & 3,000 PSI AT 28-DAY COMPRESSIVE STRENGTH & CONFORM TO ASTM C476. GROUT SHALL BE PLACED TO WITHIN 1½" OF THE TOP OF THE BLOCK COURSE TO ALLOW FOR NEXT LIFT TO BE KEYED INTO THE PREVIOUS LIFT.

RM8. HORIZONTAL JOINT REINFORCEMENT SHALL BE PLACED AT 1'-4" ON CENTER MAXIMUM. SEE JUSTICE ARCHITECTURE DRAWINGS FOR LOCATIONS OF 8" ON CENTER SPACING OF JOINT REINFORCEMENT. SEE DIVISION 4 SPECS FOR JOINT REINFORCEMENT. AT OPENINGS, HORIZONTAL REINFORCEMENT SHALL BE PLACED IN THE FIRST & SECOND BED JOINTS 8" APART IMMEDIATELY ABOVE LINTELS & BELOW SILLS. REINFORCEMENT IN THE SECOND BED ABOVE OR BELOW OPENINGS SHALL EXTEND 2'-0" MINIMUM BEYOND THE JAMBS. ALL OTHER REINFORCEMENT SHALL BE CONTINUOUS EXCEPT THAT IT SHALL NOT PASS THROUGH VERTICAL MASONRY CONTROL JOINTS. LAPS SHALL BE 6" MINIMUM FOR DEFORMED WIRE & 12" FOR PLAIN.

RM9. PROVIDE CONTINUOUS REINFORCED BOND BEAMS IN MASONRY WALLS: (A) 16" DEEP AT TOP OF ALL WALLS; (B) AS SHOWN ON THE DRAWINGS; REINFORCE BOND BEAMS WITH 2-#5 CONT IN EACH COURSE U.N.O.

RM10. ALL CMU WALLS MUST BE BRACED AT THE TOP BY THE STRUCTURE AT 4'-0" ON CENTER MAXIMUM INTERVALS.

RM11. CONDUITS, PIPES & SLEEVES EMBEDDED IN MASONRY SHALL BE NO CLOSER THAN 3 DIAMETERS ON CENTER. THE MAXIMUM AREA OF CONDUITS, PIPES & SLEEVES SHALL NOT EXCEED 2 PERCENT OF THE CROSS-SECTIONAL AREA OF THE MASONRY COLUMNS, PILASTERS OR OPENING JAMBS IN WHICH THEY ARE EMBEDDED. PIPES SHALL NOT BE EMBEDDED IN MASONRY WHEN:
- A. CONTAINING LIQUIDS, GASES OR VAPORS AT TEMPERATURES HIGHER THAN 150 DEGREES F;
- B. UNDER PRESSURE IN EXCESS OF 55 PSI;
- C. CONTAINING WATER OR OTHER LIQUIDS SUBJECT TO FREEZING.

WOOD NOTES

W1. GENERAL
- A. WOOD CONSTRUCTION SHALL BE PORVIDED IN ACCORDANCE WITH THE BUILDING CODE & THE 2005 EDITION OF AF & PA's WOOD DESIGN PACKAGE: NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION, MANUAL FOR ENGINEERED WOOD CONSTRUCTION, SPECIAL DESIGN PROVISIONS FOR WIND AND SEISMIC, & STRUCTURAL WOOD DESIGN SOLVED EXAMPLE PROBLEMS.
- B. IN ADDITION TO SELF-WEIGHT, WOOD FRAMING SHALL BE DESIGNED FOR THE LOADS INDICATED ON THE CONSTRUCTION DOCUMENTS.
- C. WOOD FLOOR & ROOF FRAMING SHALL BE DESIGNED WITH MAXIMUM DEFLECTIONS OF L/240 FOR DEAD LOADS PLUS LIVE LOAD & L/360 FOR LIVE LOADS.
- D. MINIMUM FASTENING SHALL BE IN ACCORDANCE WITH BUILDING CODE TABLE 2304.9.1 U.N.O. FASTENERS SHALL BE COMPATIBLE WITH THE CHEMICALS OF TREATED WOOD.

W2. TREATED WOOD PRODUCTS
- A. ALL EXPOSED EXTERIOR LUMBER & WOOD IN CONTACT W/ CONCRETE, MASONRY & STONE SHALL RECEIVE PRESERVATIVE PRESSURE TREATMENT FOR PROTECTION FROM DECAY & TERMITES IN ACCORDANCE WITH THE BUILDING CODE & THE AWPA.
- B. ALL FIELD CUTS SHALL BE TREATED IN ACCORDANCE WITH THE AWPA.

W3. SAWN LUMBER
- A. LUMBER GRADE & SPECIES SHALL BE SOUTHERN PINE NO. 2 OR BETTER.
- B. PROVIDE DOUBLE TOP PLATE LAPPED AT CORNERS & AT SPLICES. OFFSET SPLICES AT LEAST 4 FT OC & PLACE OVER WALL STUDS.
- C. PROVIDE AT LEAST THREE FULL HEIGHT STUDS AT WALL CORNERS. PROVIDE AT LEAST TWO FULL HEIGHT STUDS AT EACH SIDE OF OPENINGS LESS THAN 6 FT WIDE & AT LEAST ONE JACK STUD.
- D. HOLES IN JOISTS, RAFTERS, STUDS, & PLATES NOT EXCEEDING 1/3 THE DEPTH/WIDTH OF THE MEMBER & NOT CLOSER THAN 3 TIMES THE DEPTH/WIDTH OF THE MEMBER FROM THE END ARE PERMITTED AT THE MID-DEPTH/WIDTH OF THE MEMBER.
- E. BRIDGING, FULL DEPTH SOLID BLOCKING, OR CROSS BRACING SHALL BE INSTALLED AT INTERVALS NOT EXCEEDING 8 FT O.C. WHEN THE DEPTH-TO-BREADTH RATIO OF SAWN JOISTS & RAFTERS EXCEEDS 6 TO 1.

W4. PLYWOOD
- A. PLYWOOD SHALL BE PROVIDED WITH THE RECOMMENDATIONS OF THE AMERICAN PLYWOOD ASSOCIATION (APA). THE MINIMUM THICKNESS WHICH FOLLOW SHALL BE INCREASED AS REQUIRED TO SATISFY ARCHITECTURAL REQUIREMENTS.
- B. ROOF SHEATHING SHALL BE 15/32" APA RATED SHEATHING, 32/16, EXPOSURE 1, 48"x96" WOOD. PLYWOOD SHALL BE TONGUE AND GROOVE OR BE INSTALLED WITH PANEL CLIPS IN ACCORDANCE WITH APA RECOMMENDATIONS. WHERE ALLOWABLE SPANS ARE EXCEEDED AT ROOF SLOPE TRANSITIONS, PROVIDE SPECIALLY DESIGNED SUPPLEMENTAL MEMBERS AS REQUIRED.

  SHEATHING SHALL BE INSTALLED WITH THE LONG EDGE ACROSS A MINIMUM OF THREE SUPPORTING MEMBERS. SUPPORT AND STAGGER EDGES OF PLYWOOD PARALLEL TO SUPPORTING MEMBERS. PROVIDE CONTINUOUS BLOCKING AT PERIMETER OF EACH DIAPHRAGM PLANE (INCLUDING ROOF SLOPE TRANSITIONS) AND AROUND OPENINGS. WHERE INTERFERENCE OCCURS WITH RIDGE VENTS, PROVIDE SOLID BLOCKING BETWEEN ALTERNATE ROOF JOISTS OR TRUSSES. FASTEN SHEATHING WITH 8D NAILS AT 6" O/C AT SUPPORTED EDGES U.N.O. AND AT 12" O/C AT INTERMEDIATE SUPPORTS. (V = 240 LBS./FT 8D NAILS, V = 255 LBS./FT 10D NAILS FOR LOAD PARALLEL TO SUPPORTS: V = 180 LBS./FT 8D, V = 190 LBS./FT 10D NAILS FOR LOAD PERPENDICULAR TO SUPPORTS.)

  A 1/8" GAP SHALL BE LEFT BETWEEN ADJACENT PANELS. PROTECT EDGES AGAINST EXPOSURE TO WEATHER OR USE EXTERIOR GRADE PLYWOOD.

  COVER SHEATHING AS SOON AS POSSIBLE WITH ROOFING FELT OR SHINGLE UNDERLAYMENT FOR PROTECTION AGAINST EXCESSIVE MOISTURE PRIOR TO ROOFING INSTALLATION.
- C. SUPPLEMENTAL BOTTOM CHORD DIAPHRAGM SHEATHING SHALL BE 15/32" APA RATED SHEATHING, 32/16, EXPOSURE 1, 48"x96" PLYWOOD.

  SHEATHING SHALL BE INSTALLED WITH THE LONG EDGE ACROSS A MINIMUM OF THREE SUPPORTING MEMBERS. SUPPORT AND STAGGER EDGES OF PLYWOOD PARALLEL TO SUPPORTING MEMBERS. PROVIDE CONTINUOUS BLOCKING AT ALL OTHER EDGES AND AROUND OPENINGS.

  FASTEN WITH 10D NAILS AT 2" O/C AT SUPPORTED PANEL EDGES AND AT 3" O/C AT INTERMEDIATE SUPPORTS. (V = 500 LBS./FT 8D NAILS, V = 655 LBS./FT 10D NAILS FOR LOADS PARALLEL AND PERPENDICULAR TO SUPPORTS.)

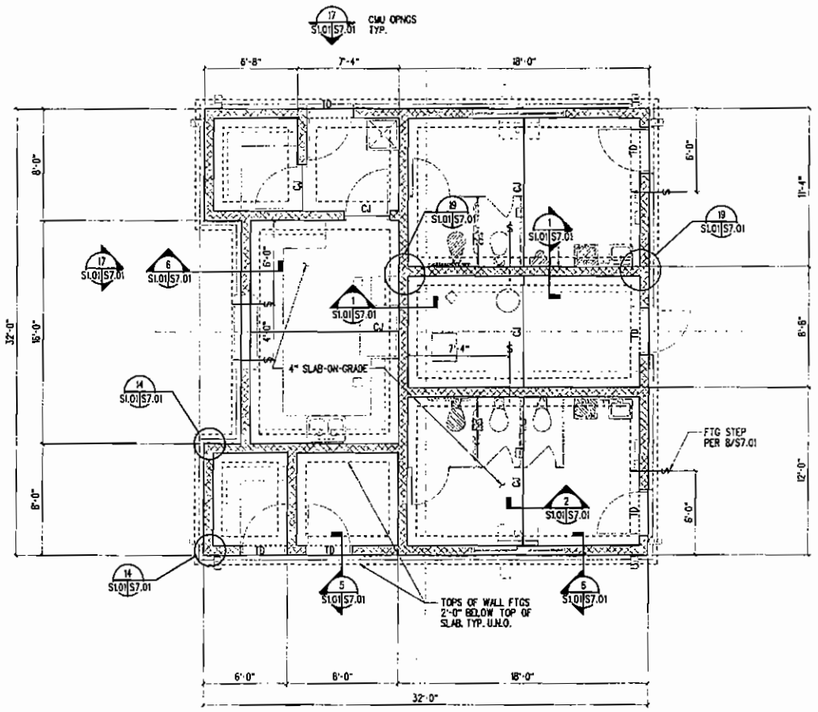
  A 1/8" GAP SHALL BE LEFT BETWEEN ADJACENT PANELS.
- D. WALL SHEATHING SHALL BE 15/32" APA RATED SHEATHING, 32/16, EXPOSURE 1 48"x96" PLYWOOD WITH CONTINUOUSLY SUPPORTED EDGES. A 1/8" GAP SHALL BE LEFT BETWEEN ADJACENT PANELS. PROVIDE A MINIMUM OF FULL HEIGHT CONTINUOUSLY BLOCKED 48" WIDE PANELS BY 2.5X OF THE WALL LENGTH AT EACH END OF EXERIOR WALLS U.N.O.

  FASTEN SHEATHING WITH 8D NAILS @4" O/C AT PANEL EDGES & AT 12" O/C AT INTERMEDIATE SUPPORTS. (V = 380 LBS./FT @ 4" O/C)
- E. FLOOR SHEATHING SHALL BE 3/4" APA RATED STURD-I-FLOOR, 48/24, EXPOSURE 1, 48"x96" TONGUE & GROOVE PLYWOOD. FASTEN ALL PANELS WITH 8D NAILS @ 6" O/C AT PANEL EDGE SUPPORTS & 12" O/C AT INTERMEDIATE SUPPORTS (V = 180 LBS./FT).

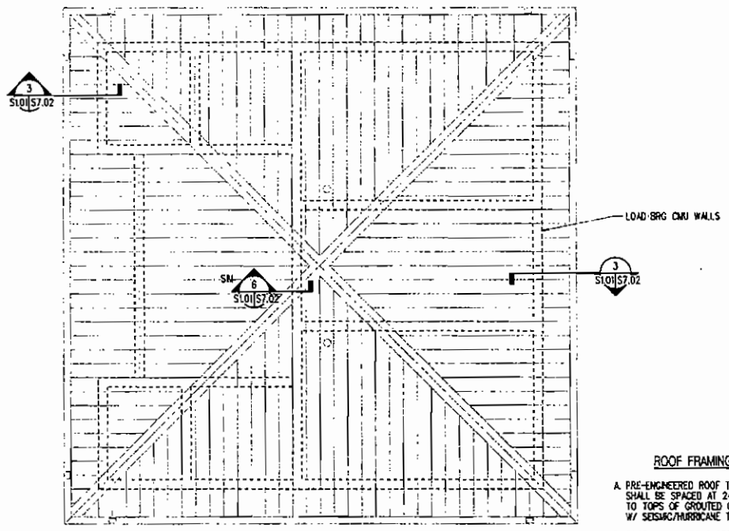
  FASTEN SHEATHING TO UNBLOCKED OR BLOCKED WOOD FRAMING WITH #8x2" GRABBER FLAT HEAD PLYWOOD SCREWS @ 6" O/C AT PANEL EDGES & @ 12" O/C AT INTERMEDIATE SUPPORTS. MINIMUM EDGE DISTANCE SHALL BE 3/8". (V = 190 PLF)

  PANELS SHALL BE APA GLUED TO SUPPORTS & AT TONGUE & GROOVE JOINTS WITH ADHESIVE CONFORMING TO PERFORMANCE SPECIFICATION APA AFG-01. AN 1/8" GAP SHALL BE LEFT BETWEEN ADJACENT PANELS & STAGGERED. SHEATHING SHALL BE INSTALLED WITH THE LONG EDGE ACROSS A MINIMUM OF THREE SUPPORTING MEMBERS. PROVIDE CONTINUOUS BLOCKING AT PERIMETER OF THE DIAPHRAGM PLANE & AROUND OPENINGS.

W5. PRE-ENGINEERED ROOF TRUSSES
- A. TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE NDS & THE DESIGN SPECIFICATION FOR METAL PLATE CONNECTED WOOD TRUSSES PUBLISHED BY THE TRUSS JOINT INSTITUTE, LATEST EDITION. CONNECTIONS BETWEEN TRUSSES AS WELL AS THE ENDS OF STUD TRUSSES SHALL BE DESIGNED & SPECIFIED BY THE TRUSS ENGINEER. TRUSSES SHALL HAVE A MINIMUM NOMINAL BEARING LENGTH OF 4 INCHES (3½" ACTUAL).
- B. SUBMITTAL FROM THE SUPPLIER SHALL CONSIST OF CALCULATIONS AND/OR PUBLISHED DESIGN DATA & SHOP DRAWINGS STAMPED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF GEORGIA. THE SHOP DRAWINGS & CALCULATIONS SHALL BE PREPARED UNDER THE DIRECT SUPERVISION OF THE ENGINEER STAMPING THIS WORK. SHOP DRAWINGS SHALL INCLUDE PLANS, DETAILS, SPACING, BRACING, BRIDGING, SUPPORT REACTIONS, & DEFLECTIONS.
- C. A COPY OF THE TRUSS SHOP DRAWINGS STAMPED BY THE TRUSS ENGINEER & REVIEWED BY THE ENGINEER-OF-RECORD SHALL BE AVAILABLE ON THE JOB SITE DURING INSPECTIONS.
- D. ADDITIONAL RIDGE & VALLEY



19 S1.01 FOUNDATION & FLOOR PLAN
1/4"=1'-0"



ROOF FRAMING NOTES

A. PRE-ENGINEERED ROOF TRUSSES AND/OR WOOD FRAMING SHALL BE SPACED AT 24" OC MAX. ANCHOR ROOF FRAMING TO TOPS OF GROUTED CMU WALLS AT ALL BEARING POINTS W/ SEISMIC/HURRICANE TIES.

B. ROOF TRUSS BOT CHORD SHEATHING SHALL BE PER THE GENERAL NOTES.

17 S1.01 ROOF FRAMING PLAN
1/4"=1'-0"

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MARK	DATE	DESCRIPTION

ROSSER PROJECT NUMBER	06241.0032
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CHECKED BY	MB
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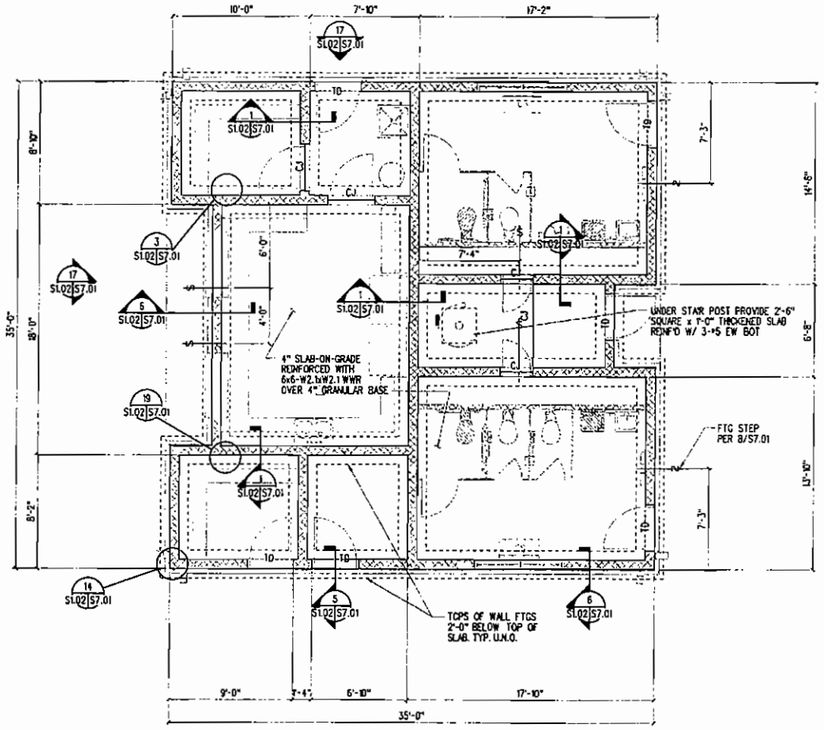
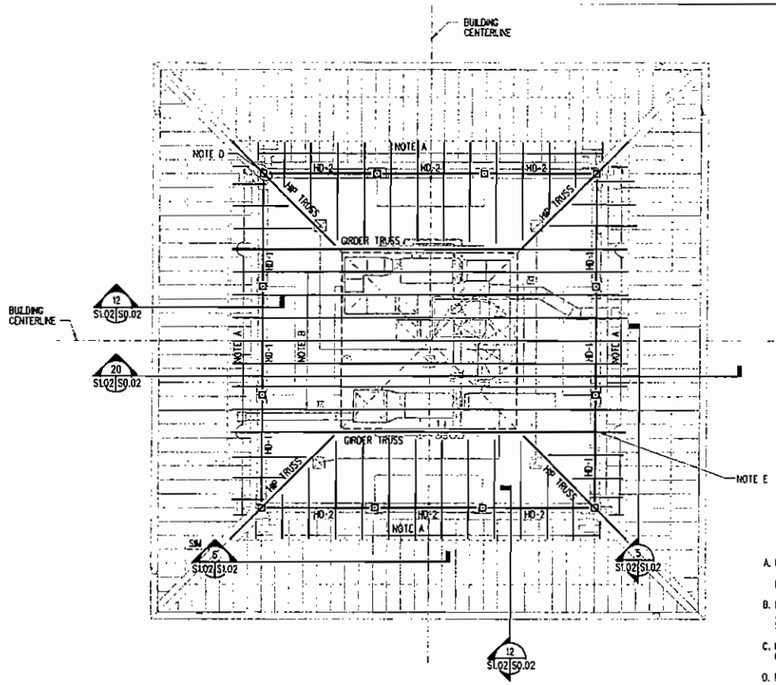


1 S1.02 FOUNDATION & FIRST FLOOR PLAN
1/4" = 1'-0"



HEADER SCHEDULE			
MARK	SIZE	JACK STUDS	POSTS
HD-1	5¼x9¼ LVL	(2) 2x6	6x6 POST BETWEEN
HD-2	(3) 2x10	(2) 2x6	6x6 POST BETWEEN

SEE ELEVATION 5/S1.02 FOR POST, HEADER AND STUD ORIENTATION

ROOF FRAMING NOTES

A. ROOF TRUSSES SHALL BE SPACED AT 24" OC MAX. ANCHOR TO TOP PLATES/HEADERS/STUDS W/ SIMPSON H-6, H-10 (OR EQUAL).

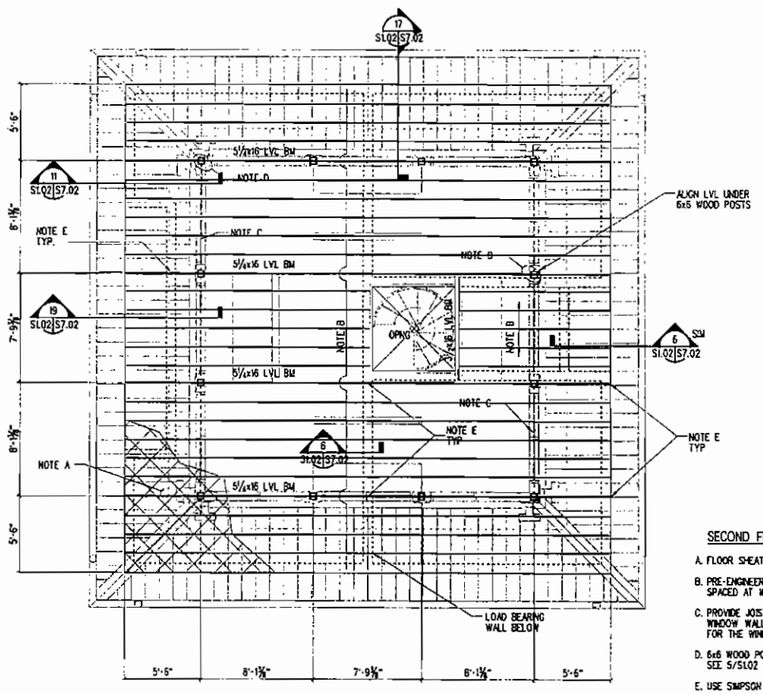
B. ROOF TRUSSES OVER MECHANICAL ATTIC PLATFORM SHALL ALLOW 30" MIN CLEARANCE BETWEEN TOP & BOT CHORDS. SEE 20/S7.02.

C. ROOF & TRUSS BOT CHORD SHEATHING SHALL BE PER THE GENERAL NOTES.

D. POST FROM FLOOR BELOW.

E. USE SIMPSON LGT-2 TIE DOWN (OR EQUAL) TO HEADER

3 S1.02 ROOF FRAMING PLAN
1/4" = 1'-0"



SECOND FLOOR FRAMING NOTES

A. FLOOR SHEATHING SHALL BE PER THE GENERAL NOTES.

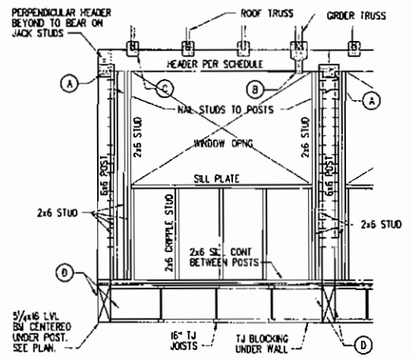
B. PRE-ENGINEERED FLOOR JOISTS SHALL BE 16" DEEP & SPACED AT 16" OC MAX.

C. PROVIDE JOIST BLOCKING BETWEEN FLOOR JOISTS UNDER WINDOW WALL SYSTEM. FLOOR JOISTS SHALL BE DESIGNED FOR THE WINDOW WALL SYSTEM WEIGHT OF 200 PLF.

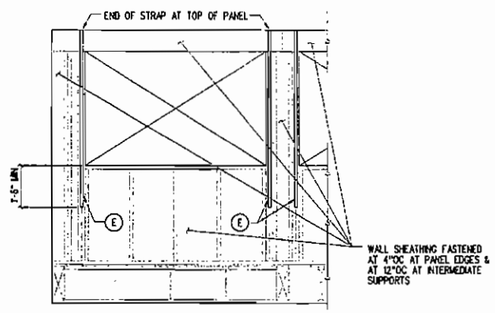
D. 6x6 WOOD POST BEARS ON FLOOR FRAMING (TYP 12 PLACES). SEE 5/S1.02 FOR WALL EL.

E. USE SIMPSON MGT TIEDOWN FOR LVL GIRDER TYP. 12 PLACES

2 S1.02 SECOND FLOOR FRAMING PLAN
1/4" = 1'-0"



A	SIMPSON AC6 MAX (OR EQUAL)
B	SIMPSON LGT2 GIRDER TRUSS TIE DOWN (OR EQUAL)
C	SIMPSON H10 TRUSS CONN (OR EQUAL)
D	2 SIMPSON COILED STRAP CS16-R (OR EQUAL) - 36" LONG. TIE 6x6 POST TO LVL BEAN EA SIDE.
E	SIMPSON COILED STRAP CS16-R (OR EQUAL). TIE HEADER TO JACK STUDS TYP EA END OF HEADER. EXTEND STRAP 18" BELOW WINDOW SILL OPENING



5 S1.01|S1.02 SECOND FLOOR WINDOW ELEVATIONS (TYP)
1" = 1'-0"

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DRAWN BY	JF
CHECKED BY	YIS
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SHEET TITLE
STRUCTURAL PLANS & ELEVATIONS ALTERNATE

SHEET NUMBER
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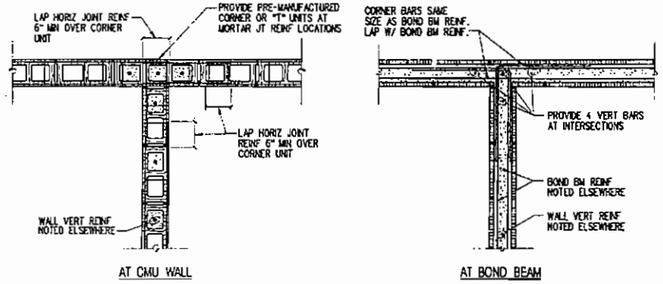
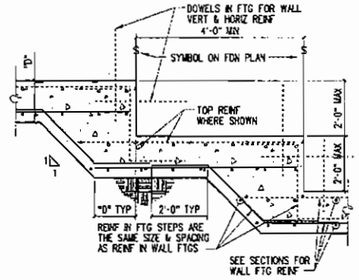
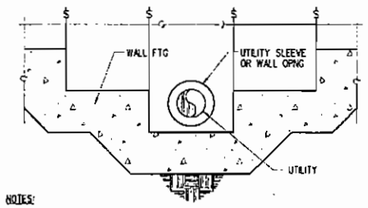


19 S7.01 CMU "T" INTERSECTION DETAILS — NO SCALE



8 S7.01 WALL FOOTING STEP DETAIL — NOT TO SCALE



NOTES:

1. STEP FTG AS REQUIRED TO BE BELOW UTILITY. COORD W/ WALL FTG STEP DETAIL.
2. WHERE UTILITY PASSES THRU WALL, PROVIDE ADDL REINF PER CONC OR CMU WALL OPNG DETAIL.
3. CONTRACTOR SHALL COORD SIZES & LOCATIONS W/ SITE & UTILITY DWGS.
4. SIZE THE SLEEVE/OPNG TO BE MIN 2" BIGGER ALL AROUND THAN UTILITY TO ALLOW FOR POTENTIAL DIFFERENTIAL SETTLEMENT.

4 S7.01 WALL FOOTING STEP AT UTILITIES — NOT TO SCALE

LAP LENGTHS FOR PLAIN OR GALVANIZED REINFORCEMENT IN NORMAL-WEIGHT CONCRETE														
BAR SIZE	COMPRESSION			HOOK		TENSION								BAR SIZE
	DEVELOPMENT LENGTH		LAP SPLICE LENGTH	DEVELOPMENT LENGTH		DEVELOPMENT LENGTH (TOP BARS)		DEVELOPMENT LENGTH (OTHER BARS)		LAP SPLICE LENGTH (TOP BARS)		LAP SPLICE LENGTH (OTHER BARS)		
	f'c=3ksi	f'c=4ksi		f'c=3ksi	f'c=4ksi	f'c=3ksi	f'c=4ksi	f'c=3ksi	f'c=4ksi	f'c=3ksi	f'c=4ksi	f'c=3ksi	f'c=4ksi	
#3	9"	8"	12"	9"	7"	22"	19"	17"	15"	28"	24"	22"	19"	#3
#4	11"	10"	15"	11"	10"	29"	25"	22"	19"	37"	32"	29"	25"	#4
#5	14"	12"	19"	14"	12"	36"	31"	28"	24"	47"	40"	36"	31"	#5
#6	17"	15"	23"	17"	15"	43"	37"	33"	29"	56"	48"	43"	37"	#6
#7	19"	17"	27"	19"	17"	63"	54"	48"	42"	81"	70"	63"	54"	#7
#8	22"	19"	30"	22"	19"	72"	62"	55"	48"	93"	80"	72"	62"	#8
#9	25"	22"	34"	25"	22"	81"	70"	62"	54"	105"	91"	81"	70"	#9
#10	28"	24"	38"	28"	24"	91"	79"	70"	61"	118"	102"	91"	79"	#10
#11	31"	27"	43"	31"	27"	101"	87"	78"	67"	131"	113"	101"	87"	#11

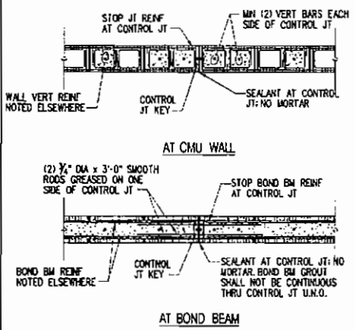
1. PROVIDE LENGTHS INDICATED UNLESS NOTED OTHERWISE IN THE CONSTRUCTION DOCUMENTS. USE TENSION LENGTHS UNLESS COMPRESSION LENGTHS ARE NOTED AS BEING APPLICABLE.
2. DEVELOPMENT VALUES ARE FOR STRAIGHT BARS. WHERE BARS ARE HOOKED, USE THE INDICATED HOOK DEVELOPMENT LENGTHS.
3. TOP BARS ARE HORIZ BARS WITH MORE THAN 12" OF CONCRETE CAST BELOW THEM.
4. WHERE BARS OF DIFFERENT SIZES ARE SPLICED, USE THE LENGTHS REQUIRED FOR THE LARGER BAR.
5. LAP SPLICE LENGTHS SHOWN ARE CLASS "B".
6. ALL LENGTHS SHOWN ARE FOR BARS W/ MIN ONE-BAR-DIA CONCRETE COVER & MIN 2-BAR-DIA SPACING IN BMS OR COLS, OR MIN 3-BAR-DIA SPACING IN OTHER CONCRETE ELEMENTS. WHEN CONCRETE COVER OR SPACING IS LESS THAN THESE VALUES, MULTIPLY THE STRAIGHT BAR LENGTHS BY 1.5.

11 S7.01 LAP LENGTHS FOR REINFORCEMENT IN NORMAL-WEIGHT CONCRETE — NO SCALE

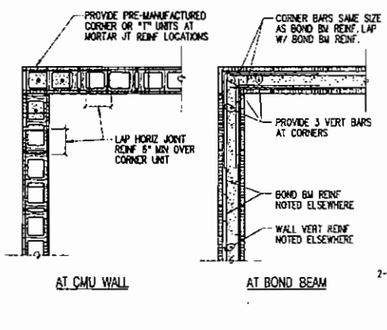
LAP LENGTHS FOR PLAIN OR GALVANIZED REINFORCEMENT IN GROUTED MASONRY: f'm=1500 PSI							
REINFORCEMENT CENTERED IN CELLS							
SIZES	#3	#4	#5	#6	#7	#8	#9
6" CMU	20"	25"	39"	67"	N/A	N/A	N/A
8" CMU	20"	25"	39"	61"	82"	MECH	N/A
12" CMU	20"	25"	39"	56"	82"	MECH	MECH
REINFORCEMENT OFFSET TOWARD CELL WALLS							
SIZES	#3	#4	#5	#6	#7	#8	#9
6" CMU	20"	35"	68"	MECH	N/A	N/A	N/A
8" CMU	20"	35"	68"	MECH	MECH	MECH	N/A
12" CMU	20"	36"	66"	MECH	MECH	MECH	MECH

1. "N/A" INDICATES BAR SIZE IS NOT ALLOWED.
2. "MECH" INDICATES MECHANICAL CONNECTION IS REQUIRED, WHICH SHALL DEVELOP AT LEAST 125% OF SPECIFIED YIELD STRENGTH OF BAR.
3. WHERE BARS OF DIFFERENT SIZES ARE LAPPED, USE THE LENGTHS REQUIRED FOR THE LARGER BAR.
4. THE THICKNESS OF THE GROUT BETWEEN THE BARS & THE CELL WALLS SHALL NOT BE LESS THAN ½".
5. BARS IN LINTELS SHALL BE ONE-PIECE.

3 S7.01 CMU REINF LAP LENGTHS: f'm=1500 PSI — NO SCALE

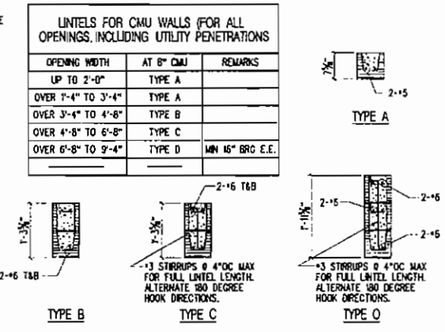


18 S7.01 CMU WALL CONTROL JOINT DETAILS — NO SCALE — COORD LOCATIONS W/ ARCH DWGS

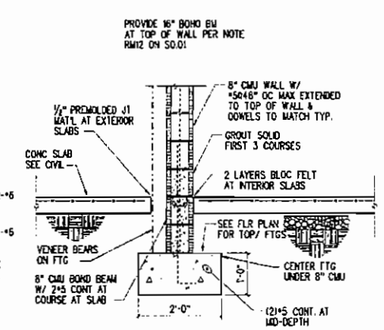


14 S7.01 CMU CORNER DETAILS — NO SCALE

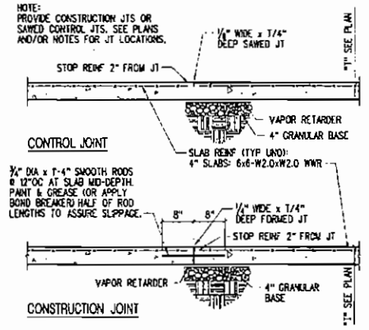
LINTELS FOR CMU WALLS (FOR ALL OPENINGS, INCLUDING UTILITY PENETRATIONS		
OPENING WIDTH	AT 8" CMU	REMARKS
UP TO 2'-0"	TYPE A	
OVER 1'-4" TO 3'-4"	TYPE A	
OVER 3'-4" TO 4'-8"	TYPE B	
OVER 4'-8" TO 6'-8"	TYPE C	
OVER 6'-8" TO 9'-4"	TYPE D	MIN 16" BRG E.E.



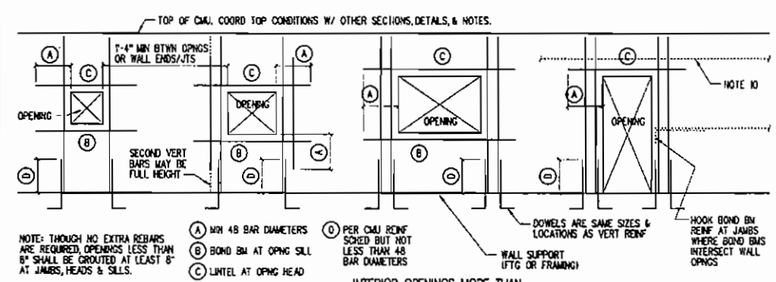
10 S7.01 LINTELS FOR CMU WALLS — NO SCALE — PROVIDE MIN 8" BEARING AT EACH END U.N.O.



8 S7.01 SECTION OF EXTERIOR CMU WALL FOOTING — ¾" = 1'-0"



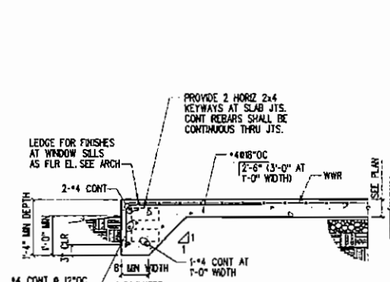
2 S7.01 SLAB-ON-GRADE JOINT DETAILS ("CJ") — NOT TO SCALE — FILL JOINTS W/ SEALANT



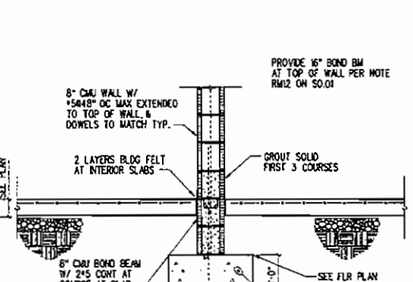
17 S7.01 DETAILS OF CMU WALL REINFORCEMENT AT OPENINGS — NO SCALE — (FOR ROUND, SQUARE OR RECTANGULAR OPENINGS, INCLUDING UTILITY PENETRATIONS)

MASONRY WALL OPENING NOTES

1. FOR VERT REINF OF 2 OR MORE BARS, EACH BAR SHALL BE PLACED IN A SEPARATE CELL U.N.O.
2. VERT BARS SHALL BE #5 MIN BUT SHALL NOT BE LESS THAN THE SIZE OF THE ADJACENT WALL REINF.
3. VERT BARS SHALL BE IN ADDITION TO THE NORMAL WALL REINF U.N.O.
4. WHERE TOP-OF-WALL BOND BMS ARE LESS THAN 1'-4" DEEP, HOOK WALL VERT REINF AT TOPS OF BOND BMS.
5. SEE MASONRY GENERAL NOTES FOR EXTRA JOINT REINF REQUIREMENTS.
6. PROVIDE BOND BMS AT OPENING SILLS. REINF W/ 2-#4 FOR 8" CMU, 2-#4 FOR 8" CMU & 2-#5 FOR 12" CMU. TYP U.N.O.
7. SEE LINTEL SCHED FOR LINTEL TYPES & REINF.
8. WHERE HORIZ REINF CANNOT EXTEND STRAIGHT AS SHOWN BECAUSE OF CONTROL JTS, CORNERS OR OTHER OPENINGS, PROVIDE STD HOOKS AT THE FACES OF THE OBSTRUCTIONS; HOWEVER, LINTEL BEARINGS SHALL NOT BE LESS THAN INDICATED IN THE LINTEL SCHED.
9. ADJACENT OPENINGS 2'-8" OR LESS APART:
   A. REINFORCE ALL VERT CELLS BETWEEN THE OPENINGS FOR THE FULL WALL HEIGHT. PROVIDE #2 HORIZ TIES @ 8" OC W/ 135 OR 180 DEG HOOKS EACH END AROUND VERT REINF.
   B. IF LINTEL BOTTOMS ALIGN, EXTEND THE WIDER OPNG'S LINTEL CONTINUOUS OVER BOTH OPNGS & PROVIDE TOP REINF (EVEN IF TOP REINF IS NOT INDICATED IN THE SCHEDULE).
10. WHERE BOND BMS ALIGN W/ LINTELS, CONTINUE THE BOND BM REINF THRU THE LINTELS. DO NOT ALTER THE LINTEL REINF OR ITS LOCATIONS.



5 S7.01 TURN DOWN SLAB EDGE ("TD") — ¾" = 1'-0"



1 S7.01 SECTION OF INTERIOR CMU WALL FOOTING — ¾" = 1'-0"

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CHECKED BY	WB
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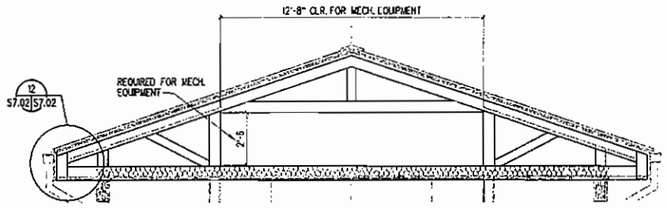
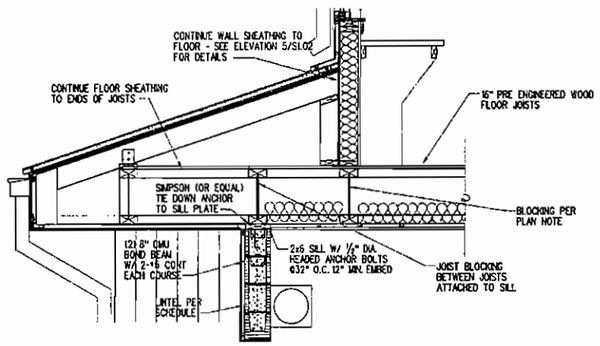
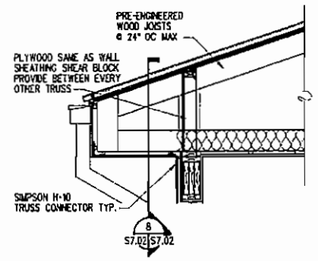


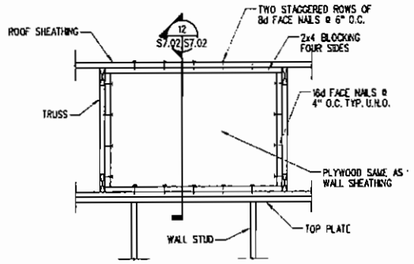
20 TRUSS ELEVATION AT MECHANICAL ATTIC SPACE
S1.02|S7.02 NO SCALE



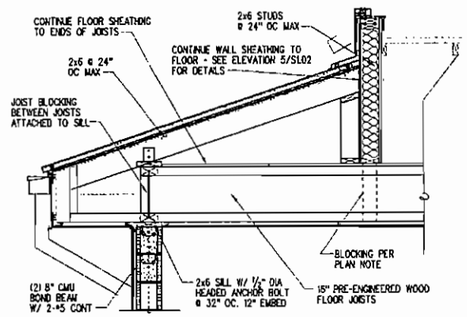
19 SECOND FLOOR & LOW ROOF SECTION
S1.02|S7.02 NO SCALE



12 ROOF SECTION
S1.02|S7.02 NOT TO SCALE



8 SHEAR BLOCK INTERIOR VIEW
S7.02|S7.02 NOT TO SCALE



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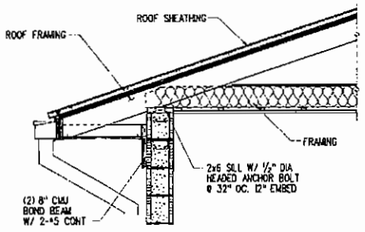
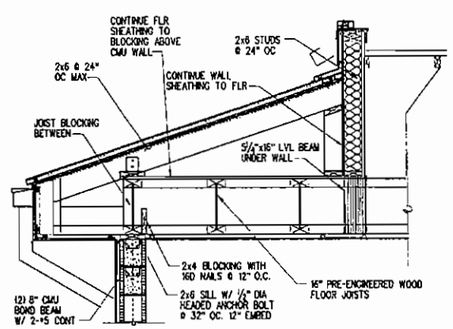
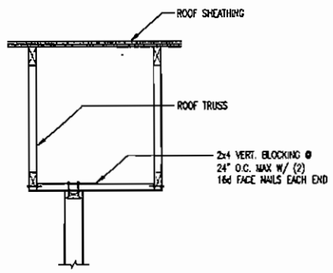
S1.02|S7.02 NO SCALE



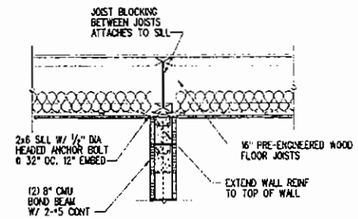
3 ROOF SECTION
S1.01|S7.02 ¼"=1'-0"



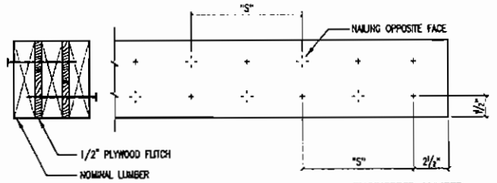
17 SECOND FLOOR & LOW ROOF SECTION
S1.02|S7.02 NO SCALE



13 NON-BEARING PARTITION DETAIL
S1.02|S7.02 NO SCALE

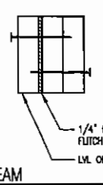


6 SECTION @ JOIST BEARING
S1.02|S7.02 NO SCALE



NOMINAL LUMBER
(SYP NO. 2 OR BETTER)

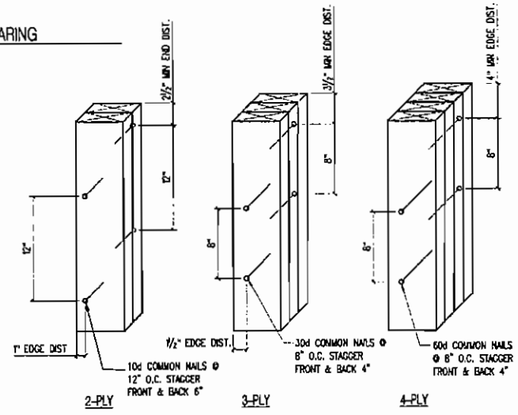
LUMBER SIZE:	NAIL SPACING "S":
2x6	12" O.C. EACH FACE
2x8	12" O.C. EACH FACE
2x10	8" O.C. EACH FACE
2x12	8" O.C. EACH FACE



ENGINEERED LUMBER
(LVL OR PSL)

LUMBER SIZE:	NAIL SPACING "S":
1.75 x 5.5	12" EACH FACE
1.75 x 7.25	9" EACH FACE
1.75 x 9.25	7" EACH FACE
1.75 x 9.50	6" EACH FACE
1.75 x 11.25	5" EACH FACE
1.75 x 11.875	5" EACH FACE
1.75 x 14	4" EACH FACE
1.75 x 16	3" EACH FACE
1.75 x 18	3" EACH FACE

9 THREE PLY BUILT-UP BEAM
S1.02|S7.02 NO SCALE



1 2x6 BUILT-UP COLUMNS
S1.02|S7.02 NO SCALE

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PREVIOUS AND REVISIONS

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DRAWN BY		JF
CHECKED BY		WS
DATE RELEASED FOR CONSTRUCTION		05/14/07

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SECTIONS & DETAILS

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S7.02
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